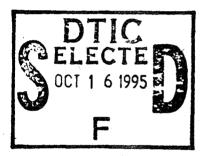
Final Technical Report

Transport Modeling of Hydrogen in Metals for Application to Hydrogen Assisted Cracking of Metals

bv

Dr. James P. Thomas (PI) and Mr. Charles E. Chopin

for the Office of Naval Research Grant Number: N00014-93-1-0845



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Final Technical Report

Transport Modeling of Hydrogen in Metals for Application to Hydrogen Assisted Cracking of Metals

submitted by:

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April 4, 1995

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ONR CONTRACT INFORMATION

Contract Title: "Transport Modeling of Hydrogen in Metals For Application to Hydrogen Assisted Cracking"

Performing Organization: University of Notre Dame

Principal Investigator: Dr. James P. Thomas

Contract Number: N00014-93-1-0845

R & T Project Number: cor5247---01

ONR Scientific Officer: Dr. A. John Sedriks, Code 3312

EXECUTIVE SUMMARY

The focus of this research was on the development of a finite element code for solute transport and trapping in linear elastic mixtures for use in modeling the hydrogen transport process in metals undergoing hydrogen assisted cracking. Specific objectives included:

- 1.) Completion of the development of a solute transport and trapping model with coupling between the concentration, deformation, and thermal field variables and trapping at reversible and irreversible trap sites.
- 2.) Implementation of the above theory in a finite element code.
- **3.)** Calculation of the crack tip deformations and chemical state variables for some high strength steels under a variety of loading, environment, and material conditions.

A solute transport model has been developed for linear elastic mixtures with coupling between the deformation, concentration, and thermal variables and trapping at reversible and irreversible trap sites. A journal publication describing this model is in preparation¹.

Work on a 2-D finite element implementation and its application to the modeling of hydrogen transport in a cracking metal is ongoing. A 1-D code has been developed and tested on a variety of problems with known analytical solutions. The "code" consists of a Fortran "user element" subroutine for use with the ABAQUS² finite element program. Documentation of the 1-D user element subroutine is contained within this report.

Work on objective 3.) could not be started without a 2-D version of the finite element code. The 1-D code was used to model a variety of hydrogen transport problems with the objective of learning more about the fully coupled transport theory. We were able to "invent" a problem with a square-root singular stress $(\sigma_x \sim 1/\sqrt{x})$. This problem was used to gain preliminary insight into the hydrogen distribution problem in planar crack geometries. The problem consisted of subjecting a 4340 steel rod $(10 \ cm \times 1 \ cm^2)$ to a singular body force $(\sim 1/x^{3/2})$ resulting in the square root singular stress. The steady-state hydrogen concentrations and deformations were determined using the fully coupled theory, developed in this work, and classical stress assisted diffusion (SAD) theory.

The fully coupled predictions showed slightly higher hydrogen concentrations, a more severe singularity in the concentration, larger axial and dilatational strains, and larger axial displacements, all of which depended on the extent of hydrogen trapping. The mathematical solution for the concentration became multi-valued as the singularity was approached. It was shown that this "non-physical" result would be eliminated if the product of the density, the molar volume of hydrogen in the mixture, and the bulk modulus decrease linearly or better with increasing hydrogen concentration. These findings are documented in the report and in a conference paper included as Appendix E.

Development of a 2-D user element subroutine is ongoing. The 2-D code will initially be limited to simple linear elastic mixture behavior and equilibrium trapping. Extensions of the model to include non-equilibrium trapping effects and plastic crack tip deformations are planned. An effort is also being made to interface the ABAQUS code with our user element routines to the Patran³ Solid Geometry Modeling program for more convenient meshing of complex 2-D geometries and for displaying the finite element results.

¹ J. P. Thomas and P. Matic, "Solute Transport Modeling in Elastic Solids", Int. J. Engnr. Sci., in preparation.

² ABAQUS is a finite element code supported by HKS, Inc., Pawtucket, RI.

³ Patran is a solid geometry modeling program supported by PDA Engineering, Costa Mesa, CA.

TRANSPORT MODELING OF HYDROGEN IN METALS FOR APPLICATION TO HYDROGEN ASSISTED CRACKING

INTRODUCTION

Two fundamental questions naturally arise in modeling the influence of hydrogen on the crack growth rate of metals. Namely, how does hydrogen "enhance" the crack growth rate (CGR); and what is the relationship between the hydrogen distribution within the material and the corresponding "enhancement" in the CGR? Quantitative knowledge of the crack tip hydrogen distribution under service or laboratory test conditions is requisite to addressing these questions and will require: a) a hydrogen transport model that incorporates trapping and deformation-concentration coupling effects (governing equations); b) knowledge of the time dependent, non-uniform hydrogen distribution along the crack walls (boundary conditions); and c) a mathematical solution technique for the resulting system of non-linear equations.

Stress-assisted diffusion (SAD) theory [1,2] is commonly used to model hydrogen transport in cracking metals systems. This theory extends classical diffusion by adding a hydrostatic stress gradient term as a driving force for diffusive transport. Equilibrium trapping effects are included through the use of an effective diffusion coefficient.

The influence of hydrogen on the material deformation state is assumed to be negligible, and this uncouples the deformation equations from the concentration variable. Hydrostatic stresses determined from solutions to standard elasticity or plasticity problems are used with the SAD equation to solve for the resulting concentration. The steady-state hydrogen distribution for Mode I cracks in linear elastic materials with uniform hydrogen concentration along the boundaries is given by [2]:

$$c = c_0 exp \left(constant \times \frac{K}{\sqrt{r}} cos \frac{\theta}{2} \right)$$

where c_0 is the boundary concentration, K is the stress intensity factor, and r and θ are polar coordinates centered at the crack tip. Note that $c \to \infty$ as $r \to 0$ (the crack tip).

Transient and steady-state hydrogen distributions for a plastically deforming crack in iron have been obtained by Sofronis & McMeeking [3] using finite element methods. Their results show large, but finite, concentrations at the crack tip region, primarily in traps near the crack surface. They conclude that the crack tip hydrogen distribution is primarily determined by the creation of dislocation traps via plastic straining at the crack tip.

Damage models that attempt to link the crack tip hydrogen distribution to the fracture process have been reviewed in [4-6]. Applications to service cracking problems have met with some success, but the lack of information on the crack wall hydrogen distributions under service or laboratory conditions has limited the usefulness of these models.

The task of specifying the crack wall hydrogen distribution is difficult because of the complex nature of the interacting chemical, mechanical, and metallurgical processes operative during hydrogen assisted cracking of metals (Fig.1). In aqueous systems, the hydrogen production process is driven by the rapid and irreversible evolution of the chemically unstable "bare" surface at the crack tip to a more stable equilibrium "filmed" state. An electron flow is induced between the bare and filmed crack flank surfaces; net

anodic (dissolution/filming) reactions take place on the bare surface and net cathodic (hydrogen reduction) reactions take place on the filmed surfaces.

Adsorbed hydrogen, H_{ads} , can be produced on both the bare and filmed crack surfaces by: (1) the reduction of hydrogen ions in acidic environments; or (2) by the reduction of water in alkaline environments. The MH_{ads} species are then free to be absorbed by the transition reaction (a); or combine to form H_2 gas via: recombination (b1); or electrochemical desorption (b2). Reactions (a), (b1), and (b2) occur in parallel, but one of the two (b1) or (b2) reactions is usually dominant (Fig.2).

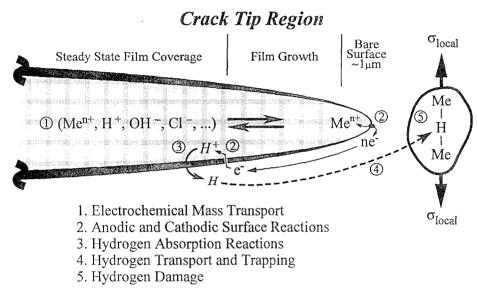


Figure 1: Schematic of the processes responsible for hydrogen assisted crack growth.

(1) Acidic:	$M + H^+ + e^- \Leftrightarrow MH_{ads}$
(2) Alkaline:	$M + H_2O + e^- \Leftrightarrow MH_{ads} + OH^-$

(a) Adsorption-Absorption:	$MH_{ads} \Leftrightarrow MH_{abs}$
(b1) Recombination:	$MH_{ads} + MH_{ads} \Leftrightarrow H_2 + 2M$
(b2) EC Desorption:	$MH_{ads} + H^+ + e^- \Leftrightarrow H_2 + M$

Figure 2: Summary of the hydrogen producing reactions.

The distribution of MH_{abs} along the crack surface is governed by the surface coverage of MH_{ads} and the kinetics of reaction (a) acting in parallel with reaction (b1) or (b2). These factors are influenced, in turn, by: the electrochemical environment at the crack tip region (e.g., the potential, pH, species concentrations, dissolved O_2 , etc.); the kinetics of the bare and filmed surface reactions; and the rate of transport of H_{abs} from the crack surface into the material.

Iyer and Pickering [7] have reviewed and modeled the kinetics of hydrogen evolution and entry in stress free metallic systems with homogeneous electrochemical conditions at the metal surface. Their model is used to quantify the rate constants associated with reactions (1) or (2) and (a) and (b1) or (b2) via analysis of experimental permeation data. Turnbull [8] has reviewed electrochemical conditions in cracks with particular emphasis on corrosion fatigue cracks of structural steels in sea water. Similarly, Beck [9] and Newman [10] have examined experimental techniques for characterizing bare (and filmed) surface reaction kinetics. The above models, data, and techniques, plus information concerning the rate controlling process during crack growth, will have to be used in an analysis of the mass transport process within the crack to develop realistic predictions of the MH_{abs} distribution.

We begin with a description of fully coupled transport and trapping theory. The use of the ABAQUS finite element "User Element" subroutines for solving 1-D problems is then outlined in full detail. This is followed by a description of three 1-D rod problems that have been studied. The results of these studies are reported next, followed by a discussion. And finally, some conclusions are drawn, and suggestions are made for further research.

THEORETICAL MODELING

This section begins with a brief description of the theory adopted for modeling coupled diffusion/trapping processes. Derivation of the finite element equations using the method of weighted residuals is described, followed by a description of analytical and finite element solutions to three simple steady-state hydrogen transport problems. While the motivation for this work is the modeling of hydrogen transport in cracking metal systems, the formulation presented here is generalized to generic solute-solid mixtures.

Three solute species are modeled in the analysis:

 $S_L \equiv$ interstitial or lattice species $S_R \equiv$ weak or moderately (reversibly) trapped species $S_I \equiv$ strongly (irreversibly) trapped species

Balance Equations

Balance equations for the mixture mass, the three solute species masses, mixture linear momentum and moment of momentum, mixture energy, and mixture entropy can be written. In this report, our modeling considerations will be restricted to isothermal linear elastic mixtures so that only the balance equations for the solute species mass and mixture linear momentum need be explicitly considered. Assuming negligible inertial effects, they are given by:

Mass:
$$\frac{\partial c_k}{\partial t} + \vec{\nabla} \cdot \vec{\mathcal{J}}_k = a_k \quad (k = L, R, or I)$$
 (1)

Linear Momentum:
$$\sigma_{ij,j} + F_i = 0$$
 $(i, j = x, y, or z)$ (2)

where: $c_k \equiv \text{mass fraction concentrations for } S_k \left[kg/kg \right].$ $\vec{\mathcal{J}}_k \equiv \text{concentration flux vectors for } S_k \left[kg/kg \cdot m/s \right].$ $a_k \equiv \text{mass supply rates for } S_k \left[kg/kg/s \right].$ $\sigma_{ij} \equiv \text{stress tensor } \left[N/m^2 \right].$ $F_i \equiv \text{the } i^{th} \text{ component of body force vector } \left[N/m^3 \right].$

Trapping Analysis

Expressions for the mass supply rates a_k in Eq. (1) are written in accordance with the trapping model of McNabb & Foster [11]:

Stoichiometry:
$$S_L \Leftrightarrow S_R$$
 and $S_L \Rightarrow S_I$ (3)

The stoichiometric relations require the supply terms sum to zero (i.e., $a_R + a_I = -a_L$).

Kinetics of Trapping:
$$a_R = k_R^f (1 - \theta_R) c_L - k_R^b \theta_R$$

$$a_I = k_I^f (1 - \theta_I) c_L - k_I^b \theta_I \approx k_I^f (1 - \theta_I) c_L$$

$$(4)$$

where: k_R^f , k_I^f , k_R^b , $k_I^b \equiv$ forward and backward trap rate constants for S_R and S_I [1/s]. θ_R and $\theta_I \equiv c_R/c_R^s$ and c_I/c_I^s , respectively, are the fraction of filled trap sites [1]. c_R^s and $c_I^s \equiv$ saturation mass fraction concentration of S_R and S_I [kg/kg].

The quantities k_R^f , k_I^f , k_R^b , k_I^b , c_R^s and c_I^s are related to trap site densities, probability of capture, etc. and can be quantified via experimental measurement (see, for example, [12-14]).

Significant simplifications are effected when the rate constants for trapping are much greater than those for diffusive transport. Trapping can then be modeled as a steady-state process (i.e., $a_L = a_R = a_I = 0$). This case is considered below.

SS Trapping:
$$c_R = \frac{c_R^s K_R c_L}{1 + K_R c_L} \approx c_R^s K_R c_L & \& c_I = c_I^s$$

$$K_R \equiv \frac{k_R^f}{k_R^b} = \exp\left(\frac{-\Delta E_b}{\mathbb{R} T}\right)$$
(5)

The total internal solute concentration is simply a linear function of $c_L(x_i, t)$:

$$c_{TOTAL}(x_i, t) = c_L(x_i, t) + c_R(x_i, t) + c_I(x_i, t) = (1 + c_R^s K_R)c_L(x_i, t) + c_I^s$$
(6)

The three versions of Eq. (1) (one for each species) can be summed to give a single equation by the following considerations. First, $\vec{\mathcal{J}}_R = \vec{\mathcal{J}}_I = 0$ because of the linear elastic material assumption which precludes trap site motion (e.g., dislocation motion during plastic deformation). Second, from Eq. (5a):

$$\frac{\partial c_R}{\partial t} \approx c_R^s K_R \frac{\partial c_L}{\partial t}$$
 and $\frac{\partial c_I}{\partial t} = 0$ (7)

Summation of the three balances results in the following single mass balance equation:

$$(1 + c_R^s K_R) \frac{\partial c_L}{\partial t} + \vec{\nabla} \cdot \vec{\mathcal{J}}_L = 0$$
 (8)

Constitutive Equations

Mass Flux:
$$\vec{\mathcal{J}}_L = -\frac{c_L D_L}{R_L T} \vec{\nabla} \mu_L$$
 Linear Onsager Relation (9)

where μ_L is a mass based chemical potential [15] defined by:

$$\mu_L \equiv \frac{\partial \psi}{\partial c_L} = \frac{1}{\mathcal{M}_L} \left(\mu_L^o(T) + \mathbb{R}T \ln(c_L) - \overline{V}_s ke \right) \qquad \left[\frac{J}{kg} \right]$$
 (10)

and: $D_L \equiv \text{diffusion coefficient for } S_L [m^2/s].$

 $R_L \equiv \text{gas constant for } S_L = \mathbb{R}/\mathcal{M}_L [J/kg \cdot {}^{\circ}K].$

 $\mathcal{M}_L \equiv \text{molecular mass of } S_L [kg/mol].$

 $T \equiv \text{temperature } [{}^{\circ}K].$

 $\psi \equiv \psi(c_L,\,c_R,\,c_I;\epsilon_{ij}\,;\,T)$ free energy per unit mixture mass [J/kg].

 $\mu_L^o(T) \equiv$ reference potential for S_L at temperature T[J/mol].

 $\overline{V}_s \equiv$ partial molar volume of solute in the metal $[m^3/mol\,S]$.

 $k \equiv \text{bulk modulus of elasticity } [N/m^2].$

 $e \equiv$ trace of the strain tensor (i.e., $e = \epsilon_{ii} = \epsilon_{11} + \epsilon_{22} + \epsilon_{33}$) [m/m].

The use of a mass based chemical potential simplifies the analysis of fully coupled deformation-diffusion processes because of the primary role played by mass in the deformation equations.

The resulting expression for concentration flux is given by:

$$\vec{\mathcal{J}}_L = -D_L \, \vec{\nabla} c_L + \frac{\overline{V}_s \, D_L}{\mathbb{R}T} \, k c_L \vec{\nabla} e \tag{11}$$

The constitutive equation for the stress consists of Hooke's law combined with a dilatational stress contribution due to changes in the total solute concentration:

Stress:
$$\sigma_{ij} \equiv \rho \, \frac{\partial \psi}{\partial \epsilon_{ij}} = \lambda \, e \, \delta_{ij} + 2G \epsilon_{ij} - 3k \, \alpha_s \, (1 + c_R^s K_R) \, \Delta c_L \, \delta_{ij}$$
 (12)

where: $\rho \equiv \text{mass density of the solid } [kg/m^3].$

 $\lambda \equiv \text{Lame' constant } [N/m^2].$

 $\delta_{ij} \equiv \text{Kronecker delta } (\delta_{ij} = 1 \text{ for } i = j \text{ and } 0 \text{ otherwise}).$

 $G \equiv \text{shear modulus } [N/m^2].$

 $\epsilon_{ij} \equiv \frac{1}{2} (u_{i,j} + u_{j,i})$ infinitesimal strain tensor [m/m].

 $u_i \equiv ext{the } i^{th} ext{ component of the displacement vector } [m].$

 $\alpha_s \equiv \text{linear expansion coefficient for internal solute} = \frac{1}{3} \frac{\rho}{\mathcal{M}_L} \overline{V}_s \left[m/m/\Delta c_s \right].$

 $\Delta c_L \equiv c_L - c_0$ = change in c_L from the reference level, c_0 .

 $(1 + c_R^s K_R) \Delta c_L \equiv$ change in c_{total} from the reference level.

The influence of the irreversibly trapped solute on the deformation of the solid has been ignored in Eq. (12). The assumption of equilibrium trapping implies that $c_I \to c_I^s$ immediately upon introducing solute into the solid. We have assumed, therefore, that $c_I = c_I^s$, and that all deformations are measured with respect to an initial uniform deformation arising from the presence of the irreversibly trapped solute species at its saturation concentration level.

The relationship between the chemical potential and stress (Eqs. (10) and (12)) is dictated by the thermodynamic reciprocity relationship:

$$\frac{\partial \mu_L}{\partial \epsilon_{ij}} = \frac{\partial (\sigma_{ij}/\rho)}{\partial c_L} \tag{13}$$

The equations of classical stress assisted diffusion violate this requirement by ignoring the concentration induced dilatational stresses.

Governing Equations

Combining the mass and momentum balance equations with the constitutive relations results in the following system of governing equations for transport:

Diffusion Equation:

$$\frac{\partial c_L}{\partial t} = D_{eff} \nabla^2 c_L - \frac{\overline{V}_s D_{eff}}{\mathbb{R} T} k \left(\vec{\nabla} c_L \cdot \vec{\nabla} e + c_L \nabla^2 e \right)$$
 (14)

Deformation Equations (i = 1, 2, 3):

$$(\lambda + G)\frac{\partial e}{\partial x_i} + G\nabla^2 u_i + F_i = 3k \,\alpha_s (1 + c_R^s K_R) \frac{\partial c_L}{\partial x_i}$$
(15)

where D_{eff} is an "effective" diffusion coefficient defined by: $D_{eff} \equiv D_L/(1+c_R^s K_R)$.

Equation (14) is identical to the SAD equations published in the literature with the exception of the $\nabla^2 e$ term which is identically zero when linear elastic material behavior is assumed (no coupling and $F_i = 0$). In the present formulation it is given by:

$$\nabla^2 e = \frac{3k \,\alpha_s}{\lambda + 2G} (1 + c_R^s K_R) \nabla^2 c_L \tag{16}$$

FINITE ELEMENT MODELING

Equations (14) and (15) form a system of non-linearly coupled partial differential equations that must be solved for c_L and u_i as functions of the space and time coordinates (x_i, t) . In developing the finite element equations, it is more convenient to work with the system of balance equations joined with appropriate constitutive equations. The 1-D form of the equations, with both "plane stress" and "plane strain" constitutive relations, is given by:

Diffusion:
$$\frac{\partial c_L}{\partial t} + \frac{\partial \mathcal{J}_L^*}{\partial x} = 0$$

$$\mathcal{J}_L^* \equiv -D_{eff} \frac{\partial c_L}{\partial x} + \frac{\overline{V}_s D_{eff}}{\mathbb{R} T} k c_L \frac{\partial e}{\partial x}$$

$$(17)$$

Deformation:
$$\frac{\partial \sigma_x}{\partial x} + F_x = 0 \tag{18}$$

Plane Stress Constitutive Relations: $(\sigma_y, \sigma_z, \sigma_{xy}, \sigma_{yz}, \sigma_{zx} = 0)$

$$\sigma_x = 3ke - 9k\alpha_s(1 + c_R^s K_R)\Delta c_L = E\epsilon_x - E\alpha_s(1 + c_R^s K_R)\Delta c_L \tag{19}$$

Plane Strain Constitutive Relations: $(\epsilon_y, \epsilon_z, \epsilon_{xy}, \epsilon_{yz}, \epsilon_{zx} = 0)$

$$\sigma_x = (\lambda + 2G)e - 3k\alpha_s(1 + c_R^s K_R)\Delta c_L$$

$$= \frac{E(1 - \nu)}{(1 - 2\nu)(1 + \nu)}\epsilon_x - \frac{E}{(1 - 2\nu)}\alpha_s(1 + c_R^s K_R)\Delta c_L$$
(20)

A single differential equation for c_L , under plane stress conditions, can be obtained using Eqs. (17) through (19):

$$\frac{\partial c_L}{\partial t} = \frac{\partial}{\partial x} \left[D_{eff} \left(1 - \frac{\rho \overline{V}_s^2 k}{\mathcal{M}_L \mathbb{R} T} (1 + c_R^s K_R) c_L \right) \frac{\partial c_L}{\partial x} + \frac{\overline{V}_s D_{eff}}{3 \mathbb{R} T} F_x c_L \right]$$
(21)

In the absence of body forces, this is a standard diffusion equation with a concentration dependent diffusion coefficient. Solutions to the steady-state problem, with and without body forces, can be obtained straightforwardly by integration; symbolic computation is recommended (see Appendix A).

Programming Simplifications

The following generalizations are made to the expressions for dilatational strain e and axial stress σ_x to simplify the subsequent finite element coding. They are obtained using Eqs. (19) and (20) and the definition for Δc_L :

$$e = A_1 \frac{\partial u}{\partial x} + A_2 c_L - A_3$$

$$\sigma_x = B_1 e - B_2 c_L + B_3$$
(22)

where A_i and B_i (i = 1, 2, or 3) are constants defined in the table below:

Constant	Plane Stress	Plane Strain
A_1	$(1-2\nu)$	1
A_2	$2(1+\nu)\alpha_s(1+c_r^sK_r)$	0
A_3	$A_2 \times c_0$	0
B_1	3k	$(\lambda + 2G)$
B_2	$9k\alpha_s(1+c_r^sK_r)$	$3k\alpha_s(1+c_r^sK_r)$
B_3	$B_2 \times c_0$	$B_2 \times c_0$

Table 1: Constants for the dilatational strain, e, and axial stress, σ_x , relations.

The elastic constants needed as input to the finite element program are the elastic modulus, E, and Poisson's ratio, ν . Conversions from bulk modulus, k, and $\lambda + 2G$ ($\lambda = \text{Lam\'e}$ constant and G = shear modulus) are given by:

$$k = \frac{E}{3(1 - 2\nu)}$$

$$\lambda + 2G = \frac{(1 - \nu)E}{(1 + \nu)(1 - 2\nu)}$$
(23)

The equations of classical stress assisted diffusion result by setting the constants A_2 , A_3 , B_2 , and B_3 equal to zero.

Formulation of the Finite Element Matrix Equations

Galerkin method of weighted residuals is used to formulate the finite element matrix equations. This procedure uses the interpolation functions as weights in the integral formulation given below [16]:

$$\int_{V_e} \left(\frac{\partial c_L}{\partial t} + \frac{\partial \mathcal{J}_L^*}{\partial x} \right) G_i \, dV_e = 0$$

$$\int_{V_e} \left(\frac{\partial \sigma_x}{\partial x} + F_x \right) H_j \, dV_e = 0$$
(24)

where: $G_i \equiv i^{th}$ concentration interpolation function, $(G_i = G_i(x))$.

 $H_j \equiv j^{th}$ displacement interpolation function, $(H_j = H_j(x))$.

 $dV_e \equiv$ differential element volume $= A_e dx$.

 $A_e \equiv \text{cross-sectional area} (A_e = A_e(x)).$

Integrating the second term of Eq.(24a) and the first term of Eq.(24b) by parts yields the coupled set of equations for a single finite element of length h ($0 \le x \le h$):

$$\int_{0}^{h} \left(\frac{\partial c_{L}}{\partial t} \{ G_{i}(x) \} - \mathcal{J}_{L}^{*} \left\{ \frac{\partial G_{i}(x)}{\partial x} \right\} \right) A_{e} dx = -A_{e} \mathcal{J}_{L}^{*} \left\{ G_{i}(x) \right\} \Big|_{0}^{h}$$

$$\int_{0}^{h} \left(-\sigma_{x} \left\{ \frac{\partial H_{j}(x)}{\partial x} \right\} + F_{x} \left\{ H_{j}(x) \right\} \right) A_{e} dx = -A_{e} \sigma_{x} \left\{ H_{j}(x) \right\} \Big|_{0}^{h}$$
(25)

The RHS term $A_e \mathcal{J}_L^*$ represents the axial solute influx through the element boundaries and $A_e \sigma_x$ represents the axial force applied at each end of the element.

<u>Interpolation Functions for the Concentration and Displacement</u>

The concentration is represented using a linear interpolation function, and the displacement is represented using a quadratic interpolation function:

$$c_{L} \equiv \sum_{i=1}^{2} G_{i}(x)c_{i}(t) = \lfloor G_{i}(x) \rfloor \{c_{i}(t)\} \qquad i = 1, 2$$

$$\frac{\partial c_{L}}{\partial x} \equiv \sum_{i=1}^{2} \frac{dG_{i}(x)}{dx}c_{i}(t) = \left\lfloor \frac{dG_{i}(x)}{dx} \right\rfloor \{c_{i}(t)\} \qquad i = 1, 2$$

$$\frac{\partial c_{L}}{\partial t} \equiv \sum_{i=1}^{2} G_{i}(x)\frac{dc_{i}(t)}{dt} = \lfloor G_{i}(x) \rfloor \left\{ \frac{dc_{i}(t)}{dt} \right\} \qquad i = 1, 2$$

$$u \equiv \sum_{j=1}^{3} H_{j}(x)u_{j}(t) = \lfloor H_{j}(x) \rfloor \{u_{j}(t)\} \qquad j = 1, 2, 3$$

$$\frac{\partial u}{\partial x} \equiv \sum_{j=1}^{3} \frac{dH_{j}(x)}{dx}u_{j}(t) = \left\lfloor \frac{dH_{j}(x)}{dx} \right\rfloor \{u_{j}(t)\} \qquad j = 1, 2, 3$$

$$\frac{\partial u}{\partial t} \equiv \sum_{i=1}^{3} H_{j}(x)\frac{du_{j}(t)}{dt} = \lfloor H_{j}(x) \rfloor \left\{ \frac{du_{j}(t)}{dt} \right\} \qquad j = 1, 2, 3$$

where $\lfloor \cdots \rfloor$ indicates a row matrix and $\{\cdots\}$ a column matrix. Substitution of these into Eqs. (22a,b) and (17b) for the dilatational strain, stress, and mass flux, respectively, give:

$$e = A_{1} \left\lfloor \frac{dH_{j}(x)}{dx} \right\rfloor \{u_{j}(t)\} + A_{2} \lfloor G_{i}(x) \rfloor \{c_{i}(t)\} - A_{3}$$

$$\sigma_{x} = B_{1}A_{1} \left\lfloor \frac{dH_{j}(x)}{dx} \right\rfloor \{u_{j}(t)\} + (B_{1}A_{2} - B_{2}) \lfloor G_{i}(x) \rfloor \{c_{i}(t)\} + (B_{3} - B_{1}A_{3})$$

$$\mathcal{J}_{L}^{*} = -D_{eff} \left\lfloor \frac{dG_{i}(x)}{dx} \right\rfloor \{c_{i}(t)\} + D_{eff} K_{1} \lfloor G_{i}(x) \rfloor \{c_{i}(t)\} \frac{\partial e}{\partial x}$$
(28)

Substituting Eqs.(28a,b,c) into Eq.(25a,b) and collecting terms in $\{c_i(t)\}$, $\{u_i(t)\}$, $\{\dot{c}_i(t)\}$, and $\{\dot{u}_i(t)\}$ yields the following matrix equation:

$$\begin{bmatrix}
[C_c] & [0] \\
[0] & [0]
\end{bmatrix}
\begin{cases}
\{\dot{c}_i\} \\
\{\dot{u}_j\}
\end{cases}
+
\begin{bmatrix}
[K_c] & [K_{cu}] \\
[K_{uc}] & [K_u]
\end{bmatrix}
\begin{Bmatrix}
\{c_i\} \\
\{u_j\}
\end{Bmatrix}
=
\begin{Bmatrix}
\{R_c\} \\
\{R_u\}
\end{Bmatrix}$$
(29)

where:

$$\{\dot{c}_i\} \equiv \frac{dc_i}{dt}$$

$$\{\dot{u}_i\} \equiv \frac{du_i}{dt}$$
(30)

$$[C_c] \equiv A_e \int_0^h \{G_i(x)\} \lfloor G_i(x) \rfloor dx \tag{31}$$

$$[K_{cu}] \equiv [0] \tag{32}$$

$$[K_{u}] \equiv A_{e} \int_{0}^{h} B_{1} A_{1} \left\{ \frac{dH_{j}(x)}{dx} \right\} \left\lfloor \frac{dH_{j}(x)}{dx} \right\rfloor dx$$

$$[K_{uc}] \equiv A_{e} \int_{0}^{h} (B_{1} A_{2} - B_{2}) \left\{ \frac{dH_{j}(x)}{dx} \right\} \left\lfloor G_{i}(x) \right\rfloor dx$$

$$[K_{c}] \equiv A_{e} \int_{0}^{h} D_{eff} \left(\left\{ \frac{dG_{i}(x)}{dx} \right\} \left\lfloor \frac{dG_{i}(x)}{dx} \right\rfloor - K_{1} \left\{ \frac{dG_{i}(x)}{dx} \right\} \frac{\partial e}{\partial x} \left\lfloor G_{i}(x) \right\rfloor \right) dx$$

$$\{R_{c}\} \equiv -A_{e} \mathcal{J}_{L}^{*} \left\{ G_{i}(x) \right\} \Big|_{0}^{h} \tag{33}$$

$$\{R_{u}\} \equiv A_{e} \int_{0}^{h} \left\{ H_{j}(x) \right\} F_{x} - (B_{3} - B_{1} A_{3}) \left\{ \frac{dH_{j}(x)}{dx} \right\} \right) dx + A_{e} \sigma_{x} \left\{ H_{j}(x) \right\} \Big|_{0}^{h}$$

and where the cross-sectional area, A_e , is assumed to be constant within the element.

The fact that $[K_{cu}]$ is zero might appear to imply that there is no coupling between concentration and deformation, but this is not the case. The $[K_c]$ contains the first derivative of the hydrostatic strain, $\partial e/\partial x$, which contains, in turn, the second derivative in the displacement, u (from Eq. (22a)):

$$\frac{\partial e}{\partial x} = A_1 \frac{\partial^2 u}{\partial x^2} + A_2 \frac{\partial c_L}{\partial x} \tag{34}$$

The presence of the second order derivative in u would normally require the use of C^1 continuous elements in order for u to satisfy element interface compatibility requirements [16]. To avoid this complication, the values of u and c_L from the previous time step are used to approximate $\partial e/\partial x$ for the current time calculations.

The integral expressions in Eqs. (31) through (33) use interpolation functions expressed in terms of the global coordinate, x. The code will be implemented using isoparametric coordinates⁴ which requires replacement of the functions $G_i(x)$ and $H_j(x)$ by an appropriate set of isoparametric interpolation functions.

Isoparametric Interpolation Functions

Standard linear and quadratic isoparametric interpolation functions, expressed in terms of the local coordinate r ($-1 \le r \le 1$) are given by [17]:

$$g_1(r) \equiv \frac{1}{2}(1-r), \quad g_2(r) \equiv \frac{1}{2}(1+r)$$

$$h_1(r) \equiv \frac{1}{2}(r^2-r), \quad h_2(r) \equiv (1-r^2), \quad h_3(r) \equiv \frac{1}{2}(r^2+r)$$
(35)

⁴ The transformation from global to local coordinates is *isoparametric* in displacement and *superparametric* in concentration. The term isoparametric will be used here to refer to the local coordinate parametrization of the element geometry which extends from -1 at the left-hand end of the element to +1 at the right-hand end.

For an element with the global node locations, x_i , the correspondence between the global coordinate, x, and the local coordinate, r, is given by the transformation:

$$x = \lfloor h_j(r) \rfloor \{x_j\} \tag{36}$$

Equations (31) through (33) can now be expressed in terms of the local r coordinates via the following substitutions:

$$\left\{ \frac{dH_{j}(x)}{dx} \right\} \mapsto \left\{ h_{j}(r) \right\}, \quad \left\{ G_{i}(x) \right\} \mapsto \left\{ g_{i}(r) \right\} \\
\left\{ \frac{dH_{j}(x)}{dx} \right\} \mapsto \left[J^{-1} \right] \left\{ \frac{dh_{j}(r)}{dr} \right\}, \quad \left\{ \frac{dG_{i}(x)}{dx} \right\} \mapsto \left[J^{-1} \right] \left\{ \frac{dg_{i}(r)}{dr} \right\} \\
\int_{0}^{h} dx \mapsto \int_{-1}^{+1} |J| dr$$
(37)

where:

$$[J] \equiv \left[\frac{dx}{dr} \right] = \left| \frac{dh_j(r)}{dr} \right| \{x_j\} = \left(r - \frac{1}{2} \right) x_1 - (2r) x_2 + \left(r + \frac{1}{2} \right) x_3$$
 (38)

is the "scalar" Jacobian transformation matrix, J, with determinant |J| = J, and inverse $[J^{-1}] = \frac{1}{J}$. Using Eqs.(36) through (38) in Eqs. (29), (30), and (33) yields the following relations:

$$[C_c] \equiv A_e \int_{-1}^{+1} \{g_i(r)\} \lfloor g_i(r) \rfloor J \, dr \tag{39}$$

$$[K_{cu}] \equiv [0]$$

$$[K_{u}] \equiv A_{e} \int_{-1}^{+1} B_{1} A_{1} \left\{ \frac{dh_{j}(r)}{dr} \right\} \left\lfloor \frac{dh_{j}(r)}{dr} \right\rfloor \frac{1}{J} dr$$

$$[K_{uc}] \equiv A_{e} \int_{-1}^{+1} (B_{1} A_{2} - B_{2}) \left\{ \frac{dh_{j}(r)}{dr} \right\} \lfloor g_{i}(r) \rfloor dr$$

$$[K_{c}] \equiv A_{e} \int_{-1}^{+1} D_{eff} \left(\left\{ \frac{dg_{i}(r)}{dr} \right\} \left\lfloor \frac{dg_{i}(r)}{dr} \right\rfloor - K_{1} \left\{ \frac{dg_{i}(r)}{dr} \right\} \frac{\partial e}{\partial r} \lfloor g_{i}(r) \rfloor \right) \frac{1}{J} dr$$

$$\{R_c\} \equiv -A_e \mathcal{J}_L^* \{g_i(r)\}\Big|_{-1}^{+1}$$

$$\{R_u\} \equiv A_e \int_{-1}^{+1} \left(\{h_j(r)\}F_x - (B_3 - B_1 A_3)\frac{1}{J} \left\{\frac{dh_j(r)}{dr}\right\}\right) J dr + A_e \sigma_x \{h_j(r)\}\Big|_{-1}^{+1}$$

$$(41)$$

and $\partial e/\partial x$ in $[K_c]$ has been replaced with $(\partial e/\partial r)/J$ via use of the chain-rule of differentiation.

Gauss-Legendre Numerical Integration

The above integrals are evaluated using a Gauss-Legendre numerical integration scheme. The variable r occurs as a 3rd degree quantity in the expression for $[K_{uc}]$. A two-

point integration using Gauss-point locations, $r_k = \pm 1/\sqrt{3}$, and Gauss weights, $W_k = 1.0$ is therefore selected for use in the numerical calculations [16,17]:

$$[C_c] \equiv \sum_{k=1}^{2} A_e W_k J(r_k) \{g_i(r_k)\} \lfloor g_i(r_k) \rfloor$$

$$(42)$$

$$[K_{cu}] \equiv [0]$$

$$[K_{u}] \equiv \sum_{k=1}^{2} \frac{A_{e}W_{k} B_{1} A_{1}}{J(r_{k})} \left\{ \frac{dh_{j}(r_{k})}{dr} \right\} \left\lfloor \frac{dh_{j}(r_{k})}{dr} \right\rfloor$$

$$[K_{uc}] \equiv \sum_{k=1}^{2} A_{e}W_{k} (B_{1} A_{2} - B_{2}) \left\{ \frac{dh_{j}(r_{k})}{dr} \right\} \left\lfloor g_{i}(r_{k}) \right\rfloor$$

$$[K_{c}] = \sum_{k=1}^{2} \frac{A_{e}W_{k} D_{eff}}{J(r_{k})} \left\{ \left\{ \frac{dg_{i}(r_{k})}{dr} \right\} \left\lfloor \frac{dg_{i}(r_{k})}{dr} \right\rfloor - K_{1} \frac{de}{dr} \left\{ \frac{dg_{i}(r_{k})}{dr} \right\} \left\lfloor g_{i}(r_{k}) \right\rfloor \right)$$

$$\{R_{c}\} \equiv -A_{e}\mathcal{J}_{L}^{*} \left\{ g_{i}(r) \right\} \Big|_{-1}^{+1}$$

$$\{R_{u}\} \equiv \sum_{k=1}^{2} A_{e}W_{k} \left(J(r_{k})F_{x} \{h_{j}(r_{k})\} - (B_{3} - B_{1}A_{3}) \left\{ \frac{dh_{j}(r_{k})}{dr} \right\} \right) + A_{e}\sigma_{x} \{h_{j}(r)\} \Big|_{-1}^{+1}$$

$$(44)$$

Finite Element Equations

The matrix finite element equations given by Eq. (29) are rewritten, following reference [16], in the form:

$$[C(v_{\theta})]\{\dot{v}\}_{\theta} + [K(v_{\theta})]\{v\}_{\theta} = \{R(t_{\theta})\}$$
(45)

where $\{v\}$ is the vector of nodal degrees of freedom and θ $(0 \le \theta \le 1)$ parameterizes the time integration scheme via the following definitions:

$$t_{\theta} \equiv t_n + \theta \Delta t$$

$$\{v\}_{\theta} \equiv (1 - \theta)\{v\}_n + \theta\{v\}_{n+1}$$

$$\{\dot{v}\}_{\theta} \equiv \frac{\{v\}_{n+1} - \{v\}_n}{\Delta t}$$

$$(46)$$

For $\theta = 0$ and a lumped capacitance matrix, the algorithm is explicit; for $\theta = \frac{1}{2}$, the algorithm is Crank-Nicolson; for $\theta = \frac{2}{3}$ the algorithm is Galerkin; and for $\theta = 1$, the algorithm is the backward difference.

The coupled temperature-displacement solver routine in ABAQUS is used to solve the fully coupled system of transport equations represented by Eq. (45). This particular ABAQUS routine uses the backward difference algorithm for time integration. Substituting $\theta=1$ into Eqs. (46) gives:

$$t_{\theta=1} = t_{n+1} = t_n + \Delta t$$

$$\{v\}_{\theta=1} = \{v\}_{n+1}$$

$$\{\dot{v}\}_{\theta=1} = \{\dot{v}\}_{n+1} = \frac{\{v\}_{n+1} - \{v\}_n}{\Delta t}$$

$$(47)$$

which, when substituted into Eq.(45) yields:

$$\left[\left[\mathcal{K}(v_{n+1}) \right] + \frac{\left[\mathcal{C}(v_{n+1}) \right]}{\Delta t} \right] \left\{ v \right\}_{n+1} = \frac{\left[\mathcal{C}(v_{n+1}) \right]}{\Delta t} \left\{ v \right\}_n + \left\{ \mathcal{R}(t_{n+1}) \right\}$$
(48)

If $[\mathcal{C}]$ and $[\mathcal{K}]$ are independent of the degrees of freedom $\{v\}$, then the problem is linear, and the equations may be solved directly for $\{v\}_{n+1}$ via standard linear systems solver routines. On the other hand, if $[\mathcal{C}]$ or $[\mathcal{K}]$ is a function of v, then Eq. (48) is non-linear and must be solved using more specialized techniques. One of the techniques used by ABAQUS is the Newton-Raphson method. It is described below.

Non-Linear Solutions via Newton-Raphson

Assume that $\{v\}_n$ is known and we wish to determine $\{v\}_{n+1}$. First, define the "residual" vector $\{f(v_{n+1})\}$:

$$\{f(v_{n+1})\} \equiv -\left[\left[\mathcal{K}(v_{n+1}) \right] + \frac{\left[\mathcal{C}(v_{n+1}) \right]}{\Delta t} \right] \{v\}_{n+1}$$

$$+ \frac{\left[\mathcal{C}(v_{n+1}) \right]}{\Delta t} \{v\}_n + \{\mathcal{R}(t_{n+1})\}$$
(49)

where $\{f(v_{n+1})\}=\{0\}$ for a $\{v\}_{n+1}$ which is a solution to Eq. (48). Defining $\{v\}_{n+1}^i$ as the i^{th} iterated approximation to the actual solution, $\{v\}_{n+1}$, leads to the following definition for the $(i+1)^{st}$ iterated approximation:

$$\{v\}_{n+1}^{i+1} \equiv \{v\}_{n+1}^{i} + \{\Delta v\}_{n+1}^{i+1} \tag{50}$$

where $\{\Delta v\}_{n+1}^{i+1}$ is a correction vector. This correction vector $\{\Delta v\}_{n+1}^{i+1}$ can be determined by expanding $\{f(v_{n+1})\}$ in a Taylor's series approximation about the point $\{v\}_{n+1}^{i}$, retaining only the zeroth and first order terms, and then setting the resulting expression equal to zero:

$$\left\{ f\left(v_{n+1}^{i+1}\right) \right\} \approx \left\{ f\left(v_{n+1}^{i}\right) \right\} + \left. \frac{\partial \left\{ f\left(v_{n+1}^{i}\right) \right\}}{\partial \left\{v\right\}_{n+1}^{i}} \right|_{\left\{v\right\}_{n+1}^{i}} \left\{ \Delta v \right\}_{n+1}^{i+1} = \left\{ 0 \right\}$$
 (51)

Now,
$$\frac{\partial \left\{ f\left(v_{n+1}^{i}\right)\right\}}{\partial \left\{v\right\}_{n+1}^{i}} = -\left[\left[\mathcal{K}\left(v_{n+1}^{i}\right)\right] + \frac{\left[\mathcal{C}\left(v_{n+1}^{i}\right)\right]}{\Delta t}\right] - \left[\left[\mathcal{K}'\left(v_{n+1}^{i}\right)\right] + \frac{\left[\mathcal{C}'\left(v_{n+1}^{i}\right)\right]}{\Delta t} - \frac{\left[\mathcal{C}''\left(v_{n+1}^{i}\right)\right]}{\Delta t}\right]$$

$$(52)$$

where we have defined:

$$\left[\mathcal{K}'(v_{n+1}^{i})\right] \equiv \left[\frac{\partial \left[\mathcal{K}(v_{n+1}^{i})\right]}{\partial \{v\}_{n+1}^{i}}\right] \{v\}_{n+1}^{i}
\left[\mathcal{C}'(v_{n+1}^{i})\right] \equiv \left[\frac{\partial \left[\mathcal{C}(v_{n+1}^{i})\right]}{\partial \{v\}_{n+1}^{i}}\right] \{v\}_{n+1}^{i}
\left[\mathcal{C}''(v_{n+1}^{i})\right] \equiv \left[\frac{\partial \left[\mathcal{C}(v_{n+1}^{i})\right]}{\partial \{v\}_{n+1}^{i}}\right] \{v\}_{n}^{i}$$
(53)

Now define:

$$\left[\Delta \mathcal{C}'(v_{n+1}^i)\right] \equiv \left[\mathcal{C}'(v_{n+1}^i)\right] - \left[\mathcal{C}''(v_{n+1}^i)\right] \tag{54}$$

then:

$$\frac{\partial \left\{ f\left(v_{n+1}^{i}\right)\right\}}{\partial \left\{v\right\}_{n+1}^{i}} = -\left[\left[\mathcal{K}\left(v_{n+1}^{i}\right)\right] + \frac{\left[\mathcal{C}\left(v_{n+1}^{i}\right)\right]}{\Delta t}\right] - \left[\left[\mathcal{K}'\left(v_{n+1}^{i}\right)\right] + \frac{\left[\Delta\mathcal{C}'\left(v_{n+1}^{i}\right)\right]}{\Delta t}\right] \tag{55}$$

A simplification used in the numerical computations is to assume that $[\mathcal{K}']$ and $[\Delta \mathcal{C}']$ are approximately zero. This results in significant computational savings but reduces the convergence speed for the interative solution. Eq. (55) defines the 5×5 "Jacobian" matrix denoted in ABAQUS by "AMATRX".

Re-ordering the Degrees of Freedom

The ordering of the degrees of freedom (dof) in the system matrix equation, Eq. (29) or (45), violates the ABAQUS convention which groups the degrees of freedom by node [18]. The current layout of [K] in Eq. (45) is given by:

$$\begin{bmatrix} [K_c] & [K_{cu}] \\ [K_{uc}] & [K_u] \end{bmatrix} = \begin{bmatrix} [K_c] & [0] \\ [K_{uc}] & [K_u] \end{bmatrix} = \begin{bmatrix} K_c^{11} & K_c^{12} & 0 & 0 & 0 \\ K_c^{21} & K_c^{22} & 0 & 0 & 0 \\ K_{uc}^{11} & K_{uc}^{12} & K_u^{11} & K_u^{12} & K_u^{13} \\ K_{uc}^{21} & K_{uc}^{22} & K_u^{21} & K_u^{22} & K_u^{23} \\ K_{uc}^{31} & K_{uc}^{32} & K_u^{31} & K_u^{32} & K_u^{33} \end{bmatrix}$$
(56)

a non-sparse block matrix. Rearranging the order of the dof's to conform with the ABAQUS convention requires:

$$\{v\} = \begin{cases} c_1 \\ u_1 \\ u_2 \\ c_3 \\ u_3 \end{cases}$$
 (57)

This reordering gives the new element stiffness matrix layout:

$$[\mathcal{K}] = \begin{bmatrix} K_c^{11} & 0 & 0 & K_c^{12} & 0 \\ K_{uc}^{11} & K_u^{11} & K_u^{12} & K_{uc}^{12} & K_u^{13} \\ K_{uc}^{21} & K_u^{21} & K_u^{22} & K_{uc}^{22} & K_u^{23} \\ K_c^{21} & 0 & 0 & K_c^{22} & 0 \\ K_{uc}^{31} & K_u^{31} & K_u^{32} & K_{uc}^{32} & K_u^{33} \end{bmatrix}$$

$$(58)$$

The capacitance matrix must also be altered to reflect the new ordering given by Eq. (57).

ABAQUS User Element Subroutines

ABAQUS executes a Fortran subroutine named UEL for each "user defined" finite element in the model. Current values for the: 1. material properties; 2. total and incremental nodal degrees of freedom; 3. time incrementation parameters; and 4. user-defined state variables (i.e., the dilatational strain and axial stress at the Gauss integration points; extrapolated axial stress at the element boundaries; the mass flux at the center of the element; and the Gauss point locations) are passed from the main ABAQUS code into the UEL. Depending on the exact stage in the time increment, the subroutine UEL must return some combination of the: "Jacobian" matrix (AMATRX); right-hand side vector (RHS); and updated values of the state variables.

From Eq.(51), we have:

$$\left. \frac{\partial \left\{ f\left(v_{n+1}^{i}\right) \right\}}{\partial \left\{v\right\}_{n+1}^{i}} \right|_{\left\{v\right\}_{n+1}^{i}} \left\{ \Delta v \right\}_{n+1}^{i+1} = -\left\{ f\left(v_{n+1}^{i}\right) \right\}$$
 (59)

or
$$AMATRX \left\{ \Delta v \right\}_{n+1}^{i+1} = -RHS \tag{60}$$

where
$$AMATRX \approx \frac{1}{\Delta t} \left[\mathcal{C}(v_{n+1}^i) \right] + \left[\mathcal{K}(v_{n+1}^i) \right]$$
 (61)

and
$$RHS = \{\mathcal{R}\} - [\mathcal{K}]\{v\}_{n+1}^{i} - \frac{1}{\Delta t}[\mathcal{C}]\{\Delta v\}_{n+1}^{i}$$
 (62)

On return from subroutine UEL, ABAQUS assembles the AMATRX and RHS from each element into the global matrix equation:

$$[AMATRX]_{G} \{ \Delta v \}_{n+1}^{i+1} = \{ RHS \}_{G}$$
(63)

and adds contributions from concentrated loads into the RHS array. ABAQUS solves Eq. (63) for the incremental correction $\{\Delta v\}_{n+1}^{i+1}$ using standard solution techniques. The solution process is repeated until the residual, that is, the $\{RHS\}_G$ vector, is smaller than some specified tolerance, at which point the solution is accepted as correct. The tolerances used with this UEL subroutine must typically be set in the ABAQUS input deck, since the residuals for the displacement and concentration variables generally show many orders of magnitude difference.

MODELING APPLICATIONS

A variety of one-dimensional hydrogen transport problems have been used to explore the differences between the fully coupled transport theory, described in this paper, and classical SAD theory. Analytical and finite element solutions for the steady-state distributions of: hydrogen, axial stress, axial and dilatational strains, and the axial displacement have been obtained for a $(10\,cm\times1.0\,cm^2)$ cylindrical rod of 4340 steel subjected to various boundary conditions and applied body forces. Plane stress conditions are assumed for all problems.

The boundary conditions consist of concentration or mass flux and displacement or load (stresses) at each end of the rod. The analytical solutions are obtained using the Maple symbolic computation program, and the numerical solutions are obtained using ABAQUS with our custom Fortran user element subroutine. The problems examined both analytically and numerically are summarized in Table 2. Other problems that have been examined analytically are documented in Appendix A.

Case #	Deformation Boundary Conditions		Diffusion Bour	idary Conditions	Body Force	# Elements
1a	u(LHS) = 0.0	$P_x(RHS) = 0.0$	$c_L(LHS) = 0.0$	$c_L(RHS) = 10^{-6}$	0	10
2a	u(LHS) = 0.0	u(RHS) = 0.0	$c_L(LHS) = 0.0$	$c_L(RHS) = 10^{-6}$	0	10
3 (LT)	u(LHS) = 0.0	$P_x(RHS) = 0.0$	$\mathcal{J}_L(LHS) = 0.0$	$c_L(RHS) = 10^{-7}$	$\frac{15\times10^6}{\sqrt{x^3}}$	200
3 (HT)	u(LHS) = 0.0	$P_x(RHS) = 0.0$	$\mathcal{J}_L(LHS) = 0.0$	$c_L(RHS) = 10^{-7}$	$\frac{15\times10^6}{\sqrt{x^3}}$	200

Table 2: Displacements, u, are specified in [m]; loads, P_x , in [N]; concentrations, c_L , in [gH/gFe]; mass flux, \mathcal{J}_L , in [m/s]; and body force, F_x , in $[N/m^3]$. Case 3a (LT) and (HT) correspond to low and high trapping $(1 + c_R^s K_R = 20 \text{ and } 500)$.

Case 1 and 2 problems were used to gain insight into the differences between the fully coupled theory, SAD theory, and classical diffusion theory. They also proved useful as benchmark problems for debugging and verifying the user element subroutine.

Case 3 was posed in order to better assess the influence of the hydrogen induced deformations, which are not accounted for in classical SAD theory, at a crack-like stress singularity. A square-root singular stress was introduced in the rod by subjecting it to the fictitious body force:

$$F_x = \frac{15 \times 10^6}{\sqrt{(x+\epsilon)^3}} \tag{64}$$

The variable ϵ is a small constant (2.0×10^{-18}) included to prevent inadvertent Fortran errors at x=0. Substituting this into Eq. (18) and integrating results in the square-root singular stress:

$$\sigma_x = \frac{30 \times 10^6}{\sqrt{x + \epsilon}} \tag{65}$$

The numerator of Eq. (65) is equivalent to a stress-field intensity factor, K, which means that we have adopted a stress-field K value of $30 \left[MPa\sqrt{m} \right]$ for these Case 3 problems.

The introduction of this singularity into the 1-D rod problem resulted in some interesting, but unexpected, behavior in the mathematical solution for the concentration as a

function of x. We found that the concentration becomes a multi-valued function of x as the singularity at x=0 is approached. The exact point at which the concentration becomes multi-valued is predictable and depends only on material constants. Various features of the analytical solution will be discussed in the following Results and Discussion sections, but as a consequence, we have had to modify the rod geometry in our Case 3 numerical analysis so that the left hand boundary starts at $x=4.0\times 10^{-5}[m]$ rather than x=0. This avoids the numerical problems associated with multi-valued concentrations.

The material properties used in the analytical and numerical analyses are given below in Table 3:

Property	Value
Mass Density, ρ	$7.8[g/cm^3]$
Temperature, T	293[K]
Lattice Diffusion Coefficient ¹ , D_L	$1 imes 10^{-5} \left[cm^2/s ight]$
Partial Molar Volume of Hydrogen ² , \overline{V}_H	$2.0[cm^3/mol]$
Saturation Trapping Concentration ³ , c_R^s	$2 imes10^{-12}$ to $10^{-4}\left[gH/gFe ight]$
Reversible Trap Binding Enthalpy ³ , H_B	3.3 to $30 \left[kJ/mol \right]$
$K_R = expig(rac{H_B}{RT}ig)$	4 to $2.2 imes 10^5$
Trapping Factor, $(1 + c_R^s K_R)$	20 and 500
Molecular Weight of Hydrogen, \mathcal{M}_L	$1.00797 \left[g/mol ight]$
Modulus of Elasticity, E	200[Gpa]
Poisson's Ratio, ν	0.3
Reference Concentration, c_0	$0\left[gH/gFe ight]$

Table 3: Material property values used in the Maple and finite element analyses.

RESULTS

The results are presented in two subsections. The first subsection corresponds with the Case 1a and 2a problems, and the second subsection corresponds with the Case 3 problem. Each subsection is further divided into parts describing the analytical and finite element solution procedures and description of results.

Case 1 & 2 Problems

Analytical solutions to the 1-D problem can be obtained by solving Eq. (21) for concentration. For the convenience of the reader, Eq. (21) is repeated below:

$$\frac{\partial c_L}{\partial t} = \frac{\partial}{\partial x} \left[D_{eff} \left(1 - \frac{\rho \overline{V}_s^2 k}{\mathcal{M}_L \mathbb{R} T} (1 + c_R^s K_R) c_L \right) \frac{\partial c_L}{\partial x} + \frac{\overline{V}_s D_{eff}}{3 \mathbb{R} T} F_x c_L \right]$$
(21)

¹ Approximate value taken for α -Fe from Figure 12.4 of reference [19].

² Approximate value taken from Section III.A.2 of reference [6].

³ Range of values obtained from Table I of Section III.B.1 of reference [6] with $H_B \leq 30 \, [kJ/mol]$.

This is a non-linear diffusion equation with a concentration dependent diffusion coefficient. Under steady state conditions the equation simplifies to:

$$\left(1 - \frac{\rho \overline{V}_s^2 k}{\mathcal{M}_L \mathbb{R} T} (1 + c_R^s K_R) c_L\right) \frac{dc_L}{dx} + \frac{\overline{V}_s}{3 \mathbb{R} T} F_x c_L = Constant$$
(66)

where the constant is proportional to the mass flux that obtains under steady-state conditions.

For Cases 1 & 2, the body force, F_x , is also taken to be equal to zero, so that the governing differential equation simplifies to:

$$\left(1 - \frac{\rho \overline{V}_s^2 k}{\mathcal{M}_L \mathbb{R} T} (1 + c_R^s K_R) c_L\right) \frac{dc_L}{dx} = Constant$$
(67)

This equation can be integrated to give $c_L(x)$, and once $c_L(x)$ is known, Eq. (18) and (22b) can be combined to give:

$$\frac{de}{dx} = \frac{B_2}{B_1} \frac{dc_L(x)}{dx} \tag{68}$$

This can be integrated using the known function $c_L(x)$ to give the dilatational strain, e, and the axial strain and axial stress can then be obtained directly from Eqs. (22a,b):

$$\epsilon_x = \frac{du}{dx} = \frac{e(x)}{A_1} - \frac{A_2}{A_1} c_L(x) + \frac{A_3}{A_1}$$
 (69)

$$\sigma_x = B_1 e(x) - B_2 c_L(x) + B_3$$

Finally, the displacement, u, can be obtained by integrating Eq. (69a). The constants of integration in the expressions for: $c_L(x)$, J_L , e(x), $\epsilon_x(x)$, $\sigma_x(x)$, and u(x) can be determined using the given boundary data.

The above steps were performed, in this work, using the Maple V symbolic computation program on a Sun SPARC10 workstation. The general results are given below:

$$c_L(x) = C_1 x + C_1 \sqrt{1 - C_2 x}$$
(70)

$$u(x) = C_3 x + (C_4 - C_5 x) \sqrt{1 - C_2 x} - C_4$$
(71)

$$e(x) = C_6 - C_7 \sqrt{1 - C_2 x} \tag{72}$$

$$\sigma_x = C_8 \tag{73}$$

$$\mathcal{J}_L = C_9 \tag{74}$$

where the C_i , $i \in \{1, ..., 9\}$ are constants that depend on the boundary conditions and material property values. Documentation of the Maple analyses is given in Appendix A.

For the ABAQUS finite element analyses, the $10 \ [cm]$ rod is discretized using ten elements of length $1 \ [cm]$. The initial concentration and displacement are specified as zero throughout the rod, and the boundary conditions are as indicated in Table 2, and in the figures below. A "coupled temperature-displacement; steady-state" ABAQUS analysis is performed with an initial time increment of $5 \times 10^{-3} \ [s]$, a total step time of $1.0 \ [s]$, and a maximum allowable increment of $0.5 \ [s]$. The residual magnitude for the concentration dof is significantly smaller than the residual for displacement dof. A separate convergence criterion for the concentration (temperature in ABAQUS) dof is therefore adopted. The initial time-average flux for convergence of the concentration dof is set at 5.0×10^{-21} (see the AUM [II;9.6.2-1]). The input decks for the Case 1a and 2a analyses are given in Appendix C.

Case 1a and 2a rod geometries and boundary conditions are shown in Figures 3 and 4. Figures 5 through 11 show plots of: $c_L(x)$; the difference between the linear concentration distribution that obtains from classical SAD and diffusion analysis and $c_L(x)$; e(x); u(x); and $\sigma_x(x)$. Keep in mind while examining these plots that e(x), $e_x(x)$, $e_x(x)$, and $e_x(x)$ are identically zero for the classical SAD or diffusion analyses.

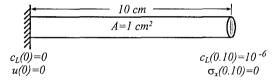


Figure 3: The rod geometry and boundary conditions for Case 1a.

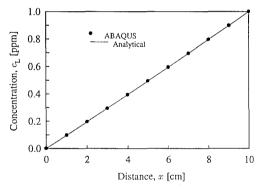


Figure 5: Analytical and finite element concentration predictions for Cases 1a and 2a using the fully coupled theory.

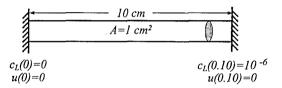


Figure 4: The rod geometry and boundary conditions for Case 2a.

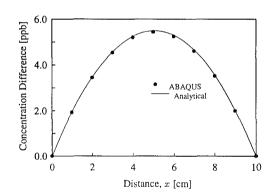


Figure 6: Difference between classical diffusion or SAD and the fully coupled concentrations (i.e., $10^{-7}x - c_L(x)$).

Figures 5 and 6 show the concentration and concentration difference, which are identical for the Case 1a and 2a problems, as a function of position along the rod. The concentration difference is defined as: $10^{-7}x - c_L(x)$ where $10^{-7}x$ is the concentration distribution that obtains for the SAD and classical diffusion theories. The difference is maximum at the center of the rod, but is very small at ≈ 5.5 parts per billion (ppb). The difference between the analytical and finite element predictions is less than 0.05 ppb.

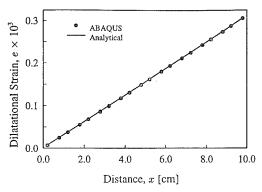


Figure 7: Fully coupled dilatational strain prediction for Case 1a. The FE predictions are given at Gauss integration points.

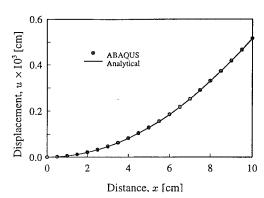


Figure 8: Fully coupled displacement prediction for Case 1a. The FE predictions are given at the nodes.

Figures 7 and 8 show the dilatational strain and the axial displacement experienced by the rod for the case 1a conditions. The rod is traction free, and there are no applied body forces, so by Eq. (18), the stress in the rod must be zero. By way of verification, we note that the FE analysis (not shown) also predicted a zero stress throughout the rod. The maximum dilatational strain and axial displacement occur at the free end of the rod with magnitudes of $\approx 310 \, [\mu m/m]$ and $\approx 5.2 \, [\mu m]$ respectively.

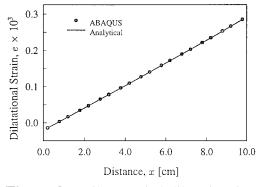


Figure 9: Fully coupled dilatational strain prediction for Case 2a. The FE predictions are given at the Gauss integration points.

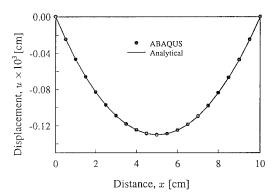


Figure 10: Fully coupled displacement prediction for Case 2a. The FE predictions are given at the nodes.

Figures 9 and 10 show the dilatational strain and axial displacement for Case 2a. The rod, stress free in the absence of hydrogen, tries to expand when the hydrogen is introduced. A uniform axial compressive stress of $\approx 10.4 \, [MPa]$ (see Figure 11) is introduced because of the zero displacement boundary restraints at the walls. The dilatational strain is positive and maximum at the RH boundary of the rod, and small, but negative at the LH boundary. The axial displacement is negative throughout the rod indicating a leftward movement of the rod material. The maximum displacement of $\approx -1.3 \, [\mu m]$ occurs at the center of the rod. Recalling the definition of axial strain, $\epsilon_x = du/dx$, we see that it is negative in the left half of the bar, positive in the right half, and zero in the middle. The maximum strain ($\approx \pm 50 \, [\mu \epsilon]$) occurs at each end of the rod (see the figure on page A-2a-14 of Appendix A).

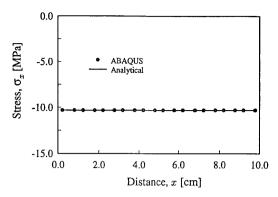


Figure 11: Fully coupled axial stress prediction for Case 2a. The FE predictions are given at the Gauss integration points.

Case 3 Problems

The governing differential equation for the Case 3 problem is obtained from Eq. (66) by substituting in for the applied body force, F_x :

$$\left(1 - \frac{\rho \overline{V}_s^2 k}{\mathcal{M}_L \mathbb{R} T} (1 + c_R^s K_R) c_L\right) \frac{dc_L}{dx} + \frac{\overline{V}_s \left(15 \times 10^6\right)}{3 \mathbb{R} T \sqrt{x^3}} c_L = Constant \tag{75}$$

Analytical solutions to this equation are obviously more difficult to obtain than in the previous two cases.

The LHS of Eq. (75) is directly proportional to the mass flux \mathcal{J}_L ; the mass flux is therefore constant throughout the rod. The Case 3 problem under study imposes a zero mass flux boundary condition on the LH boundary. The constant in Eq. (75) can therefore be set equal to zero, leading to the following governing equation:

$$\left(1 - \frac{\rho \overline{V}_s^2 k}{\mathcal{M}_L \mathbb{R} T} (1 + c_R^s K_R) c_L\right) \frac{dc_L}{dx} + \frac{\overline{V}_s \left(15 \times 10^6\right)}{3 \mathbb{R} T \sqrt{x^3}} c_L = 0$$
(76)

This must be solved for c_L as a function of x (i.e., for $c_L(x)$).

An interesting observation regarding Eq. (76) can be made. As $c_L(x)$ increases from some vanishingly small value, the coefficient of the derivative term will go from positive to negative, passing through zero on the way. When the coefficient equals zero, the derivative term drops out and we are left with the equation:

$$\frac{\overline{V}_s \left(15 \times 10^6\right)}{3 \,\mathbb{R} \, T \,\sqrt{x^3}} \, c_L = 0 \tag{77}$$

This equation cannot be satisfied because c_L is not zero, x is finite-valued, and the rest of the variables are either non-zero constants or material properties. A mathematical solution to the problem can only be obtained by letting $dc_L/dx \to \infty$ as:

$$c_L(x) \to \frac{\mathcal{M}_L \mathbb{R} T}{\rho \overline{V}_s^2 k (1 + c_R^s K_R)}$$
(78)

Allowing this results in multi-valued concentration solutions to Eq. (76); Eq. (78) defines the point at which the solution becomes multi-valued. Since multi-valued concentrations are not physically realistic, the assumptions made in the derivation of the fully coupled transport equations must be re-examined. This is done in the following Discussion section.

The solution to Eq. (76) is obtained by integration using Maple (see Appendix A):

$$c_L(x) = -C_{10} W \left(-\frac{exp\left(-C_{11} + \frac{C_{12}}{\sqrt{x}}\right)}{C_{10}} \right)$$
 (79)

W(x) is Lambert's W function⁴, and the C_i , $i \in \{10, 11, 12\}$ are constants related to the material properties and boundary conditions. This equation is multi-valued in x, but it can be inverted to give x as a single-valued function of c_L :

$$x(c_L) = \frac{(C_{12})^2}{\left[\frac{c_L}{C_{10}} - \ln(c_L) - C_{11}\right]^2}$$
(80)

The stress field is obtained by integrating Eq. (18) with the applied body force substituted in for F_x , yielding:

$$\sigma_x(x) = \frac{30 \times 10^6}{\sqrt{x}} \tag{81}$$

The dilatational strain can be written in terms of σ_x and c_L using Eq. (22b):

$$e(x) = \frac{1}{B_1}(\sigma_x(x) + B_2 c_L(x) - B_3)$$
(82)

Similar operations can be performed on Eq. (22a) to determine du/dx which can then be integrated (analytical integration may not be possible) to obtain the displacement, u(x).

The ABAQUS analysis for Case 3 requires significantly smaller elements than the Case 1 and 2 analyses, particularly near the singularity. The rod is discretized into 200 elements of length $l_i = 0.104/0.94^{i-1} \, [\mu m]$ where the first and smallest element, $l_1 = 0.104 \, [\mu m]$, is placed at the left hand end of the bar, $x = 4 \times 10^{-3} \, [cm]$. The 0.94 factor in the denominator is known as the element "bias" in ABAQUS. A "coupled temperature-displacement; steady-state" analysis is used with an initial time increment of $1.0 \times 10^{-4} \, [s]$, a total time of $1.0 \, [s]$, and a maximum time increment of $5.0 \times 10^{-3} \, [s]$. The initial displacements in the rod are taken as zero, but an initial uniform concentration equal to the RH boundary concentration of $1 \times 10^{-7} \, [g \, H/g \, Fe]$ is assumed. To match this initial condition in the analysis, the RH boundary concentration is applied as a step rather than a

⁴ Lambert's W function satisfies the equation: $W(x) \times exp(W(x)) = x$. Additional information can be found using the interactive Maple V help program.

ramp function. Providing an initial concentration resulted in a more rapid convergence of the solution to the steady-state values. The residual magnitude for the concentration dof is significantly smaller than the residual for displacement dof. A separate convergence criteria for the concentration (temperature in ABAQUS) dof was therefore adopted. The starting time-average flux for convergence of the concentration dof is set at 5.0×10^{-21} (see the AUM [II;9.6.2-1]). The input deck for the Case 3 analysis is given in Appendix C.

The Case 3 rod geometry and boundary conditions are shown in Figure 12. Figures 13 through 17 show: $c_L(x)$; $ln(c_L(x)) - constant$ which illustrates the different concentration singularities; e(x); u(x); and $\sigma_x(x)$. Plots are given for the fully coupled theory under low and high trapping conditions, for SAD theory, and in many cases, for $F_x = 0$. The finite element results are plotted for every 20th element.

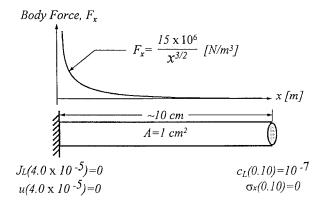


Figure 12: The rod geometry, boundary conditions, and applied body force for the Case 3 problem.

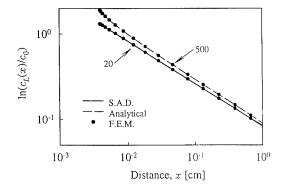


Figure 14: Concentration singularities. The SAD & LT-20 curves are coincident.

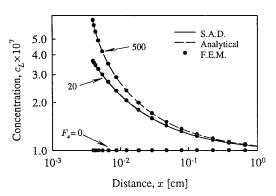


Figure 13: Concentration predictions for Case 3. The SAD results lie under the LT (20) curve of the fully coupled theory.

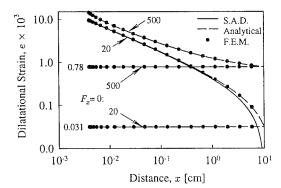


Figure 15: Dilatational strain for Case 3 using the fully coupled and SAD theories.

Figures 13 and 14 show the predicted concentrations for the Case 3 problems using fully coupled and SAD theories. The SAD predictions are made using the classic expression:

$$c_L(x) = c_0 \exp\left(\frac{\overline{V}_s}{\mathbb{R}T} \times \frac{1}{3} \times \frac{30 \times 10^6}{\sqrt{x}}\right)$$
 (83)

The fully coupled model predicts slightly larger concentrations than the SAD model, with increasing differences as the singularity is approached and as the degree of trapping increases. Figure 14 shows that the singularity for the fully coupled concentration is more severe than the $exp(1/\sqrt{x})$ singularity of the SAD model, at least for the high trapping case.

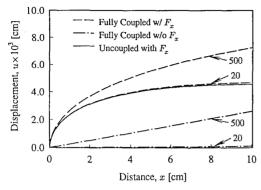


Figure 16: Nodal displacement curves for the FE analysis of Case 3 for the fully coupled and SAD theories.

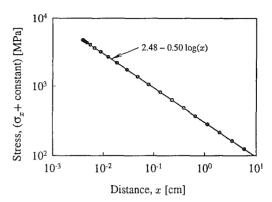


Figure 17: Gauss point stresses with a least squares line fit showing the required straight line behavior with a -1/2 slope.

Figures 15 and 16 illustrate the dilatational strains and nodal displacement curves. As with Cases 1 and 2, the fully coupled model predicts larger displacements and dilatational strains throughout the rod. Again, the difference between the fully coupled and SAD model predictions increase with the degree of trapping. The zero-body force strains and displacements are also shown for comparison with the zero valued strains and displacements that obtain for the SAD and classical diffusion model. Figure 17 shows the FE calculated Gauss point stresses; these match the expected behavior.

DISCUSSION

The above three rod problems illustrate the differences between the fully coupled and classical SAD theory under steady-state conditions (additional results can be found in Appendix A). The results show that the concentration differences are generally small, but grow in the region very near to the singularity. The deformation differences are a bit more pronounced, especially in Cases 1 and 2 where the boundary and body force loadings are absent. All of the fully coupled results are dependent on the degree of trapping.

The results also showed that accurate finite element solutions are possible for the fully coupled transport theory using ABAQUS with custom user element subroutines. The experience gained during the development and application of this 1-D user element routine is proving to be very useful in the ongoing development of the 2-D user element subroutine.

The dependence of the concentration and deformation distributions on the trapped hydrogen may be useful in the development of new "Gorsky effect" experiments for trapping characterization. Perhaps the time dependent displacement at the free end of a cantilevered rod can be related to some transport or trapping parameter of interest. Transient Case 1 type problems are probably be most useful in this regard.

Regarding the multi-valued concentrations that appear in the mathematical analysis of the Case 3 problem, the values of c_L satisfying Eq. (78) for the 4340 steel considered are: 2.3×10^{-5} and 9.3×10^{-7} [g H/g Fe] for the low (LT) and high (HT) trapping conditions, respectively. The total internal hydrogen, in mass fraction concentration units⁵, is given by Eq. (6): $c_{total} \approx 4.7 \times 10^{-4} + c_I^s$ [g H/g Fe]. Now, this is a very large concentration compared with the solubility of hydrogen in pure iron at room temperature and 1 [atm] pressure ($c_{total} \approx 2 \times 10^{-9}$ [g H/g Fe] or 1×10^{-7} [H/Fe]). It leads us to question the assumptions made regarding the magnitude of the concentration in the development of the fully coupled model. The three major assumptions include: 1.) "infinite" dilution of the total hydrogen in the mixture; 2.) ideal behavior; and 3.) material properties that are independent of the hydrogen concentration level.

The magnitude of the *lattice* concentrations at the critical point are still much less than one. It is unlikely, therefore, that the assumption of infinite dilution is playing any role in this particular situation. Extension of the model to include finite concentrations, while still retaining the ideal solution assumption, can be made by adopting a "reduced" chemical potential [15], which takes into account the blocking of interstitial sites in the neighborhood of a hydrogen atom⁶. Reduced chemical potentials will be adopted in a future version of the theory assuming that the multi-valued solutions for the concentration can be eliminated.

If we maintain the infinite dilution assumption, then non-ideality cannot play a role because an infinitely dilute mixture is by definition, ideal. Non-ideal effects are only possible in finite dilution mixtures [15], and the use of non-ideal expressions in classical SAD modeling is almost non-existent. Kirchheim and Hirth [20] have proposed a first order extension which accounts for H-H interactions. The work by Fukai [21] and coworkers on predicting hydrogen solubility in metals under very high pressures may also be of use in extending our model to include non-ideal effects. The question still remains; will the adoption of a non-ideal chemical potential result in governing equations with single-valued concentrations? We do not presently have an answer for this question; it remains to be investigated in the future.

The last possibility involves the assumption of constant valued material parameters. If the RHS of Eq. (78) were to increase at least linearly with c_L , through some concentration dependence of the material parameters, then the multi-valued solutions will not occur. A linear decrease in ρ , or k, or a square-root decrease in \overline{V}_s , with increasing c_L levels would satisfy this requirement. Experimental evidence for these decreases in 4340 steel do not appear to be available in the literature. There is evidence, though, for decreases in \overline{V}_s at large c_L values for various other alloys (see, for example, the article by Peisl in [19; pp. 53-74]; or Fukai [21; pp. 95-100]. This possibility will also have to be investigated in the future.

There remains one final aspect of this multi-valued solution dilemma that needs to be discussed. That is the validity of square-root stress singularities in the fully coupled theory. Remember that this singular body force was artificially introduced in order to get the square-root singular stress. We have not yet determined whether square-root singular stresses will naturally occur at the tip of a crack in the fully coupled theory. The 2-D finite element model under development should shed some light on this issue.

⁶ Reduced chemical potentials are also referred to as the "Fermi-Dirac" potentials [20-22].

⁵ Multiplication of c_{total} by $\mathcal{M}_{Fe}/\mathcal{M}_H \approx 55$ gives the mole fraction concentration, $x_{total} = 2.6 \times 10^{-2} + 55 \, c_I^s$.

SUMMARY AND FUTURE RESEARCH

The focus of this research was on the development of a finite element code for coupled hydrogen transport and trapping in linear elastic metals for use in modeling hydrogen assisted cracking processes. A fully coupled solute transport model was developed; a 1-D version of the model was implemented in a finite element code via a custom Fortran "user element" subroutine for use with the ABAQUS finite element program. A series of three simple 1-D problems were posed to develop an understanding of the fully coupled transport theory, and for verifying the accuracy and coding of the 1-D user element subroutine. Steady-state solutions for the fully coupled theory, and for classical diffusion and stress-assisted diffusion theories, were obtained analytically and numerically using the Maple symbolic computation program and ABAQUS. Differences in the predicted concentrations and deformations between the various theories were observed.

One of the problems incorporated a square-root singular stress as a "1-D analog" of the hydrogen transport problem in planar crack geometries. The fully coupled predictions showed slightly higher hydrogen concentrations, a more severe singularity in the concentration, larger axial and dilatational strains, and larger axial displacements, all of which depended on the extent of hydrogen trapping. The results indicated that the hydrogen induced deformations, present only in the fully coupled theory, were more influential with regard to the displacements and strains.

Development of a 2-D user element subroutine is ongoing. The initial versions will be limited in scope to simple linear elastic mixture behavior and equilibrium trapping at reversible and irreversible trap sites. Rectilinear isoparametric 8-node displacement/4-node concentration interpolation functions will be adopted. Extensions of the model to include non-equilibrium trapping effects and plastic crack tip deformations are planned. An effort is also being made to interface the ABAQUS code with our user element routines to the Patran Solid Geometry Modeling program. This will provide us with a convenient means of meshing complex 2-D geometries and manipulating and displaying the finite element results.

After the 2-D code has been developed, it will be used to calculate deformations and concentrations in the crack tip region of a metal with uniform hydrogen concentrations imposed along the crack walls. Steady-state SAD solutions for this idealized problem are available for comparison. The results will provide definitive answers on the importance of the hydrogen induced deformations in crack tip modeling.

A longer term goal is the use of the 2-D code to help establish an energy-based parameter that characterizes the *driving force* for crack growth in the presence of a deleterious environment. This parameter, the *free energy release rate* \mathcal{F} , will generalize the classical strain energy release rate \mathcal{G} to include the hydrogen induced deformation energy and the "free" chemical energy. It is defined by:

$$\mathcal{F} \coloneqq \frac{\Delta(\text{Externally Supplied Work} - \text{Specific Free Energy of the Hydrogen-Metal Mixture})}{\Delta(\text{Crack Length})}$$

The use of $\mathcal F$ as a driving force for crack growth in environmentally assisted cracking appears to be a novel extension of the classical fracture mechanics concept. The advantage of $\mathcal F$ over the stress intensity factor K (or ΔK) approach will be its ability to account for loading, environment, and material effects on the driving force for crack growth in terms of a single variable. The concept will be applicable many material-environment crack systems of

technological interest including: hydrogen assisted cracking of metals, high temperature oxidation cracking of superalloys, and moisture induced cracking of organic composites.

The magnitude of \mathcal{F} in any environmental cracking situation will have to be determined by an analysis of the deformation and diffusion/trapping processes operative in the crack tip region. More specifically, the deformation state (stresses and strains) and chemical state (potentials and concentrations for each solute species) of the mixture will have to be determined as a function of the loading (e.g., σ or $\Delta \sigma$, R, f, waveform, etc.), environment (e.g., phase, species, concentrations, T, pH, potentials, partial pressures, etc.) and material (e.g., elastic moduli, trapping parameters, solubility, diffusion coefficient, etc.) parameters. This information will be obtained using: a.) the fully coupled theory to model the deformation-diffusion-trapping processes occurring in the crack tip region (governing equations); b.) an experimentally determined distribution of absorbed species along the crack walls (boundary conditions); and c.) a method for solving the resulting mathematical equations (the FE code).

Our initial efforts will be focused on the application of this concept to hydrogen assisted cracking of high strength metals. This system is ideal in the sense that it shows large environmental effects, is important in many applications, and is supported by an extensive data base. It also avoids the complications of large scale crack tip plasticity. Validation of the concept will require demonstration of a unique correlation between the crack growth rate and \mathcal{F} (or $\Delta \mathcal{F}$) for a variety of materials subject to a variety of loading, environment, and material conditions.

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APPENDIX A: Analytical Solutions Using Maple

This Appendix contains the Maple analyses of the three steady-state rod transport problems studied in this work. All analyses were performed using Maple V, Release 3, on a Sun SPARC 10 workstation. The specific cases analyzed are summarized in the tables below:

Case #	Deformation Boundary Conditions		Diffusion Bour	$c_R^s K_R$	Page	
1a	u(LHS) = 0.0	$P_x(RHS) = 0.0$	$c_L(LHS)=0.0$	$c_L(RHS) = 10^{-6}$	19	A-1a-1
1b	u(LHS) = 0.0	$P_x(RHS) = 0.0$	$c_L(LHS) = 0.0$	$c_L(RHS) = 10^{-7}$	19	A-1b-1
1c	u(LHS) = 0.0	$P_x(RHS) = 0.0$	$c_L(LHS) = 0.0$	$c_L(RHS) = 10^{-7}$	499	A-1c-1
1d	u(LHS) = 0.0	$P_x(RHS) = 0.0$	$\mathcal{J}_L(LHS)=0.0$	$c_L(RHS) = 10^{-7}$	19	A-1d-1
1e	u(LHS) = 0.0	$P_x(RHS) = 0.0$	$\mathcal{J}_L(LHS) = 0.0$	$c_L(RHS) = 10^{-7}$	499	A-1e-1

Table A-1: Case summary for rod problem #1. Displacements, u, are specified in [m]; loads, P_x , in [N]; concentrations, c_L , in [kg H/kg Fe]; and mass flux, \mathcal{J}_L , in [m/s].

Case #	Deformation Boundary Conditions		Diffusion Bour	$c_R^s K_R$	Page	
2a	u(LHS) = 0.0	u(RHS) = 0.0	$c_L(LHS) = 0.0$	$c_L(RHS) = 10^{-6}$	19	A-2a-1
2b	u(LHS) = 0.0	u(RHS)=0.0	$c_L(LHS) = 0.0$	$c_L(RHS) = 10^{-7}$	19	A-2b-1
2c	u(LHS) = 0.0	u(RHS) = 0.0	$c_L(LHS) = 0.0$	$c_L(RHS) = 10^{-7}$	499	A-2c-1
2d	u(LHS) = 0.0	u(RHS) = 0.0	$\mathcal{J}_L(LHS)=0.0$	$c_L(RHS) = 10^{-7}$	19	A-2d-1
2e	u(LHS) = 0.0	u(RHS) = 0.0	$\mathcal{J}_L(LHS)=0.0$	$c_L(RHS) = 10^{-7}$	499	A-2e-1

Table A-2: Summary for rod problem #2. Displacements, u, are specified in [m]; concentrations, c_L , in [kg H/kg Fe]; and mass flux, \mathcal{J}_L , in [m/s].

Case #	Deformation Boundary Conditions		Diffusion Boundary Conditions		$c_R^s K_R$	Body Force	Page
3a	u(LHS) = 0.0	$P_x(RHS) = 0.0$	$\mathcal{J}_L(LHS) = 0.0$	$c_L(RHS) = 10^{-7}$	19	$\frac{15 \times 10^6}{\sqrt{x^3}}$	A-3a-1
3a	u(LHS) = 0.0	$P_x(RHS) = 0.0$	$\mathcal{J}_L(LHS) = 0.0$	$c_L(RHS) = 10^{-7}$	499	$\frac{15\times10^6}{\sqrt{x^3}}$	A-3a-1

Table A-3: Summary for rod problem #3. The rod length in used in this exact analysis is 10 [cm], in distinction with the rod length used in the finite element analysis. Displacements, u, are specified in [m]; loads, P_x , in [N]; concentrations, c_L , in [kg H/kg Fe]; mass flux, \mathcal{J}_L , in [m/s]; and body force, F_x , in $[N/m^3]$.

1. MAPLE PROGRAM DIRECTORY AND FILE NAME: /afs/nd.edu/user4/jthomas5/Maple/Transport/1DSSsol#la.ms

2. ORIGINATOR:
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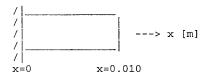
3. HISTORY:

Written: June-1994

Latest Revision: 13-Feb-1995

4. PROGRAM DESCRIPTION:

One-dimensional, steady-state analysis of the fully coupled solute transport and trapping equations using "plane stress" constitutive equations for stress. The particular problem solved is shown below:



Deformation variable boundary conditions: u(0)=0, S(0.10)=0Concentration variable boundary conditions: c(0)=0, c(0.10)=1e-6Reference concentration level: c0=0.0Trapping parameter: traps=19 (low trapping)

The equations used in the analysis are:

- 1. dJ/dx=0 (steady-state diffusion equation)
- 2. J=Ds*(K1*c*de/dx-dc/dx) (mass flux)
- 3. dS/dx=0 (steady-state deformation equation with zero body forces)
- 4. S=B1*e-B2*c+B3 (axial stress)
- 5. de/dx-K2*dc/dx=0 (dilational strain gradient)
- 6. e=A1*du/dx+A2*c-A3 (dilatational strain)

The constants used in the plane stress analysis are defined below:

Ds:=lattice diffusivity [m^2/sec]

Delc:= c-c0 [1]

K1 := (Vs*K) / (R*T)Vs:= partial molar volume of solute [m^3/mol solute] $K:= bulk modulus=Em/(3*(1-2*nu)) [N/m^2]$ Em: = Modulus of Elasticity [N/m^2] nu:= Poisson's ratio [1] R:= universal gas constant=8.31432 [J/mol-K] T:= temperature [K] B1:=3*K B2:=9*K*alpha*(1+traps)B3:=B2*co traps:= Csr*Kr=trapping constant [1] Csr:= saturation trap concentration [1] Kr:= equilibrium trapping constant [1] c0:= reference solute concentration [1] K2:=3*alpha*(1+traps) alpha:= (rho*Vs)/(3*MWs)=solute concentration expansion coefficient [m/m/Delc] rho:= mass density of the solid [kg/m^3] MWs:= molecular weight of the solute [kg/mol]

A1:=1-2*nu A2:=2*(1+nu)*alpha*(1+traps) A3:=A2*c0

5. REQUIRED INPUTS:

Ds:=lattice diffusivity [m^2/sec]
Vs:= partial molar volume of solute [m^3/mol solute]
traps:= Csr*Kr=trapping constant [1]
c0:= reference solute concentration [1]
MWs:= molecular weight of the solute [kg/mol]

Em:= Modulus of Elasticity [Pa]
nu:= Poisson's ratio [1]
rho:= mass density of the solid [kg/m^3]

T:= temperature [K]

6. SYMBOLIC ANALYSIS:

> Digits:=trunc(evalhf(Digits));

$$Digits := 15$$

> J:=Ds*(K1*K2*c(x)-1)*diff(c(x),x);

$$J := Ds (K1 \ K2 \ c(x) - 1) \left(\frac{\partial}{\partial x} c(x) \right)$$

> deqn:=diff(J,x)=0;

$$deqn := Ds \ K1 \ K2 \left(\frac{\partial}{\partial x} c(x)\right)^2 + Ds \left(K1 \ K2 \ c(x) - 1\right) \left(\frac{\partial^2}{\partial x^2} c(x)\right) = 0$$

> csoln:=dsolve(deqn,c(x),explicit);

$$csoln := c(x) = -\frac{_CI\left(-\frac{1}{_CI} - \frac{\int 1 + 2 \, KI \, K2 \, _CI \, x + 2 \, KI \, K2 \, _CI \, _C2}{_CI}\right)}{KI \, K2}$$

$$c(x) = -\frac{_CI\left(-\frac{1}{_CI} + \frac{\int 1 + 2 \, KI \, K2 \, _CI \, x + 2 \, KI \, K2 \, _CI \, _C2}{_CI}\right)}{KI \, K2}$$

Note #1: There are apparently two "roots" to the solution for c(x). The boundary conditions will be used to select the proper one.

> c1:=unapply(simplify(op(2,csoln[1])),x);

$$c1 := x \to \frac{1 + \sqrt{1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2}}{K1 K2}$$

> c1(x);

$$\frac{1 + \sqrt{1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2}}{K1 K2}$$

> c2:=unapply(simplify(op(2,csoln[2])),x);

$$c2 := x \to -\frac{-1 + \sqrt{1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2}}{K1 K2}$$

> c2(x);

$$-\frac{-1 + \sqrt{1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2}}{K1 K2}$$

> J1:=unapply(simplify(Ds*(K1*K2*c1(x)-1)*diff(c1(x),x),x));

$$J1 := () \rightarrow Ds \quad C1$$

> J1(x);

> J2:=unapply(simplify(Ds*(K1*K2*c2(x)-1)*diff(c2(x),x)));

$$J2 := () \rightarrow Ds _C1$$

> J2(x);

Note #2: The above results show that both mass flux solutions are identical and equal to Ds*_C1 where _C1 is a constant of integration!

> ex1:=dsolve(diff(e(x),x)-K2*diff(c1(x),x)=0,e(x));

$$ex1 := e(x) = \frac{\sqrt{1 + 2 K1 K2 C1 x + 2 K1 K2 C1 C2} + C3 K1}{K1}$$

> e1:=unapply(simplify(op(2,ex1)),x);

fy(op(2,ex1)),x);

$$e1 := x \rightarrow \frac{\sqrt{1 + 2 \ K1 \ K2 \ C1 \ x + 2 \ K1 \ K2 \ C1 \ C2} + C3 \ K1}{K1}$$

> e1(x);

$$\frac{\sqrt{1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2} + _C3 K1}{K1}$$

> ex2:=dsolve(diff(e(x),x)-K2*diff(c2(x),x)=0,e(x));

$$e(x),x)-K2^*diff(c2(x),x)=0,e(x));$$

$$ex2 := e(x) = \frac{-\sqrt{1+2 K1 K2 C1 x+2 K1 K2 C1 C2} + C3 K1}{K1}$$

> e2:=unapply(simplify(op(2,ex2)),x);

$$e2 := x \to \frac{-\sqrt{1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2} + _C3 K1}{K1}$$

> e2(x);

$$\frac{-\sqrt{1 + 2 \ K1 \ K2 \ _C1 \ x + 2 \ K1 \ K2 \ _C1 \ _C2} + _C3 \ K1}{K1}$$

> ux1:=dsolve(A1*diff(u(x),x)+A2*c1(x)-A3-e1(x)=0,u(x));

$$uxI := u(x) = \frac{1}{3} \left(-3 A2 x K1 K2 _CI - A2 \sqrt{\%1} - 2 \sqrt{\%1} A2 K1 K2 _CI x \right)$$

$$-2 \sqrt{\%1} A2 K1 K2 _CI _C2 + 3 A3 x KI^2 K2^2 _CI + \sqrt{\%1} K2 + 2 \sqrt{\%1} K2^2 KI _CI x$$

$$+2 \sqrt{\%1} K2^2 K1 _CI _C2 + 3 _C3 x KI^2 K2^2 _CI + 3 _C4 A1 KI^2 K2^2 _CI \right) / (A1 KI^2 K2^2 _CI)$$

%1 := 1 + 2 K1 K2 C1 x + 2 K1 K2 C1 C2

> u1:=unapply(simplify(op(2,ux1)),x);

$$u1 := x \to \frac{1}{3} \left(-3 \text{ A2 } x \text{ K1 K2 _C1 - A2} \sqrt{1 + 2 \text{ K1 K2 _C1 } x + 2 \text{ K1 K2 _C1 _C2}} \right.$$

$$-2 \sqrt{1 + 2 \text{ K1 K2 _C1 } x + 2 \text{ K1 K2 _C1 _C2}} \text{ A2 K1 K2 _C1 } x$$

$$-2 \sqrt{1 + 2 \text{ K1 K2 _C1 } x + 2 \text{ K1 K2 _C1 _C2}} \text{ A2 K1 K2 _C1 _C2 + 3 A3 } x \text{ K1}^2 \text{ K2}^2 \text{ _C1}}$$

$$+ \sqrt{1 + 2 \text{ K1 K2 _C1 } x + 2 \text{ K1 K2 _C1 _C2}} \text{ K2}$$

$$+2 \sqrt{1 + 2 \text{ K1 K2 _C1 } x + 2 \text{ K1 K2 _C1 _C2}} \text{ K2}^2 \text{ K1 _C1 } x$$

$$+2 \sqrt{1 + 2 \text{ K1 K2 _C1 } x + 2 \text{ K1 K2 _C1 _C2}} \text{ K2}^2 \text{ K1 _C1 _C2 + 3 _C3 } x \text{ K1}^2 \text{ K2}^2 \text{ _C1}}$$

$$+3 \text{_C4 A1 K1}^2 \text{ K2}^2 \text{_C1} / (\text{A1 K1}^2 \text{ K2}^2 \text{_C1})$$

> u1(x);

$$\frac{1}{3} \left(-3 A2 x K1 K2 _C1 - A2 \sqrt{\%1} - 2 \sqrt{\%1} A2 K1 K2 _C1 x - 2 \sqrt{\%1} A2 K1 K2 _C1 _C2 \right.$$

$$+ 3 A3 x K1^2 K2^2 _C1 + \sqrt{\%1} K2 + 2 \sqrt{\%1} K2^2 K1 _C1 x + 2 \sqrt{\%1} K2^2 K1 _C1 _C2$$

$$+ 3 _C3 x K1^2 K2^2 _C1 + 3 _C4 A1 K1^2 K2^2 _C1 \right) / (A1 K1^2 K2^2 _C1)$$

 $%1 := 1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2$

> ux2:=dsolve(A1*diff(u(x),x)+A2*c2(x)-A3-e2(x)=0,u(x));

$$ux2 := u(x) = \frac{1}{3} \left(-3 A2 x K1 K2 _C1 + A2 \sqrt{\%1} + 2 \sqrt{\%1} A2 K1 K2 _C1 x \right)$$

$$+ 2 \sqrt{\%1} A2 K1 K2 _C1 _C2 + 3 A3 x K1^2 K2^2 _C1 - \sqrt{\%1} K2 - 2 \sqrt{\%1} K2^2 K1 _C1 x$$

$$- 2 \sqrt{\%1} K2^2 K1 _C1 _C2 + 3 _C3 x K1^2 K2^2 _C1 + 3 _C4 A1 K1^2 K2^2 _C1 \right) / (A1 K1^2 K2^2 _C1)$$

%1 := 1 + 2 K1 K2 C1 x + 2 K1 K2 C1 C2

> u2:=unapply(simplify(op(2,ux2)),x);

$$u2 := x \rightarrow \frac{1}{3} \left(-3 \ A2 \ x \ K1 \ K2 \ C1 + A2 \sqrt{1 + 2 \ K1 \ K2 \ C1 \ x + 2 \ K1 \ K2 \ C1 \ C2} \right)$$

$$+ 2 \sqrt{1 + 2 KI K2 _CI x + 2 KI K2 _CI _C2} A2 KI K2 _CI x$$

$$+ 2 \sqrt{1 + 2 KI K2 _CI x + 2 KI K2 _CI _C2} A2 KI K2 _CI _C2 + 3 A3 x KI^2 K2^2 _CI$$

$$- \sqrt{1 + 2 KI K2 _CI x + 2 KI K2 _CI _C2} K2$$

$$- 2 \sqrt{1 + 2 KI K2 _CI x + 2 KI K2 _CI _C2} K2^2 KI _CI x$$

$$- 2 \sqrt{1 + 2 KI K2 _CI x + 2 KI K2 _CI _C2} K2^2 KI _CI _C2 + 3 _C3 x KI^2 K2^2 _CI$$

$$+ 3 _C4 AI KI^2 K2^2 _CI) / (AI KI^2 K2^2 _CI)$$

> u2(x);

$$\frac{1}{3} \left(-3 A2 x K1 K2 _C1 + A2 \sqrt{\%1} + 2 \sqrt{\%1} A2 K1 K2 _C1 x + 2 \sqrt{\%1} A2 K1 K2 _C1 _C2 + 3 A3 x K1^2 K2^2 _C1 - \sqrt{\%1} K2 - 2 \sqrt{\%1} K2^2 K1 _C1 x - 2 \sqrt{\%1} K2^2 K1 _C1 _C2 + 3 _C3 x K1^2 K2^2 _C1 + 3 _C4 A1 K1^2 K2^2 _C1 \right) / (A1 K1^2 K2^2 _C1)$$

$$%1 := 1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2$$

> S1:=x->simplify(B1*e1(x)-B2*c1(x)+B3);

$$S1 := x \rightarrow \text{simplify}(B1 \text{ e1}(x) - B2 \text{ c1}(x) + B3)$$

> S1(x);

$$(B1 K2 \sqrt{1 + 2 K1 K2 C1 x + 2 K1 K2 C1 C2 + B1 K2 C3 K1 - B2} - B2 \sqrt{1 + 2 K1 K2 C1 x + 2 K1 K2 C1 C2 + B3 K1 K2}) / (K1 K2)$$

> S2:=x->simplify(B1*e2(x)-B2*c2(x)+B3);

$$S2 := x \rightarrow \text{simplify}(B1 \text{ e2}(x) - B2 \text{ c2}(x) + B3)$$

> S2(x);

$$\left(-B1 \ K2 \sqrt{1 + 2 \ K1 \ K2 \ _C1 \ x + 2 \ K1 \ K2 \ _C1 \ _C2} + B1 \ K2 \ _C3 \ K1 - B2 \right. \\ + B2 \sqrt{1 + 2 \ K1 \ K2 \ _C1 \ x + 2 \ K1 \ K2 \ _C1 \ _C2} + B3 \ K1 \ K2 \right) / (K1 \ K2)$$

Note #3: At this point, we will introduce expressions for the constants and then simplify the results before quantifying the constants and determining the constants of integration.

> K:=(Em/(3*(1-2*nu)));

$$K := \frac{Em}{3 - 6 v}$$

> lambda:=(Em*nu)/((1+nu)*(1-2*nu));

$$\lambda := \frac{Em \, v}{(1+v)(1-2 \, v)}$$

> mu:=Em/(2*(1+nu));

$$\mu := \frac{Em}{2 + 2 v}$$

> alpha:=(rho*Vs)/(3*MWs);

$$\alpha := \frac{1}{3} \frac{\rho \ Vs}{MWs}$$

> K1:=(Vs*K)/(R*T);

$$KI := \frac{Vs Em}{(3-6 \text{ V}) R T}$$

> K2:=3*alpha*(1+traps);

$$K2 := \frac{\rho \ Vs (1 + traps)}{MWs}$$

> K1*K2;

$$\frac{Vs^2 Em \rho (1 + traps)}{(3 - 6 v) R T MWs}$$

> A1:=(1-2*nu);

$$A1 := 1 - 2 v$$

> A2:=2*(1+nu)*alpha*(1+traps);

$$A2 := \frac{2}{3} \frac{(1+v) \rho Vs (1+traps)}{MWs}$$

> A3:=A2*c0;

$$A3 := \frac{2}{3} \frac{(1+v) \rho Vs (1 + traps) cO}{MWs}$$

> B1:=3*K;

$$BI := 3 \frac{Em}{3 - 6 v}$$

> B2:=9*K*alpha*(1+traps);

$$B2 := 3 \frac{Em \rho Vs (1 + traps)}{(3 - 6 v) MWs}$$

> B3:=B2*c0;

$$B3 := 3 \frac{Em \rho \ Vs (1 + traps) \ c0}{(3 - 6 \ v) \ MWs}$$

> et(x);
$$\left(1 + \sqrt{1 + 2 \frac{Vs^2 Em \rho (1 + traps) CI x}{(3 - 6 v) R T MWs}} + 2 \frac{Vs^2 Em \rho (1 + traps) CI C2}{(3 - 6 v) R T MWs}\right) (3 - 6 v) R T$$

$$MWs/(Vs^2 Em \rho (1 + traps)) > e2(x);$$

$$- \left(-1 + \sqrt{1 + 2 \frac{Vs^2 Em \rho (1 + traps) CI x}{(3 - 6 v) R T MWs}} + 2 \frac{Vs^2 Em \rho (1 + traps) CI C2}{(3 - 6 v) R T MWs}\right) (3 - 6 v) R T$$

$$MWs/(Vs^2 Em \rho (1 + traps)) > e1(x);$$

$$\left(\sqrt{1 + 2 \frac{Vs^2 Em \rho (1 + traps) CI x}{(3 - 6 v) R T MWs}} + 2 \frac{Vs^2 Em \rho (1 + traps) CI C2}{(3 - 6 v) R T MWs}\right) + \frac{-C3 Vs Em}{(3 - 6 v) R T}$$

$$(3 - 6 v) R T/(Vs Em) > e2(x);$$

$$\left(-\sqrt{1 + 2 \frac{Vs^2 Em \rho (1 + traps) CI x}{(3 - 6 v) R T MWs}} + 2 \frac{Vs^2 Em \rho (1 + traps) CI C2}{(3 - 6 v) R T MWs}\right) + \frac{-C3 Vs Em}{(3 - 6 v) R T}$$

$$(3 - 6 v) R T/(Vs Em) > e1(x);$$

$$\frac{1}{3} \left(-2 \frac{(1 + v) \rho^2 Vs^3 (1 + traps)^2 x Em CI}{MWs^2 (3 - 6 v) R T} - \frac{2}{3} \frac{(1 + v) \rho Vs (1 + traps) \sqrt{\%1}}{MWs} \right)$$

$$- \frac{4 \sqrt{\%1} (1 + v) \rho^2 Vs^3 (1 + traps)^2 Em CI x}{MWs^2 (3 - 6 v) R T} + 2 \frac{4 \sqrt{\%1} (1 + v) \rho^2 Vs^3 (1 + traps)^2 Em CI C2}{MWs^2 (3 - 6 v) R T} + 2 \frac{(1 + v) \rho^3 Vs^5 (1 + traps)^2 Em CI C2}{MWs^2 (3 - 6 v) R T} + 2 \frac{(1 + v) \rho^3 Vs^5 (1 + traps)^2 Em CI x}{MWs^2 (3 - 6 v) R T} + 2 \frac{\sqrt{\%1} \rho^2 Vs^3 (1 + traps)^2 Em CI x}{MWs^2 (3 - 6 v) R T} + 2 \frac{\sqrt{\%1} \rho^2 Vs^3 (1 + traps)^2 Em CI x}{MWs^2 (3 - 6 v) R T} + 2 \frac{\sqrt{\%1} \rho^2 Vs^3 (1 + traps)^2 Em CI x}{MWs^2 (3 - 6 v) R T} + 2 \frac{\sqrt{\%1} \rho^2 Vs^3 (1 + traps)^2 Em CI x}{MWs^2 (3 - 6 v) R T} + 2 \frac{\sqrt{\%1} \rho^2 Vs^3 (1 + traps)^2 CI}{MWs^2 (3 - 6 v) R T} + 2 \frac{-C3 Vs Em \rho CI x}{MWs^2 (3 - 6 v) R T} + 2 \frac{-C3 Vs Em \rho CI x}{MWs^2 (3 - 6 v) R T} + 2 \frac{-C4 (1 - 2 v) Vs^4 Em^2 \rho^2 (1 + traps)^2 CI}{(3 - 6 v)^2 R^2 T^2 MWs^2} + 2 \frac{-C4 (1 - 2 v) Vs^4 Em^2 \rho^2 (1 + traps)^2 CI}{(3 - 6 v)^2 R^2 T^2 MWs^2} + 2 \frac{-C4 (1 - 2 v) Vs^4 Em^2 \rho^2 (1 + traps)^2 CI}{(3 - 6 v)^2 R^2 T^2 MWs^2} + 2 \frac{-C4 (1 - 2 v) Vs^4 Em^2 \rho^2 (1 + traps)^2 CI}{(3 - 6 v)^2 R^2 T^2 MWs^2} + 2 \frac{-C4 (1 - 2 v) Vs^4 Em^2 \rho^2 (1 + traps)^2 CI}{(3 - 6 v)^2 R^2 T^2 MWs^2} + 2 \frac{-C4 (1 - 2 v) Vs^4 Em^2 \rho^2 (1 + traps)^2 CI}{(3 - 6 v)^2 R^2 T^2 MWs^2} +$$

$$\%1 := 1 + 2 \frac{Vs^2 Em \rho (1 + traps) _C1 x}{(3 - 6 v) R T MWs} + 2 \frac{Vs^2 Em \rho (1 + traps) _C1 _C2}{(3 - 6 v) R T MWs}$$

$$\frac{1}{3} \left(-2 \frac{(1+v) \rho^{2} V s^{3} (1 + traps)^{2} x Em_CI}{MW s^{2} (3 - 6 v) R T} + \frac{2}{3} \frac{(1+v) \rho V s (1 + traps) \sqrt{61}}{MW s} \right)$$

$$+ \frac{4}{3} \frac{\sqrt{61} (1+v) \rho^{2} V s^{3} (1 + traps)^{2} Em_CI x}{MW s^{2} (3 - 6 v) R T}$$

$$+ \frac{4}{3} \frac{\sqrt{61} (1+v) \rho^{2} V s^{3} (1 + traps)^{2} Em_CI_C2}{MW s^{2} (3 - 6 v) R T}$$

$$+ 2 \frac{(1+v) \rho^{3} V s^{5} (1 + traps)^{3} co x Em^{2}_CI}{MW s^{3} (3 - 6 v)^{2} R^{2} T^{2}} - \frac{\sqrt{61} \rho V s (1 + traps)}{MW s}$$

$$- 2 \frac{\sqrt{61} \rho^{2} V s^{3} (1 + traps)^{2} Em_CI x}{MW s^{2} (3 - 6 v) R T} - 2 \frac{\sqrt{61} \rho^{2} V s^{3} (1 + traps)^{2} Em_CI_C2}{MW s^{2} (3 - 6 v) R T}$$

$$+ 3 \frac{-C3 x V s^{4} Em^{2} \rho^{2} (1 + traps)^{2} - CI}{(3 - 6 v)^{2} R^{2} T^{2} MW s^{2}} + 3 \frac{-C4 (1 - 2 v) V s^{4} Em^{2} \rho^{2} (1 + traps)^{2} - CI}{(3 - 6 v)^{2} R^{2} T^{2} MW s^{2}}$$

$$- \frac{(3 - 6 v)^{2} R^{2} T^{2} MW s^{2}}{(1 - 2 v) V s^{4} Em^{2} \rho^{2} (1 + traps)^{2} - CI}$$

$$\%1 := 1 + 2 \frac{Vs^2 Em \rho (1 + traps) _C1 x}{(3 - 6 v) R T MWs} + 2 \frac{Vs^2 Em \rho (1 + traps) _C1 _C2}{(3 - 6 v) R T MWs}$$

> S1(x);

-
$$(MWs _C3 \ Vs \ Em - 3 \ R \ T \ MWs + 6 \ R \ T \ MWs \ v + Em \ \rho \ Vs^2 \ c0 + Em \ \rho \ Vs^2 \ c0 \ traps)/((-1 + 2 \ v) \ Vs \ MWs)$$

> S2(x);

-
$$(MWs _C3 \ Vs \ Em - 3 \ R \ T \ MWs + 6 \ R \ T \ MWs \ v + Em \ \rho \ Vs^2 \ c0 + Em \ \rho \ Vs^2 \ c0 \ traps)/($$
 $(-1 + 2 \ v) \ Vs \ MWs)$

> S1(x)-S2(x);

0

Note #4: The stresses are same for each solution root, and do not depend on the position x! The stresses are therefore constant throughout the body. We will solve for $_C3$ in terms of the boundary stress, S0.

> _C3:=simplify(solve(S1(x)=S0,_C3));

$$C3 := -(-3 R T MWs + 6 R T MWs v + Em \rho Vs^2 c0 + Em \rho Vs^2 c0 traps - S0 Vs MWs + 2 S0 Vs MWs v) / (MWs Vs Em)$$

> S1(x); S0 > S2(x); *S0* > S1:=x->S0; $S1 := x \rightarrow S0$ > S2:=x->S0; $S2 := x \rightarrow S0$ solute in AISI 4340 steel.

Note #5: Now we will introduce numerical values for the material properties. The values selected are typical for internal hydrogen as a

> S0:=0.0; S0 := 0> Ds:=1e-9; $Ds := .1 \ 10^{-8}$ > Vs:=2.02e-6; $Vs := .202 \ 10^{-5}$ > traps:=19; traps := 19> c0:=0.0; c0 := 0> MWs:=0.00100797; MWs := .00100797> Em:=200e9; $Em := .200 \ 10^{12}$ > nu:=0.3; v := .3> rho:=7800; $\rho := 7800$ > T:=293; T := 293> R:=8.31432; R := 8.31432> K;

> lambda;

 $.115384615384616\ 10^{12}$

> mu;

 $.769230769230769\ 10^{11}$

> alpha;

```
5.21047253390476
```

> Rh:=R/MWs; Rh := 8248.57882675080> K1; 138.199274509089 > K2; 312.628352034286 > K1*K2; 43205.0114421104 > 1/(K1*K2); .0000231454631447068 > A1; .4 > A2; 270.944571763048 > A3; 0 > B1; $.500000000000001\ 10^{12}$ > B2; $.156314176017143\ 10^{15}$ > B3; 0 > _C3; .00723592800000000> S1(x);0 > S2(x);0 Note #6: Now let's solve for the integration constants: _C1, _C2, and _C4, using the boundary data. > c1(x);.0000231454631447068 + .0000231454631447068 $\int 1 + 86410.0228842208 \ CI \ x + 86410.0228842208 \ CI \ C2$ > c2(x); .0000231454631447068 - .0000231454631447068 $\int 1 + 86410.0228842208 \ C1 \ x + 86410.0228842208 \ C1 \ C2$

> evalf(c1(x),5);

$$.000023146 + .000023146 \sqrt{1. + 86408. _C1 x + 86408. _C1 _C2}$$

> eqnset1:={0=c1(0),1e-6=c1(0.10)};

$$eqnset1 := \begin{cases} .1 \ 10^{-5} = .0000231454631447068 + .0000231454631447068 \\ \sqrt{1 + 8641.00228842208} \ _CI + 86410.0228842208 \ _CI \ _C2 \ , 0 = \end{cases}$$

 $.0000231454631447068 + .0000231454631447068 \sqrt{1 + 86410.0228842208} \ _C1 \ _C2$

> varset1:={_C1,_C2};

$$varset1 := \{ _C2, _C1 \}$$

> solnset1:=solve(eqnset1,varset1);

> eqnset2:={0=c2(0),1e-6=c2(0.10)};

$$eqnset2 := \{ .1 \ 10^{-5} = .0000231454631447068 \}$$

- .0000231454631447068 $\sqrt{1 + 8641.00228842208}$ _*C1* + 86410.0228842208 _*C1* _*C2* , 0 =

 $.0000231454631447068 - .0000231454631447068 \sqrt{1 + 86410.0228842208 _C1 _C2}$ > varset2:={_C1,_C2};

> solnset2:=solve(eqnset2,varset2);

$$solnset2 := \{ C1 = -.978397494278943 \ 10^{-5}, \ C2 = 0 \}$$

> assign(solnset2);

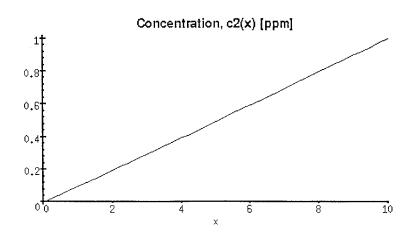
Note #7: The solution c1(x) is not capable of satisfying the boundary conditions, as evidenced by the lack of solution for the given boundary conditions. The concentration function c2(x) is the correct solution in this case!

> c2(x);

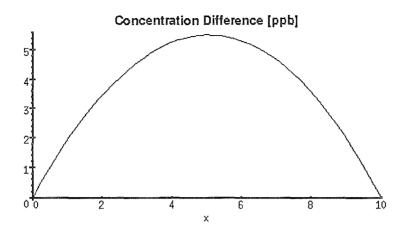
$$.0000231454631447068 - .0000231454631447068 \sqrt{1 - .845433498705078} x$$

- > p1:=plot(1e6*c2(x/100),x=0..10,title='Concentration, c2(x) [ppm]'):
- > p1;

> .



> p2:=plot(1e9*(c2(0)+((c2(0.10)-c2(0))/0.10)*x/100-c2(x/100)),x=0..10,title='Concentration Difference [ppb]'): > p2;

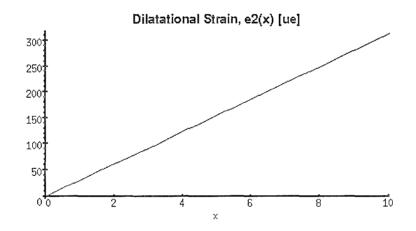


> e2(x);

 $-.0072359279999998\sqrt{1 - .845433498705078 x} + .00723592799999998$

> p3:=plot(1e6*e2(x/100),x=0..10,title='Dilatational Strain, e2(x) [ue]'):

> p3;



> u2(x);

 $.00241197600000008\,x + .00190196390663828\,\sqrt{1 - .845433498705078\,x}$

 $-.00160798400000005\sqrt{1-.845433498705078 x} x + .999999999999996 _C4$

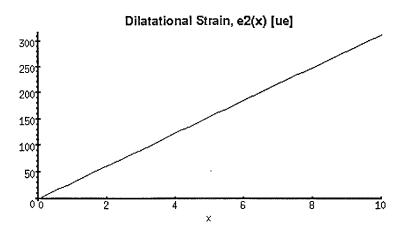
> _C4:=solve(u2(0)=0,_C4);

_*C4* := -.00190196390663829

> u2(x);

.00241197600000008 x + .00190196390663828 $\sqrt{1$ - .845433498705078 x - .00160798400000005 $\sqrt{1}$ - .845433498705078 x x - .00190196390663827

> p4:=plot(1e6*u2(x/100),x=0..10,title='Displacement, u2(x) [um]'):



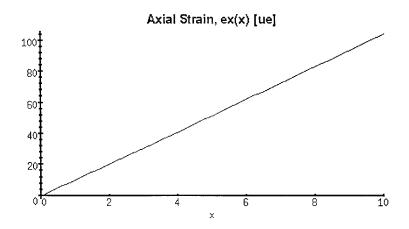
> ex:=unapply(diff(u2(x),x),x);

$$ex := x \to .00241197600000008 - .00080399199999990 \frac{1}{\sqrt{1 - .845433498705078 \, x}}$$

$$+ .000679721769490914 \frac{x}{\sqrt{1 - .845433498705078} x}$$

$$- .00160798400000005 \sqrt{1 - .845433498705078} x$$

> ex(x);



> p5:=plot(1e6*ex(x/100),x=0..10,title='Axial Strain, ex(x) [ue]'):

1. MAPLE PROGRAM DIRECTORY AND FILE NAME: /afs/nd.edu/user4/jthomas5/Maple/Transport/1DSSsol#1b.ms _______________________ 2. ORIGINATOR: Dr. James P. Thomas University of Notre Dame Department of Aerospace and Mechanical Engineering 374 Fitzpatrick Hall Notre Dame, IN 46556-5637 (219) 631-9371 3. HISTORY. Written: June-1994 Latest Revision: 13-Feb-1995 4. PROGRAM DESCRIPTION: 4. PROGRAM DESCRIPTION:
One-dimensional, steady-state analysis of the fully coupled solute transport and trapping equations using "plane stress" constitutive equations for stress. The particular problem solved is shown below: / 1 ---> x [m] x=0 x=0.010Deformation variable boundary conditions: u(0)=0, S(0.10)=0 Concentration variable boundary conditions: c(0)=0, c(0.10)=1e-7 Reference concentration level: c0=0.0 Trapping parameter: traps=19 (low trapping) The equations used in the analysis are: 1. dJ/dx=0 (steady-state diffusion equation) 2. J=Ds*(K1*c*de/dx-dc/dx) (mass flux) 3. dS/dx=0 (steady-state deformation equation with zero body forces) 4. S=B1*e-B2*c+B3 (axial stress) 5. de/dx-K2*dc/dx=() (dilational strain gradient) 6. e=A1*du/dx+A2*c-A3 (dilatational strain) The constants used in the plane stress analysis are defined below: Ds:=lattice diffusivity [m^2/sec] K1 := (Vs*K) / (R*T)Vs:= partial molar volume of solute [m^3/mol solute] $K:= bulk modulus=Em/(3*(1-2*nu)) [N/m^2]$ Em: = Modulus of Elasticity [N/m^2] nu:= Poisson's ratio [1] R:= universal gas constant=8.31432 [J/mol-K] T:= temperature [K] B1:=3*K B2:=9*K*alpha*(1+traps) B3:=B2*co traps:= Csr*Kr=trapping constant [1] Csr:= saturation trap concentration [1] Kr:= equilibrium trapping constant [1] c0:= reference solute concentration [1]

alpha:= (rho*Vs)/(3*MWs)=solute concentration expansion coefficient [m/m/Delc]

K2:=3*alpha*(1+traps)

Delc:= c-c0 [1]

rho:= mass density of the solid [kg/m^3]
MWs:= molecular weight of the solute [kg/mol]

A1:=1-2*nu A2:=2*(1+nu)*alpha*(1+traps) A3:=A2*c0

5. REQUIRED INPUTS:

Ds:=lattice diffusivity [m^2/sec]
Vs:= partial molar volume of solute [m^3/mol solute]
traps:= Csr*Kr=trapping constant [1]
c0:= reference solute concentration [1]
MWs:= molecular weight of the solute [kg/mol]

Em:= Modulus of Elasticity [Pa]
nu:= Poisson's ratio [1]
rho:= mass density of the solid [kg/m^3]

T:= temperature [K]

6. SYMBOLIC ANALYSIS:

> Digits:=trunc(evalhf(Digits));

$$Digits := 15$$

> J:=Ds*(K1*K2*c(x)-1)*diff(c(x),x);

$$J := Ds (K1 \ K2 \ c(x) - 1) \left(\frac{\partial}{\partial x} c(x) \right)$$

> deqn:=diff(J,x)=0;

$$deqn := Ds K1 K2 \left(\frac{\partial}{\partial x} c(x)\right)^2 + Ds (K1 K2 c(x) - 1) \left(\frac{\partial^2}{\partial x^2} c(x)\right) = 0$$

> csoln:=dsolve(deqn,c(x),explicit);

$$csoln := c(x) = -\frac{-CI\left(-\frac{1}{_CI} - \frac{\sqrt{1 + 2 \ KI \ K2 \ _CI \ x + 2 \ KI \ K2 \ _CI} - \frac{CI}{_CI}\right)}{KI \ K2},$$

$$c(x) = -\frac{-CI\left(-\frac{1}{_CI} + \frac{\sqrt{1 + 2 \ KI \ K2 \ _CI \ x + 2 \ KI \ K2 \ _CI} - \frac{CI}{_CI}\right)}{KI \ K2}$$

Note #1: There are apparently two "roots" to the solution for c(x). The boundary conditions will be used to select the proper one.

> c1:=unapply(simplify(op(2,csoln[1])),x);

$$c1 := x \to \frac{1 + \sqrt{1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2}}{K1 K2}$$

> c1(x);

$$\frac{1 + \sqrt{1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2}}{K1 K2}$$

> c2:=unapply(simplify(op(2,csoln[2])),x);

$$c2 := x \to -\frac{-1 + \sqrt{1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2}}{K1 K2}$$

> c2(x);

$$\frac{-1 + \sqrt{1 + 2 KI K2 _CI x + 2 KI K2 _CI _C2}}{KI K2}$$

> J1:=unapply(simplify(Ds*(K1*K2*c1(x)-1)*diff(c1(x),x),x));

$$J1 := () \rightarrow Ds _C1$$

> J1(x);

> J2:=unapply(simplify(Ds*(K1*K2*c2(x)-1)*diff(c2(x),x)));

$$J2 := () \rightarrow Ds _C1$$

> J2(x);

Note #2: The above results show that both mass flux solutions are identical and equal to Ds*_C1 where _C1 is a constant of integration!

> ex1:=dsolve(diff(e(x),x)-K2*diff(c1(x),x)=0,e(x));

$$ex1 := e(x) = \frac{\sqrt{1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2} + _C3 K1}{K1}$$

> e1:=unapply(simplify(op(2,ex1)),x);

$$e1 := x \to \frac{\sqrt{1 + 2 K1 K2 C1 x + 2 K1 K2 C1 C2} + C3 K1}{K1}$$

> e1(x);

$$\sqrt{1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2} + _C3 K1$$
K1

> ex2:=dsolve(diff(e(x),x)-K2*diff(c2(x),x)=0,e(x));

$$ex2 := e(x) = \frac{-\sqrt{1 + 2 K1 K2 C1 x + 2 K1 K2 C1 C2} + C3 K1}{K1}$$

> e2:=unapply(simplify(op(2,ex2)),x);

$$e2 := x \to -\frac{\sqrt{1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2} - _C3 K1}{K1}$$

> e2(x);

$$-\frac{\sqrt{1+2 K1 K2 _C1 x+2 K1 K2 _C1 _C2} - _C3 K1}{K1}$$

> ux1:=dsolve(A1*diff(u(x),x)+A2*c1(x)-A3-e1(x)=0,u(x));

$$uxI := u(x) = \frac{1}{3} \left(-3 \text{ A2 } x \text{ K1 } \text{ K2 } _\text{C1} - \text{A2} \sqrt{\%1} - 2 \sqrt{\%1} \text{ A2 } \text{K1 } \text{K2 } _\text{C1 } x \right)$$

$$- 2 \sqrt{\%1} \text{ A2 } \text{K1 } \text{K2 } _\text{C1 } _\text{C2} + 3 \text{ A3 } x \text{ K1}^2 \text{ K2}^2 _\text{C1} + \sqrt{\%1} \text{ K2} + 2 \sqrt{\%1} \text{ K2}^2 \text{ K1 } _\text{C1 } x$$

$$+ 2 \sqrt{\%1} \text{ K2}^2 \text{ K1 } _\text{C1 } _\text{C2} + 3 \text{ _C3 } x \text{ K1}^2 \text{ K2}^2 _\text{C1} + 3 \text{ _C4 } \text{A1 } \text{K1}^2 \text{ K2}^2 _\text{C1} \right) / (\text{A1 } \text{K1}^2 \text{ K2}^2 _\text{C1})$$

 $%1 := 1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2$

> u1:=unapply(simplify(op(2,ux1)),x);

$$ul := x \rightarrow -\frac{1}{3} \left(3 \text{ A2 } x \text{ K1 K2 _C1} + \text{A2} \sqrt{1 + 2 \text{ K1 K2 _C1} x + 2 \text{ K1 K2 _C1} _C2} \right.$$

$$+ 2 \sqrt{1 + 2 \text{ K1 K2 _C1} x + 2 \text{ K1 K2 _C1} _C2} \text{ A2 K1 K2 _C1} x$$

$$+ 2 \sqrt{1 + 2 \text{ K1 K2 _C1} x + 2 \text{ K1 K2} _C1} _C2 \text{ A2 K1 K2 _C1} _C2 - 3 \text{ A3 } x \text{ K1}^2 \text{ K2}^2 _C1}$$

$$- \sqrt{1 + 2 \text{ K1 K2} _C1 x + 2 \text{ K1 K2} _C1} _C2 \text{ K2}$$

$$- 2 \sqrt{1 + 2 \text{ K1 K2} _C1 x + 2 \text{ K1 K2} _C1} _C2 \text{ K2}^2 \text{ K1} _C1 x$$

$$- 2 \sqrt{1 + 2 \text{ K1 K2} _C1 x + 2 \text{ K1 K2} _C1} _C2 \text{ K2}^2 \text{ K1} _C1 _C2 - 3 _C3 x \text{ K1}^2 \text{ K2}^2 _C1}$$

$$- 3 _C4 \text{ A1 K1}^2 \text{ K2}^2 _C1 \right) / (\text{A1 K1}^2 \text{ K2}^2 _C1)$$

> u1(x);

$$-\frac{1}{3} \left(3 A2 x K1 K2 _C1 + A2 \sqrt{\%1} + 2 \sqrt{\%1} A2 K1 K2 _C1 x + 2 \sqrt{\%1} A2 K1 K2 _C1 _C2 \right)$$

$$-3 A3 x K1^2 K2^2 _C1 - \sqrt{\%1} K2 - 2 \sqrt{\%1} K2^2 K1 _C1 x - 2 \sqrt{\%1} K2^2 K1 _C1 _C2$$

$$-3 _C3 x K1^2 K2^2 _C1 - 3 _C4 A1 K1^2 K2^2 _C1 \right) / (A1 K1^2 K2^2 _C1)$$

 $%1 := 1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2$

> ux2:=dsolve(A1*diff(u(x),x)+A2*c2(x)-A3-e2(x)=0,u(x));

$$ux2 := u(x) = \frac{1}{3} \left(-3 A2 x K1 K2 _C1 + A2 \sqrt{\%1} + 2 \sqrt{\%1} A2 K1 K2 _C1 x \right)$$

$$+ 2 \sqrt{\%1} A2 K1 K2 _C1 _C2 + 3 A3 x K1^2 K2^2 _C1 - \sqrt{\%1} K2 - 2 \sqrt{\%1} K2^2 K1 _C1 x$$

$$- 2 \sqrt{\%1} K2^2 K1 _C1 _C2 + 3 _C3 x K1^2 K2^2 _C1 + 3 _C4 A1 K1^2 K2^2 _C1 \right) / (A1 K1^2 K2^2 _C1)$$

$$%1 := 1 + 2 KI K2 _CI x + 2 KI K2 _CI _C2$$

> u2:=unapply(simplify(op(2,ux2)),x);

$$u2 := x \to \frac{1}{3} \left(-3 \ A2 \ x \ K1 \ K2 \ _C1 + A2 \sqrt{1 + 2 \ K1 \ K2 \ _C1 \ x + 2 \ K1 \ K2 \ _C1 \ _C2} \right)$$

> u2(x);

$$\frac{1}{3} \left(-3 A2 x K1 K2 _C1 + A2 \sqrt{\%1} + 2 \sqrt{\%1} A2 K1 K2 _C1 x + 2 \sqrt{\%1} A2 K1 K2 _C1 _C2 + 3 A3 x K1^2 K2^2 _C1 - \sqrt{\%1} K2 - 2 \sqrt{\%1} K2^2 K1 _C1 x - 2 \sqrt{\%1} K2^2 K1 _C1 _C2 + 3 _C3 x K1^2 K2^2 _C1 + 3 _C4 A1 K1^2 K2^2 _C1 \right) / (A1 K1^2 K2^2 _C1)$$

$$%1 := 1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2$$

> S1:=x->simplify(B1*e1(x)-B2*c1(x)+B3);

$$S1 := x \rightarrow \text{simplify}(B1 \text{ e1}(x) - B2 \text{ c1}(x) + B3)$$

> S1(x);

$$(B1 \ K2 \sqrt{1 + 2 \ K1 \ K2 \ _C1 \ x + 2 \ K1 \ K2 \ _C1 \ _C2} + B1 \ K2 \ _C3 \ K1 - B2$$

$$- B2 \sqrt{1 + 2 \ K1 \ K2 \ _C1 \ x + 2 \ K1 \ K2 \ _C1 \ _C2} + B3 \ K1 \ K2) / (K1 \ K2)$$

> S2:=x->simplify(B1*e2(x)-B2*c2(x)+B3);

$$S2 := x \rightarrow \text{simplify}(B1 \text{ e2}(x) - B2 \text{ c2}(x) + B3)$$

> S2(x);

Note #3: At this point, we will introduce expressions for the constants and then simplify the results before quantifying the constants and determining the constants of integration.

> K:=(Em/(3*(1-2*nu)));

$$K := \frac{Em}{3 - 6 \,\mathrm{V}}$$

> lambda:=(Em*nu)/((1+nu)*(1-2*nu));

$$\lambda := \frac{Em \, \nu}{(1 + \nu) (1 - 2 \, \nu)}$$

> mu:=Em/(2*(1+nu));

$$\mu := \frac{Em}{2 + 2v}$$

> alpha:=(rho*Vs)/(3*MWs);

$$\alpha := \frac{1}{3} \frac{\rho \ Vs}{MWs}$$

> K1:=(Vs*K)/(R*T);

$$KI := \frac{Vs Em}{(3-6 \text{ V}) R T}$$

> K2:=3*alpha*(1+traps);

$$K2 := \frac{\rho \ Vs \ (1 + traps)}{MWs}$$

> K1*K2;

$$\frac{Vs^2 Em \rho (1 + traps)}{(3 - 6 v) R T MWs}$$

> A1:=(1-2*nu);

$$AI := 1 - 2 v$$

> A2:=2*(1+nu)*alpha*(1+traps);

$$A2 := \frac{2}{3} \frac{(1+v) \rho Vs (1+traps)}{MWs}$$

> A3:=A2*c0;

$$A3 := \frac{2}{3} \frac{(1+v) \rho Vs (1+traps) c\theta}{MWs}$$

> B1:=3*K;

$$B1 := 3 \frac{Em}{3 - 6 v}$$

> B2:=9*K*alpha*(1+traps);

$$B2 := 3 \frac{Em \rho Vs (1 + traps)}{(3 - 6 \vee) MWs}$$

> B3:=B2*c0;

$$B3 := 3 \frac{Em \rho \ Vs (1 + traps) \ cO}{(3 - 6 \ v) \ MWs}$$

>et(x);
$$\left(1 + \sqrt{1 + 2 \frac{Vs^2 Em p (1 + traps) CI x}{(3 - 6 v) R T MWs}} + 2 \frac{Vs^2 Em p (1 + traps) CI C2}{(3 - 6 v) R T MWs}}\right) (3 - 6 v) R T MWs / (Vs^2 Em p (1 + traps))$$
>e2(x);
$$- \left(-1 + \sqrt{1 + 2 \frac{Vs^2 Em p (1 + traps) CI x}{(3 - 6 v) R T MWs}} + 2 \frac{Vs^2 Em p (1 + traps) CI C2}{(3 - 6 v) R T MWs}\right) (3 - 6 v) R T MWs / (Vs^2 Em p (1 + traps))$$
>e1(x);
$$\left(\sqrt{1 + 2 \frac{Vs^2 Em p (1 + traps) CI x}{(3 - 6 v) R T MWs}} + 2 \frac{Vs^2 Em p (1 + traps) CI C2}{(3 - 6 v) R T MWs} + \frac{C3 Vs Em}{(3 - 6 v) R T}\right)$$

$$- \frac{1}{3} \left(2 \frac{Vs^2 Em p (1 + traps) CI x}{(3 - 6 v) R T MWs} + 2 \frac{Vs^2 Em p (1 + traps) CI C2}{(3 - 6 v) R T MWs} - \frac{C3 Vs Em}{(3 - 6 v) R T}\right)$$

$$- \frac{1}{3} \left(2 \frac{(1 + v) p^2 Vs^3 (1 + traps)^2 x Em CI}{MWs^2 (3 - 6 v) R T} + \frac{2}{3 \frac{(1 + v) p Vs (1 + traps) \sqrt{\%1}}{MWs} + \frac{4 \sqrt{\%1} (1 + v) p^2 Vs^3 (1 + traps)^2 Em CI x}{3 \frac{MWs^2 (3 - 6 v) R T}{3 \frac{(1 + traps)^3 CO x Em^2 CI}{3 \frac{(1 + traps)^3 CO x E$$

$$\frac{1}{3} \left(-2 \frac{(1+v) \rho^{2} V s^{3} (1 + traps)^{2} x Em _CI}{MWs^{2} (3 - 6 v) R T} + \frac{2}{3} \frac{(1+v) \rho V s (1 + traps) \sqrt{\%1}}{MWs} \right)$$

$$+ \frac{4}{3} \frac{\sqrt{\%1} (1+v) \rho^{2} V s^{3} (1 + traps)^{2} Em _CI x}{MWs^{2} (3 - 6 v) R T}$$

$$+ \frac{4}{3} \frac{\sqrt{\%1} (1+v) \rho^{2} V s^{3} (1 + traps)^{2} Em _CI _C2}{MWs^{2} (3 - 6 v) R T}$$

$$+ 2 \frac{(1+v) \rho^{3} V s^{5} (1 + traps)^{3} co x Em^{2} _CI}{MWs^{3} (3 - 6 v)^{2} R^{2} T^{2}} - \frac{\sqrt{\%1} \rho V s (1 + traps)}{MWs}$$

$$- 2 \frac{\sqrt{\%1} \rho^{2} V s^{3} (1 + traps)^{2} Em _CI x}{MWs^{2} (3 - 6 v) R T} - 2 \frac{\sqrt{\%1} \rho^{2} V s^{3} (1 + traps)^{2} Em _CI _C2}{MWs^{2} (3 - 6 v) R T}$$

$$+ 3 \frac{-C3 x V s^{4} Em^{2} \rho^{2} (1 + traps)^{2} _CI}{(3 - 6 v)^{2} R^{2} T^{2} MWs^{2}} + 3 \frac{-C4 (1 - 2 v) V s^{4} Em^{2} \rho^{2} (1 + traps)^{2} _CI}{(3 - 6 v)^{2} R^{2} T^{2} MWs^{2}}$$

$$- 2 \frac{\sqrt{(3 - 6 v)^{2} R^{2} T^{2} MWs^{2}}}{(3 - 6 v) R T MWs^{2}} + 2 \frac{V s^{2} Em \rho (1 + traps) _CI _C2}{(3 - 6 v) R T MWs}$$

> S1(x);

-
$$(MWs _C3 \ Vs \ Em - 3 \ R \ T \ MWs + 6 \ R \ T \ MWs \ v + Em \ \rho \ Vs^2 \ c0 + Em \ \rho \ Vs^2 \ c0 \ traps)/($$
 $(-1 + 2 \ v) \ Vs \ MWs)$

> S2(x);

-
$$(MWs _C3 \ Vs \ Em - 3 \ R \ T \ MWs + 6 \ R \ T \ MWs \ v + Em \ \rho \ Vs^2 \ c0 + Em \ \rho \ Vs^2 \ c0 \ traps)/((-1 + 2 \ v) \ Vs \ MWs)$$

> S1(x)-S2(x);

0

Note #4: The stresses are same for each solution root, and do not depend on the position x! The stresses are therefore constant throughout the body. We will solve for $_C3$ in terms of the boundary stress, S0.

> _C3:=simplify(solve(S1(x)=S0, C3));

$$C3 := -(-3 R T MWs + 6 R T MWs v + Em \rho Vs^2 c0 + Em \rho Vs^2 c0 traps - S0 Vs MWs + 2 S0 Vs MWs v) / (Em MWs Vs)$$

> S1(x);

S0

> S2(x);

> S1:=x->S0;

 $S1 := x \rightarrow S0$

> S2:=x->S0;

 $S2 := x \rightarrow S0$

Note #5: Now we will introduce numerical values for the material properties.

The values selected are typical for internal hydrogen as a solute in AISI 4340 steel.

> S0:=0.0;

S0 := 0

> Ds:=1e-9;

 $Ds := .1 \ 10^{-8}$

> Vs:=2.02e-6;

 $Vs := .202 \ 10^{-5}$

> traps:=19;

traps := 19

> c0:=0.0;

c0 := 0

> MWs:=0.00100797;

MWs := .00100797

> Em:=200e9;

 $Em := .200 \ 10^{12}$

> nu:=0.3;

v := .3

> rho:=7800;

 $\rho := 7800$

> T:=293;

T := 293

> R:=8.31432;

R := 8.31432

> K;

 $.166666666666667\ 10^{12}$

> lambda;

 $.115384615384616\ 10^{12}$

> mu;

 $.769230769230769\ 10^{11}$

> alpha;

5.21047253390476

> Rh:=R/MWs;

```
Rh := 8248.57882675080
> K1;
                                       138.199274509089
> K2;
                                       312.628352034286
> K1*K2;
                                       43205.0114421104
> 1/(K1*K2);
                                     .0000231454631447068
> A1;
                                                .4
> A2;
                                       270.944571763048
> A3;
                                                0
> B1;
                                     .500000000000001 \ 10^{12}
> B2;
                                    .156314176017143\ 10^{15}
> B3;
                                                0
> _C3;
                                      .00723592800000000
> S1(x);
                                                0
> S2(x);
                                                0
  Note #6: Now let's solve for the integration constants: _Cl, _C2, and _C4, using the
           boundary data.
> c1(x);
               .0000231454631447068 + .0000231454631447068
                     \int 1 + 86410.0228842208 \ C1 \ x + 86410.0228842208 \ C1 \ C2
> c2(x);
               .0000231454631447068 - .0000231454631447068
                     \sqrt{1 + 86410.0228842208} _C1 x + 86410.0228842208 _C1 _C2
> evalf(c1(x),5);
                .000023146 + .000023146 \sqrt{1. + 86408} \_C1 x + 86408 \_C1 \_C2
```

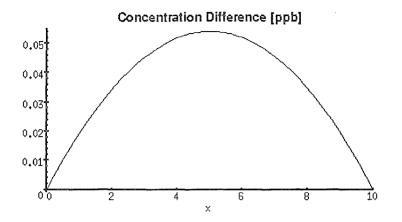
> eqnset1:={0=c1(0),1e-7=c1(0.10)};

0.04

0.02

> p2:=plot(1e9*(c2(0)+((c2(0.10)-c2(0))/0.10)*x/100-c2(x/100)),x=0..10,title='Concentration Difference [ppb]'):

> .

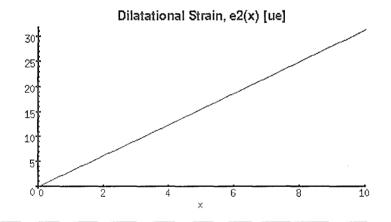


> e2(x);

 $-.0072359279999998\sqrt{1 - .0862233555828494 x} + .00723592799999998$

 $> p3{:=}plot(1e6*e2(x/100),x{=}0..10,title='Dilatational Strain, e2(x)~[ue]'){:}$

> p3;



> u2(x);

.00241197600000003 x + .0186490538338527 $\sqrt{1 - .0862233555828494}$ x

 $-.00160798400000002\sqrt{1-.0862233555828494 x} +.99999999999999992$ _C4

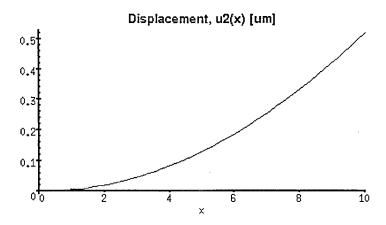
> _C4:=solve(u2(0)=0, C4);

$$_C4 := -.0186490538338528$$

> u2(x);

 > p4:=plot(1e6*u2(x/100),x=0..10,title='Displacement, u2(x) [um]'):

> p4;



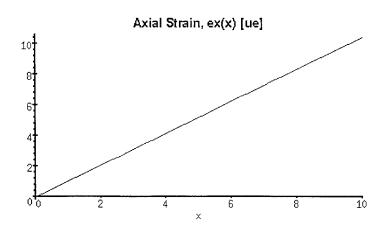
> ex:=unapply(diff(u2(x),x),x);

$$ex := x \to .00241197600000003 - .000803991999999991 \frac{1}{\sqrt{1 - .0862233555828494 \ x}}$$

$$+ .0000693228881017671 \frac{x}{\sqrt{1 - .0862233555828494 \ x}}$$

$$- .00160798400000002 \sqrt{1 - .0862233555828494 \ x}$$

> ex(x);



1. MAPLE PROGRAM DIRECTORY AND FILE NAME: /afs/nd.edu/user4/jthomas5/Maple/Transport/1DSSsol#1c.ms

2. ORIGINATOR:

Dr. James P. Thomas University of Notre Dame Department of Aerospace and Mechanical Engineering 374 Fitzpatrick Hall Notre Dame, IN 46556-5637 (219) 631-9371

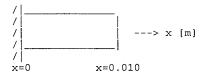
3. HISTORY:

Written: 21-Jan-1995

Latest Revision: 13-Feb-1995

4. PROGRAM DESCRIPTION:

One-dimensional, steady-state analysis of the fully coupled solute transport and trapping equations using "plane stress" constitutive equations for stress. The particular problem solved is shown below:



Deformation variable boundary conditions: u(0)=0, S(0.10)=0Concentration variable boundary conditions: c(0)=0, c(0.10)=1e-7Reference concentration level: c0=0.0Trapping parameter: traps=499 (high trapping)

The equations used in the analysis are:

- dJ/dx=0 (steady-state diffusion equation)
- 2. J=Ds*(K1*c*de/dx-dc/dx) (mass flux)
- 3. dS/dx=0 (steady-state deformation equation with zero body forces)
- 4. S=B1*e-B2*c+B3 (axial stress)
- 5. de/dx-K2*dc/dx=0 (dilational strain gradient)
- 6. e=A1*du/dx+A2*c-A3 (dilatational strain)

The constants used in the plane stress analysis are defined below:

Ds:=lattice diffusivity [m^2/sec]

Delc:= c-c0 [1]

```
K1 := (Vs*K) / (R*T)
Vs:= partial molar volume of solute [m^3/mol solute]
K:= bulk modulus=Em/(3*(1-2*nu)) [N/m^2]
Em:= Modulus of Elasticity [N/m^2]
nu:= Poisson's ratio [1]
R:= universal gas constant=8.31432 [J/mol-K]
T:= temperature [K]
B1:=3*K
B2:=9*K*alpha*(1+traps)
B3:=B2*co
traps:= Csr*Kr=trapping constant [1]
Csr:= saturation trap concentration [1]
Kr:= equilibrium trapping constant [1]
c0:= reference solute concentration [1]
K2:=3*alpha*(1+traps)
alpha:= (rho*Vs)/(3*MWs)=solute concentration expansion coefficient [m/m/Delc]
rho:= mass density of the solid [kg/m^3]
MWs:= molecular weight of the solute [kg/mol]
```

A1:=1-2*nu A2:=2*(1+nu)*alpha*(1+traps) A3:=A2*c0

5. REQUIRED INPUTS:

Ds:=lattice diffusivity [m^2/sec]
Vs:= partial molar volume of solute [m^3/mol solute]
traps:= Csr*Kr=trapping constant [1]
c0:= reference solute concentration [1]
MWs:= molecular weight of the solute [kg/mol]

Em:= Modulus of Elasticity [Pa]
nu:= Poisson's ratio [1]
rho:= mass density of the solid [kg/m^3]

T:= temperature [K]

6. SYMBOLIC ANALYSIS:

> Digits:=trunc(evalhf(Digits));

$$Digits := 15$$

> J:=Ds*(K1*K2*c(x)-1)*diff(c(x),x);

$$J := Ds (K1 \ K2 \ c(x) - 1) \left(\frac{\partial}{\partial x} c(x) \right)$$

> deqn:=diff(J,x)=0;

$$deqn := Ds K1 K2 \left(\frac{\partial}{\partial x} c(x)\right)^{2} + Ds (K1 K2 c(x) - 1) \left(\frac{\partial^{2}}{\partial x^{2}} c(x)\right) = 0$$

> csoln:=dsolve(deqn,c(x),explicit);

$$csoln := c(x) = -\frac{CI\left(-\frac{1}{CI} - \frac{\sqrt{1 + 2 KI K2 _CI x + 2 KI K2 _CI _C2}}{CI}\right)}{KI K2},$$

$$c(x) = -\frac{CI\left(-\frac{1}{CI} + \frac{\sqrt{1 + 2 KI K2 _CI x + 2 KI K2 _CI _C2}}{CI}\right)}{KI K2}$$

Note #1: There are apparently two "roots" to the solution for c(x). The boundary conditions will be used to select the proper one.

> c1:=unapply(simplify(op(2,csoln[1])),x);

$$c1 := x \to \frac{1 + \sqrt{1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2}}{K1 K2}$$

> c1(x);

$$\frac{1 + \sqrt{1 + 2 KI K2 _CI x + 2 KI K2 _CI _C2}}{KI K2}$$

> c2:=unapply(simplify(op(2,csoln[2])),x);

$$c2 := x \to -\frac{-1 + \sqrt{1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2}}{K1 K2}$$

> c2(x);

$$-\frac{-1 + \sqrt{1 + 2 KI K2 _CI x + 2 KI K2 _CI _C2}}{KI K2}$$

> J1:=unapply(simplify(Ds*(K1*K2*c1(x)-1)*diff(c1(x),x),x));

$$J1 := () \rightarrow Ds _C1$$

> J1(x);

> J2:=unapply(simplify(Ds*(K1*K2*c2(x)-1)*diff(c2(x),x)));

$$J2 := () \rightarrow Ds _C1$$

> J2(x);

Note #2: The above results show that both mass flux solutions are identical and equal to Ds*_C1 where _C1 is a constant of integration!

> ex1:=dsolve(diff(e(x),x)-K2*diff(c1(x),x)=0,e(x));

$$ex1 := e(x) = \frac{\sqrt{1 + 2 K1 K2 C1 x + 2 K1 K2 C1 C2} + C3 K1}{K1}$$

> e1:=unapply(simplify(op(2,ex1)),x);

fy(op(2,ex1)),x);

$$e1 := x \rightarrow \frac{\sqrt{1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2} + _C3 K1}{K1}$$

> e1(x);

$$\int 1 + 2 KI K2 _CI x + 2 KI K2 _CI _C2 + _C3 KI$$

$$KI$$

> ex2:=dsolve(diff(e(x),x)-K2*diff(c2(x),x)=0,e(x));

$$ex2 := e(x) = -\frac{\sqrt{1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2} - _C3 K1}{K1}$$

$$explify(op(2,ex2)),x);$$

> e2:=unapply(simplify(op(2,ex2)),x);

ify(op(2,ex2)),x);

$$e2 := x \rightarrow -\frac{\sqrt{1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2} - _C3 K1}{K1}$$

> e2(x);

$$-\frac{\int 1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2 - _C3 K1}{K1}$$

> ux1:=dsolve(A1*diff(u(x),x)+A2*c1(x)-A3-e1(x)=0,u(x));

$$uxI := u(x) = \frac{1}{3} \left(-3 A2 x KI K2 _CI - A2 \sqrt{\%1} - 2 \sqrt{\%1} A2 KI K2 _CI x \right)$$

$$- 2 \sqrt{\%1} A2 KI K2 _CI _C2 + 3 A3 x KI^2 K2^2 _CI + \sqrt{\%1} K2 + 2 \sqrt{\%1} K2^2 KI _CI x$$

$$+ 2 \sqrt{\%1} K2^2 KI _CI _C2 + 3 _C3 x KI^2 K2^2 _CI + 3 _C4 AI KI^2 K2^2 _CI \right) / (AI KI^2 K2^2 _CI)$$

$$%1 := 1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2$$

> u1:=unapply(simplify(op(2,ux1)),x);

$$ul := x \to \frac{1}{3} \left(-3 \text{ A2 } x \text{ K1 K2 _C1 - A2} \sqrt{1 + 2 \text{ K1 K2 _C1 } x + 2 \text{ K1 K2 _C1 _C2}} \right.$$

$$- 2 \sqrt{1 + 2 \text{ K1 K2 _C1 } x + 2 \text{ K1 K2 _C1 _C2}} \text{ A2 K1 K2 _C1 } x$$

$$- 2 \sqrt{1 + 2 \text{ K1 K2 _C1 } x + 2 \text{ K1 K2 _C1 _C2}} \text{ A2 K1 K2 _C1 _C2 + 3 A3 } x \text{ K1}^2 \text{ K2}^2 \text{ _C1}}$$

$$+ \sqrt{1 + 2 \text{ K1 K2 _C1 } x + 2 \text{ K1 K2 _C1 _C2}} \text{ K2}$$

$$+ 2 \sqrt{1 + 2 \text{ K1 K2 _C1 } x + 2 \text{ K1 K2 _C1 _C2}} \text{ K2}^2 \text{ K1 _C1 } x$$

$$+ 2 \sqrt{1 + 2 \text{ K1 K2 _C1 } x + 2 \text{ K1 K2 _C1 _C2}} \text{ K2}^2 \text{ K1 _C1 _C2 + 3 _C3 } x \text{ K1}^2 \text{ K2}^2 \text{ _C1}}$$

$$+ 3 \text{_C4 A1 K1}^2 \text{ K2}^2 \text{_C1} / (\text{A1 K1}^2 \text{ K2}^2 \text{_C1})$$

> u1(x);

$$\frac{1}{3} \left(-3 A2 x K1 K2 _C1 - A2 \sqrt{\%1} - 2 \sqrt{\%1} A2 K1 K2 _C1 x - 2 \sqrt{\%1} A2 K1 K2 _C1 _C2 \right)$$

$$+ 3 A3 x K1^{2} K2^{2} _C1 + \sqrt{\%1} K2 + 2 \sqrt{\%1} K2^{2} K1 _C1 x + 2 \sqrt{\%1} K2^{2} K1 _C1 _C2$$

$$+ 3 _C3 x K1^{2} K2^{2} _C1 + 3 _C4 A1 K1^{2} K2^{2} _C1 \right) / (A1 K1^{2} K2^{2} _C1)$$

$$%1 := 1 + 2 KI K2 _CI x + 2 KI K2 _CI _C2$$

> ux2:=dsolve(A1*diff(u(x),x)+A2*c2(x)-A3-e2(x)=0,u(x));

$$ux2 := u(x) = -\frac{1}{3} \left(3 A2 x K1 K2 _C1 - A2 \sqrt{\%1} - 2 \sqrt{\%1} A2 K1 K2 _C1 x \right)$$

$$-2 \sqrt{\%1} A2 K1 K2 _C1 _C2 - 3 A3 x K1^2 K2^2 _C1 + \sqrt{\%1} K2 + 2 \sqrt{\%1} K2^2 K1 _C1 x$$

$$+2 \sqrt{\%1} K2^2 K1 _C1 _C2 - 3 _C3 x K1^2 K2^2 _C1 - 3 _C4 A1 K1^2 K2^2 _C1 \right) / (A1 K1^2 K2^2 _C1)$$

$$%1 := 1 + 2 KI K2 CI x + 2 KI K2 CI C2$$

> u2:=unapply(simplify(op(2,ux2)),x);

$$u2 := x \rightarrow \frac{1}{3} \left(-3 \ A2 \ x \ K1 \ K2 \ C1 + A2 \sqrt{1 + 2 \ K1 \ K2 \ C1 \ x + 2 \ K1 \ K2 \ C1 \ C2} \right)$$

$$+ 2\sqrt{1 + 2 KI K2 _CI x + 2 KI K2 _CI _C2} A2 KI K2 _CI x$$

$$+ 2\sqrt{1 + 2 KI K2 _CI x + 2 KI K2 _CI _C2} A2 KI K2 _CI _C2 + 3 A3 x KI^2 K2^2 _CI$$

$$- \sqrt{1 + 2 KI K2 _CI x + 2 KI K2 _CI _C2} K2$$

$$- 2\sqrt{1 + 2 KI K2 _CI x + 2 KI K2 _CI _C2} K2^2 KI _CI x$$

$$- 2\sqrt{1 + 2 KI K2 _CI x + 2 KI K2 _CI _C2} K2^2 KI _CI _C2 + 3 _C3 x KI^2 K2^2 _CI$$

$$+ 3 _C4 AI KI^2 K2^2 _CI) / (AI KI^2 K2^2 _CI)$$

> u2(x);

$$\frac{1}{3} \left(-3 A2 x K1 K2 _C1 + A2 \sqrt{\%1} + 2 \sqrt{\%1} A2 K1 K2 _C1 x + 2 \sqrt{\%1} A2 K1 K2 _C1 _C2 + 3 A3 x K1^2 K2^2 _C1 - \sqrt{\%1} K2 - 2 \sqrt{\%1} K2^2 K1 _C1 x - 2 \sqrt{\%1} K2^2 K1 _C1 _C2 + 3 _C3 x K1^2 K2^2 _C1 + 3 _C4 A1 K1^2 K2^2 _C1 \right) / (A1 K1^2 K2^2 _C1)$$

$$%1 := 1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2$$

> S1:=x->simplify(B1*e1(x)-B2*c1(x)+B3);

$$SI := x \rightarrow \text{simplify}(BI \text{ e1}(x) - B2 \text{ c1}(x) + B3)$$

> S1(x);

$$(B1 \ K2 \sqrt{1 + 2 \ K1 \ K2 \ _C1 \ x + 2 \ K1 \ K2 \ _C1 \ _C2} + B1 \ K2 \ _C3 \ K1 - B2$$

$$- B2 \sqrt{1 + 2 \ K1 \ K2 \ _C1 \ x + 2 \ K1 \ K2 \ _C1 \ _C2} + B3 \ K1 \ K2) / (K1 \ K2)$$

> S2:=x->simplify(B1*e2(x)-B2*c2(x)+B3);

$$S2 := x \rightarrow \text{simplify}(B1 \text{ e2}(x) - B2 \text{ c2}(x) + B3)$$

> S2(x);

$$- \left(B1 \ K2 \sqrt{1 + 2 \ K1 \ K2 \ _C1 \ x + 2 \ K1 \ K2 \ _C1 \ _C2} - B1 \ K2 \ _C3 \ K1 + B2 \right) - B2 \sqrt{1 + 2 \ K1 \ K2 \ _C1 \ x + 2 \ K1 \ K2 \ _C1 \ _C2} - B3 \ K1 \ K2 \right) / (K1 \ K2)$$

Note #3: At this point, we will introduce expressions for the constants and then simplify the results before quantifying the constants and determining the constants of integration.

> K:=(Em/(3*(1-2*nu)));

$$K := \frac{Em}{3 - 6 v}$$

> lambda:=(Em*nu)/((1+nu)*(1-2*nu));

$$\lambda := \frac{Em \, v}{(1+v) (1-2 \, v)}$$

> mu:=Em/(2*(1+nu));

$$\mu := \frac{Em}{2 + 2 v}$$

> alpha:=(rho*Vs)/(3*MWs);

$$\alpha := \frac{1}{3} \frac{\rho \ Vs}{MWs}$$

> K1:=(Vs*K)/(R*T);

$$K1 := \frac{Vs Em}{(3-6 \text{ V}) R T}$$

> K2:=3*alpha*(1+traps);

$$K2 := \frac{\rho \ Vs (1 + traps)}{MWs}$$

> K1*K2;

$$\frac{Vs^2 Em \rho (1 + traps)}{(3 - 6 v) R T MWs}$$

> A1:=(1-2*nu);

$$A1 := 1 - 2 v$$

> A2:=2*(1+nu)*alpha*(1+traps);

$$A2 := \frac{2}{3} \frac{(1+v) \rho Vs (1+traps)}{MWs}$$

> A3:=A2*c0;

$$A3 := \frac{2}{3} \frac{(1+v) \rho Vs (1 + traps) cO}{MWs}$$

> B1:=3*K;

$$BI := 3 \frac{Em}{3 - 6 v}$$

> B2:=9*K*alpha*(1+traps);

$$B2 := 3 \frac{Em \rho Vs (1 + traps)}{(3 - 6 v) MWs}$$

> B3:=B2*c0;

$$B3 := 3 \frac{Em \rho Vs (1 + traps) cO}{(3 - 6 v) MWs}$$

> c1(x);
$$\left(1 + \sqrt{1 + 2 \frac{Vs^2 Em \rho (1 + traps) CI x}{(3 - 6 v) R T MWs}} + 2 \frac{Vs^2 Em \rho (1 + traps) CI C2}{(3 - 6 v) R T MWs}\right) (3 - 6 v) R T$$

$$MWs/(Vs^2 Em \rho (1 + traps))$$
> c2(x);
$$- \left(-1 + \sqrt{1 + 2 \frac{Vs^2 Em \rho (1 + traps) CI x}{(3 - 6 v) R T MWs}} + 2 \frac{Vs^2 Em \rho (1 + traps) CI C2}{(3 - 6 v) R T MWs}\right) (3 - 6 v) R T$$

$$MWs/(Vs^2 Em \rho (1 + traps))$$
> c1(x);
$$\left(\sqrt{1 + 2 \frac{Vs^2 Em \rho (1 + traps) CI x}{(3 - 6 v) R T MWs}} + 2 \frac{Vs^2 Em \rho (1 + traps) CI C2}{(3 - 6 v) R T MWs} + \frac{C3 Vs Em}{(3 - 6 v) R T}\right)$$

$$(3 - 6 v) R T/(Vs Em)$$
> c2(x);
$$- \left(\sqrt{1 + 2 \frac{Vs^2 Em \rho (1 + traps) CI x}{(3 - 6 v) R T MWs}} + 2 \frac{Vs^2 Em \rho (1 + traps) CI C2}{(3 - 6 v) R T MWs} - \frac{C3 Vs Em}{(3 - 6 v) R T}\right)$$

$$- (3 - 6 v) R T/(Vs Em)$$
> c1(x);
$$\frac{1}{3} \left(-2 \frac{(1 + v) \rho^2 Vs^3 (1 + traps)^2 x Em CI}{MWs^2 (3 - 6 v) R T} - \frac{2}{3} \frac{(1 + v) \rho Vs (1 + traps) \sqrt{21}}{MWs} \right)$$

$$- \frac{4 \sqrt{361} (1 + v) \rho^2 Vs^3 (1 + traps)^2 Em CI x}{MWs^2 (3 - 6 v) R T}$$

$$- \frac{4 \sqrt{361} (1 + v) \rho^2 Vs^3 (1 + traps)^2 Em CI C2}{MWs^2 (3 - 6 v) R T} + 2 \frac{4 \sqrt{361} \rho^2 Vs^3 (1 + traps)^2 Em CI C2}{MWs^3 (3 - 6 v)^2 R^2 T^2} + \frac{\sqrt{361} \rho^2 Vs^3 (1 + traps)^2 Em CI C2}{MWs^2 (3 - 6 v) R T}$$

$$+ 2 \frac{\sqrt{361} \rho^2 Vs^3 (1 + traps)^2 Em CI x}{MWs^2 (3 - 6 v) R T} + 2 \frac{\sqrt{361} \rho^2 Vs^3 (1 + traps)^2 Em CI C2}{MWs^2 (3 - 6 v) R T}$$

$$+ 3 \frac{C3 x Vs^4 Em^2 \rho^2 (1 + traps)^2 CI}{(3 - 6 v)^2 R^2 T^2 MWs^2} + 3 \frac{C4 (1 - 2 v) Vs^4 Em^2 \rho^2 (1 + traps)^2 CI}{(3 - 6 v)^2 R^2 T^2 MWs^2}$$

$$(3 - 6 v)^2 R^2 T^2 MWs^2 / \left((1 - 2 v) Vs^4 Em^2 \rho^2 (1 + traps)^2 CI\right)$$

$$\%1 := 1 + 2 \frac{Vs^2 Em \rho (1 + traps) _C1 x}{(3 - 6 v) R T MWs} + 2 \frac{Vs^2 Em \rho (1 + traps) _C1 _C2}{(3 - 6 v) R T MWs}$$

$$\frac{1}{3} \left(-2 \frac{(1+v) \rho^{2} V s^{3} (1 + traps)^{2} x Em _C1}{MW s^{2} (3 - 6 v) R T} + \frac{2}{3} \frac{(1+v) \rho V s (1 + traps) \sqrt{61}}{MW s} \right)$$

$$+ \frac{4}{3} \frac{\sqrt{61} (1+v) \rho^{2} V s^{3} (1 + traps)^{2} Em _C1 x}{MW s^{2} (3 - 6 v) R T}$$

$$+ \frac{4}{3} \frac{\sqrt{61} (1+v) \rho^{2} V s^{3} (1 + traps)^{2} Em _C1 _C2}{MW s^{2} (3 - 6 v) R T}$$

$$+ 2 \frac{(1+v) \rho^{3} V s^{5} (1 + traps)^{3} co x Em^{2} _C1}{MW s^{3} (3 - 6 v)^{2} R^{2} T^{2}} - \frac{\sqrt{61} \rho V s (1 + traps)}{MW s}$$

$$- 2 \frac{\sqrt{61} \rho^{2} V s^{3} (1 + traps)^{2} Em _C1 x}{MW s^{2} (3 - 6 v) R T} - 2 \frac{\sqrt{61} \rho^{2} V s^{3} (1 + traps)^{2} Em _C1 _C2}{MW s^{2} (3 - 6 v) R T}$$

$$+ 3 \frac{-C3 x V s^{4} Em^{2} \rho^{2} (1 + traps)^{2} _C1}{(3 - 6 v)^{2} R^{2} T^{2} MW s^{2}} + 3 \frac{-C4 (1 - 2 v) V s^{4} Em^{2} \rho^{2} (1 + traps)^{2} _C1}{(3 - 6 v)^{2} R^{2} T^{2} MW s^{2}}$$

$$- (3 - 6 v)^{2} R^{2} T^{2} MW s^{2} / ((1 - 2 v) V s^{4} Em^{2} \rho^{2} (1 + traps)^{2} _C1)$$

$$\%1 := 1 + 2 \frac{Vs^2 \ Em \ \rho \ (1 + traps) \ _C1 \ x}{(3 - 6 \ v) \ R \ T \ MWs} + 2 \frac{Vs^2 \ Em \ \rho \ (1 + traps) \ _C1 \ _C2}{(3 - 6 \ v) \ R \ T \ MWs}$$

> S1(x);

-
$$(MWs _C3 \ Vs \ Em - 3 \ R \ T \ MWs + 6 \ R \ T \ MWs \ v + Em \ \rho \ Vs^2 \ c0 + Em \ \rho \ Vs^2 \ c0 \ traps)/((-1 + 2 \ v) \ Vs \ MWs)$$

> S2(x);

$$-(MWs _C3 \ Vs \ Em - 3 \ R \ T \ MWs + 6 \ R \ T \ MWs \ v + Em \ \rho \ Vs^2 \ c0 + Em \ \rho \ Vs^2 \ c0 \ traps) / ($$

$$(-1 + 2 \ v) \ Vs \ MWs)$$

> S1(x)-S2(x);

0

Note #4: The stresses are same for each solution root, and do not depend on the position x! The stresses are therefore constant throughout the body. We will solve for _C3 in terms of the boundary stress, S0.

> _C3:=simplify(solve(S1(x)=S0,_C3));

$$C3 := -(-3 R T MWs + 6 R T MWs v + Em \rho Vs^2 cO + Em \rho Vs^2 cO traps - SO Vs MWs + 2 SO Vs MWs v) / (Em Vs MWs)$$

> S1(x); *S0* > S2(x); S0 > S1:=x->S0; $S1 := x \rightarrow S0$ > S2:=x->S0; $S2 := x \rightarrow S0$ Note #5: Now we will introduce numerical values for the material properties. The values selected are typical for internal hydrogen as a solute in AISI 4340 steel. > S0:=0.0; S0 := 0> Ds:=1e-9; $Ds := .1 \ 10^{-8}$ > Vs:=2.02e-6; $Vs := .202 \ 10^{-5}$ > traps:=499; traps := 499> c0:=0.0; c0 := 0> MWs:=0.00100797; MWs := .00100797> Em:=200e9; $Em := .200 \ 10^{12}$ > nu:=0.3; v := .3> rho:=7800; $\rho := 7800$ > T:=293; T := 293> R:=8.31432; R := 8.31432> K; .16666666666667 1012 > lambda; $.115384615384616\ 10^{12}$

 $.769230769230769\ 10^{11}$

> mu;

> alpha;

```
5.21047253390476
> Rh:=R/MWs:
                                      Rh := 8248.57882675080
> K1;
                                         138.199274509089
> K2;
                                         7815.70880085715
> K1*K2;
                                       .108012528605276\ 10^{7}
> 1/(K1*K2);
                                       .925818525788271\ 10^{-6}
> A1;
                                                  .4
> A2;
                                         6773.61429407620
> A3;
                                                  0
> B1;
                                      .500000000000001 \ 10^{12}
> B2;
                                      .390785440042860\ 10^{16}
> B3;
                                                  0
> _C3;
                                       .00723592800000000
> S1(x);
                                                  0
> S2(x);
                                                  0
  Note \#6: Now let's solve for the integration constants: _C1, _C2, and _C4, using the
           boundary data.
> c1(x);
           .925818525788271\ 10^{-6} + .925818525788271\ 10^{-6}
                  \sqrt{1 + .216025057210552 \cdot 10^7} _C1 x + .216025057210552 10^7 _C1 _C2
> c2(x);
           .925818525788271\ 10^{-6} - .925818525788271\ 10^{-6}
```

> evalf(c1(x),5);

 $\sqrt{1 + .216025057210552 \cdot 10^7 - CI \cdot x + .216025057210552 \cdot 10^7 - CI - C2}$

.92584
$$10^{-6}$$
 + .92584 10^{-6} $\sqrt{1. + .21602}$ 10^{7} _C1 x + .21602 10^{7} _C1 _C2

> eqnset1:={0=c1(0),1e-7=c1(0.10)};

$$eqnset1 := \begin{cases} .1 \ 10^{-6} = .925818525788271 \ 10^{-6} + .925818525788271 \ 10^{-6} \\ \sqrt{1 + 216025.057210552} \ _CI + .216025057210552 \ 10^{7} \ _CI \ _C2 \ , 0 = .925818525788271 \ 10^{-6} \end{cases}$$

 $+.925818525788271\ 10^{-6} \sqrt{1+.216025057210552\ 10^{7}\ CI\ C2}$

> varset1:={_C1,_C2};

$$varset1 := \{ _C1, _C2 \}$$

> solnset1:=solve(eqnset1,varset1);

> eqnset2:={0=c2(0),1e-7=c2(0.10)};

$$eqnset2 := \left\{ 0 = .925818525788271 \ 10^{-6} \right. \\ \left. - .925818525788271 \ 10^{-6} \sqrt{1 + .216025057210552} \ 10^{7} \ _CI \ _C2 \right\}, .1 \ 10^{-6} = .925818525788271 \ 10^{-6} - .925818525788271 \ 10^{-6} \\ \left. \sqrt{1 + 216025.057210552} \ _CI + .216025057210552 \ 10^{7} \ _CI \ _C2 \right\}$$

> varset2:={_C1,_C2};

$$varset2 := \{ _C1, _C2 \}$$

> solnset2:=solve(eqnset2,varset2);

$$solnset2 := \{ CI = -.945993735697361 \ 10^{-6}, C2 = 0 \}$$

> assign(solnset2);

Note #7: The solution c1(x) is not capable of satisfying the boundary conditions, as evidenced by the lack of solution for the given boundary conditions. The concentration function c2(x) is the correct solution in this case!

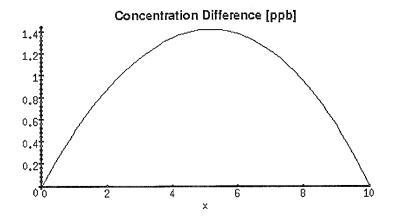
> c2(x);

$$.925818525788271 \ 10^{-6} - .925818525788271 \ 10^{-6} \sqrt{1 - 2.04358350874846} \ x$$

> p1:=plot(1e6*c2(x/100),x=0..10,title='Concentration, c2(x) [ppm]'):

> p1;

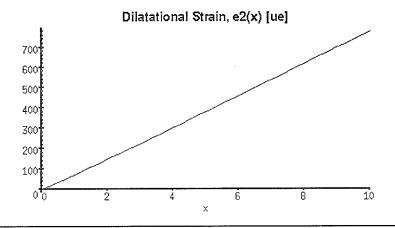
> p2:=plot(1e9*(c2(0)+((c2(0.10)-c2(0))/0.10)*x/100-c2(x/100)),x=0..10,title='Concentration Difference [ppb]'): > p2; >



> e2(x);

 $-.00723592799999998 \int 1 - 2.04358350874846 x + .00723592799999998$

> p3:=plot(1e6*e2(x/100),x=0..10,title='Dilatational Strain, e2(x) [ue]'):



> u2(x);

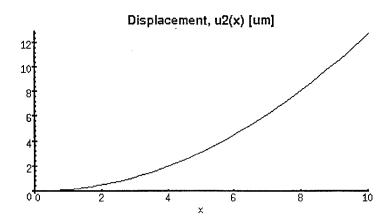
> _C4:=solve(u2(0)=0,_C4);

C4 := -.000786845261334469

> u2(x);

 $.00241197599999993\;x + .000786845261334466 \sqrt{1 - 2.04358350874846}\;x$ $-.0016079839999998\sqrt{1-2.04358350874846}\,x\,x-.000786845261334466$ > p4:=plot(1e6*u2(x/100),x=0..10,title='Displacement, u2(x) [um]'): > p4; >





> ex:=unapply(diff(u2(x),x),x);

$$ex := x \to .00241197599999993 - .000803991999999993 \frac{1}{\sqrt{1 - 2.04358350874846 \, x}}$$

$$+ .00164302479236567 \frac{x}{\sqrt{1 - 2.04358350874846 \, x}} \\ - .00160798399999998 \sqrt{1 - 2.04358350874846 \, x}$$

> ex(x);

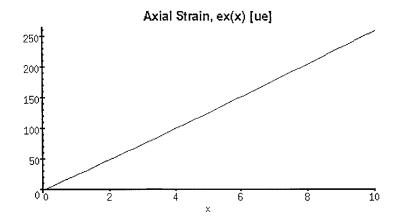
.0024119759999993 - .00080399199999993
$$\frac{1}{\sqrt{1 - 2.04358350874846 \, x}}$$

$$+ .00164302479236567 \frac{x}{\sqrt{1 - 2.04358350874846 \, x}}$$

$$- .00160798399999998 \sqrt{1 - 2.04358350874846 \, x}$$

> p5:=plot(1e6*ex(x/100),x=0..10,title='Axial Strain, ex(x) [ue]'):

> p5; >



-.945993735697361 10-15

1. MAPLE PROGRAM DIRECTORY AND FILE NAME: /afs/nd.edu/user4/jthomas5/Maple/Transport/1DSSsol#1d.ms

2. ORIGINATOR: Dr. James P. Thomas

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3. HISTORY:

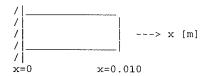
Written: June-1994

Latest Revision: 13-Feb-1995

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4. PROGRAM DESCRIPTION:

One-dimensional, steady-state analysis of the fully coupled solute transport and trapping equations using "plane stress" constitutive equations for stress. The particular problem solved is shown below:



Deformation variable boundary conditions: u(0)=0, S(0.10)=0

Concentration variable boundary conditions: J(0)=0, c(0.10)=1e-7

Reference concentration level: c0=0.0

Trapping parameter: traps=19 (low trapping)

The equations used in the analysis are:

- dJ/dx=0 (steady-state diffusion equation)
- 2. J=Ds*(K1*c*de/dx-dc/dx) (mass flux)
- 3. dS/dx=0 (steady-state deformation equation with zero body forces)
- 4. S=B1*e-B2*c+B3 (axial stress)
- 5. de/dx-K2*dc/dx=0 (dilational strain gradient)
- 6. e=A1*du/dx+A2*c-A3 (dilatational strain)

The constants used in the plane stress analysis are defined below:

Ds:=lattice diffusivity [m^2/sec]

K1 := (Vs*K) / (R*T)

Vs:= partial molar volume of solute [m^3/mol solute]

 $K:= bulk modulus=Em/(3*(1-2*nu)) [N/m^2]$

Em:= Modulus of Elasticity [N/m^2]

nu:= Poisson's ratio [1]

R:= universal gas constant=8.31432 [J/mol-K]

T:= temperature [K]

B1:=3*K

B2:=9*K*alpha*(1+traps)

B3:=B2*co

traps:= Csr*Kr=trapping constant [1]

Csr:= saturation trap concentration [1]

Kr:= equilibrium trapping constant [1]

c0:= reference solute concentration [1]

K2:=3*alpha*(1+traps)

alpha:= (rho*Vs)/(3*MWs)=solute concentration expansion coefficient [m/m/Delc]

rho:= mass density of the solid [kg/m^3]

MWs:= molecular weight of the solute [kg/mol]

Delc:= c-c0 [1]

A1:=1-2*nu A2:=2*(1+nu)*alpha*(1+traps) A3:=A2*c0

===============

5. REQUIRED INPUTS:

Ds:=lattice diffusivity [m^2/sec]
Vs:= partial molar volume of solute [m^3/mol solute]
traps:= Csr*Kr=trapping constant [1]
c0:= reference solute concentration [1]
MWs:= molecular weight of the solute [kg/mol]

Em:= Modulus of Elasticity [Pa]
nu:= Poisson's ratio [1]
rho:= mass density of the solid [kg/m^3]

T:= temperature [K]

================

6. SYMBOLIC ANALYSIS:

> restart:

> Digits:=trunc(evalhf(Digits));

$$Digits := 15$$

> J:=Ds*(K1*K2*c(x)-1)*diff(c(x),x);

$$J := Ds (K1 \ K2 \ c(x) - 1) \left(\frac{\partial}{\partial x} c(x) \right)$$

> deqn:=J=0;

$$deqn := Ds (K1 K2 c(x) - 1) \left(\frac{\partial}{\partial x} c(x) \right) = 0$$

> csoln:=dsolve(deqn,c(x),explicit);

$$csoln := c(x) = \frac{1}{K1 \ K2}, c(x) = _C1$$

Note #1: There are apparently two "roots" to the solution for c(x). The first solution is obviously incorrect in that the concentration throughout the rod is defined in terms of the constants K1 and K2. The second solution is adopted as correct.

> c2:=unapply(simplify(op(2,csoln[2])),x);

$$c2 := x \rightarrow _C1$$

> c2(x);

 $_{-}C1$

 $\overline{}$ > J2:=unapply(simplify(Ds*(K1*K2*c2(x)-1)*diff(c2(x),x)));

J2 := 0

> J2(x);

0

Note #2: The above results show that the mass flux is identically zero, as it should be.

> e2:=x->_C2;

$$e2 := x \rightarrow C2$$

> e2(x);

 $_C2$

> ux2:=dsolve(A1*diff(u(x),x)+A2*c2(x)-A3-e2(x)=0,u(x));

$$ux2 := u(x) = \frac{-x A2 _C1 + x A3 + x _C2 + _C3 A1}{A1}$$

> u2:=unapply(simplify(op(2,ux2)),x);

$$u2 := x \to \frac{-x A2 _C1 + x A3 + x _C2 + _C3 A1}{A1}$$

> u2(x);

$$\frac{-x A2 \ _C1 + x A3 + x \ _C2 + \ _C3 \ A1}{A1}$$

> S2:=x->simplify(B1*e2(x)-B2*c2(x)+B3);

$$S2 := x \rightarrow \text{simplify}(B1 \text{ e2}(x) - B2 \text{ c2}(x) + B3)$$

> S2(x);

Note #3: At this point, we will introduce expressions for the constants and then simplify the results before quantifying the constants and determining the constants of integration.

> K:=(Em/(3*(1-2*nu)));

$$K := \frac{Em}{3 - 6 \,\mathrm{V}}$$

> lambda:=(Em*nu)/((1+nu)*(1-2*nu));

$$\lambda := \frac{Em \, V}{(1+V)(1-2 \, V)}$$

> mu:=Em/(2*(1+nu));

$$\mu := \frac{Em}{2+2v}$$

> alpha:=(rho*Vs)/(3*MWs);

$$\alpha := \frac{1}{3} \frac{\rho \ Vs}{MWs}$$

> K1:=(Vs*K)/(R*T);

$$K1 := \frac{Vs Em}{(3-6 v) R T}$$

> K2:=3*alpha*(1+traps);

 $K2 := \frac{\rho \ Vs \ (1 + traps)}{MWs}$

> K1*K2;

 $\frac{Vs^2 Em \rho (1 + traps)}{(3 - 6 v) R T MWs}$

> A1:=(1-2*nu);

 $AI := 1 - 2 \nu$

> A2:=2*(1+nu)*alpha*(1+traps);

$$A2 := \frac{2}{3} \frac{(1+v) \rho Vs (1+traps)}{MWs}$$

> A3:=A2*c0;

$$A3 := \frac{2}{3} \frac{(1+v) \rho \ Vs (1+traps) \ cO}{MWs}$$

> B1:=3*K;

$$BI := 3 \frac{Em}{3 - 6 v}$$

> B2:=9*K*alpha*(1+traps);

$$B2 := 3 \frac{Em \rho Vs (1 + traps)}{(3 - 6 v) MWs}$$

> B3:=B2*c0;

$$B3 := 3 \frac{Em \rho Vs (1 + traps) c0}{(3 - 6 v) MWs}$$

> c2(x);

 $_C1$

> e2(x);

 $_C2$

> u2(x);

$$\left(-\frac{2}{3}\frac{x(1+v)\rho Vs(1+traps)_{C1}}{MWs} + \frac{2}{3}\frac{x(1+v)\rho Vs(1+traps)c0}{MWs} + x_{C2} + C3(1-2v)\right) / (1-2v)$$

> S2(x);

$$\frac{Em\left(-_C2\ MWs + \rho\ Vs\ _C1 + \rho\ Vs\ _C1\ traps - \rho\ Vs\ c0 - \rho\ Vs\ c0\ traps\right)}{\left(-1 + 2\ v\right)\ MWs}$$

Note #4: Now we will introduce numerical values for the material properties.

The values selected are typical for internal hydrogen as a solute in AISI 4340 steel.

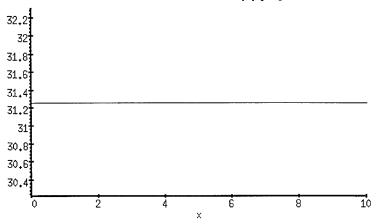
> S0:=0.0;		
	<i>SO</i> := 0	
	<i>SU</i> := 0	
> Ds:=1e-9;		
	$Ds := .1 \ 10^{-8}$	
> Vs:=2.02e-6;		
→ V32.02e-0,		
	$Vs := .202 \ 10^{-5}$	
> traps:=19;		
	<i>traps</i> := 19	
	iraps .= 19	
> c0:=0.0;		
	<i>c0</i> := 0	
> MWs:=0.00100797;		
/ III 11 3. <u>– 0.00 100 707 7</u>		
	MWs := .00100797	
> Em:=200e9;		
	$Em := .200 \ 10^{12}$	
	Ent200 10	
> nu:=0.3;		
	v := .3	
> rho:=7800;		
×1110.=7000,	= 000	
	$\rho := 7800$	
> T:=293;		
	T := 293	
	1 .= 273	
> R:=8.31696;		
	R := 8.31696	
> K;		
> n,		
	.166666666666667 10 ¹²	
> lambda;		
	.115384615384616 10 ¹²	
	.115564015564010 10	
> mu;		
	.769230769230769 1011	
> alpha;		
> aipiia,		
	5.21047253390476	
> Rh:=R/MWs;		
	<i>Rh</i> := 8251,19795232001	
	Rit .= 0231,17773232001	
> K1;		
	138.155406787625	
> K2;		
· - 7	212 (2025222 122)	
	312.628352034286	
> K1*K2;		

```
43191.2971486416
> 1/(K1*K2);
                                  .0000231528123954816
> A1;
                                             .4
> A2;
                                    270.944571763048
> A3;
                                             0
> B1;
                                   .50000000000001 \ 10^{12}
> B2;
                                  .156314176017143\ 10^{15}
> B3;
                                             0
  Note #6: Now let's solve for the integration constants: _C1, _C2, and _C3, using the
           boundary data.
> c2(x);
                                            _C1
> _C1:=1e-7;
                                       _{C1} := .1 \ 10^{-6}
> c2(x);
                                           .1\ 10^{-6}
> u2(x);
       > S2(x);
                     .500000000000001 10^{12} _C2 - .156314176017143 10^{8}
> _C2:=solve(S2(0.1)=0.0,_C2);
                               \_C2 := .0000312628352034285
> S2(x);
                                             0
> u2(x);
                      .0000104209450678093 \ x + 1.000000000000000 \ \_C3
> _C3:=solve(u2(0.0)=0,_C3);
                                          _{C3} := 0
 > u2(x);
                                  .0000104209450678093 x
```

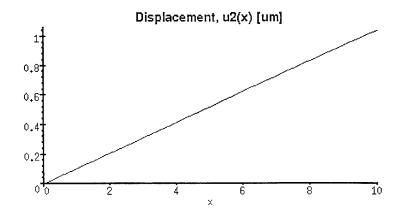
.0000312628352034285

> p1:=plot(1e6*e2(x/100),x=0..10,title='Dilatational Strain, e2(x) [ue]'): > p1; >

Dilatational Strain, e2(x) [ue]



> p2:=plot(1e6*u2(x/100),x=0..10,title='Displacement, u2(x) [um]'): > p2; >



> ex:=unapply(diff(u2(x),x),x);

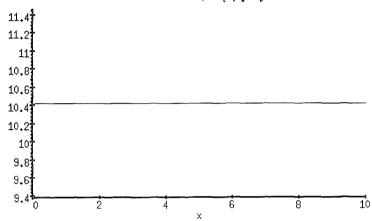
ex := .0000104209450678093

> ex(x);

.0000104209450678093

> p3:=plot(1e6*ex(x/100),x=0..10,title='Axial Strain, ex(x) [ue]'): > p3;

Axial Strain, ex(x) [ue]



> evalf(10^6*u2(x),5);		
	10.420 <i>x</i>	
> evalf(c2(x),5);		
	.1 10-6	
> evalf(10^6*e2(x),5);		
	31.263	
> evalf(10^6*ex(x),5);		
	10.421	
	101121	

1. MAPLE PROGRAM DIRECTORY AND FILE NAME: /afs/nd.edu/user4/jthomas5/Maple/Transport/1DSSsol#1e.ms

2. ORIGINATOR:
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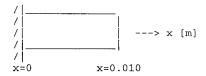
3. HISTORY:

Written: June-1994

Latest Revision: 13-Feb-1995

4. PROGRAM DESCRIPTION:

One-dimensional, steady-state analysis of the fully coupled solute transport and trapping equations using "plane stress" constitutive equations for stress. The particular problem solved is shown below:



Deformation variable boundary conditions: u(0)=0, S(0.10)=0Concentration variable boundary conditions: J(0)=0, c(0.10)=1e-7Reference concentration level: c0=0.0Trapping parameter: traps=499 (high trapping)

The equations used in the analysis are:

- dJ/dx=0 (steady-state diffusion equation)
- 2. J=Ds*(K1*c*de/dx-dc/dx) (mass flux)
- 3. dS/dx=0 (steady-state deformation equation with zero body forces)
- 4. S=B1*e-B2*c+B3 (axial stress)
- 5. de/dx-K2*dc/dx=0 (dilational strain gradient)
- 6. e=A1*du/dx+A2*c-A3 (dilatational strain)

The constants used in the plane stress analysis are defined below:

Ds:=lattice diffusivity [m^2/sec]

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K1 := (Vs*K) / (R*T)
Vs:= partial molar volume of solute [m^3/mol solute]
K:= bulk modulus=Em/(3*(1-2*nu)) [N/m^2]
Em: = Modulus of Elasticity [N/m^2]
nu:= Poisson's ratio [1]
R:= universal gas constant=8.31432 [J/mol-K]
T:= temperature [K]
B1:=3*K
B2:=9*K*alpha*(1+traps)
B3:=B2*co
traps:= Csr*Kr=trapping constant [1]
Csr:= saturation trap concentration [1]
Kr:= equilibrium trapping constant [1]
c0:= reference solute concentration [1]
K2:=3*alpha*(1+traps)
alpha:= (rho*Vs)/(3*MWs)=solute concentration expansion coefficient [m/m/Delc]
rho:= mass density of the solid [kg/m^3]
MWs:= molecular weight of the solute [kg/mol]
Delc:= c-c0 [1]
```

A1:=1-2*nu

A2:=2*(1+nu)*alpha*(1+traps)

A3:=A2*c0

5. REQUIRED INPUTS:

Ds:=lattice diffusivity [m^2/sec]

Vs:= partial molar volume of solute [m^3/mol solute]

traps:= Csr*Kr=trapping constant [1]

c0:= reference solute concentration [1]

MWs:= molecular weight of the solute [kg/mol]

Em:= Modulus of Elasticity [Pa]

nu:= Poisson's ratio [1]

rho:= mass density of the solid [kg/m^3]

T:= temperature [K]

6. SYMBOLIC ANALYSIS:

> Digits:≈trunc(evalhf(Digits));

$$Digits := 15$$

> J:=Ds*(K1*K2*c(x)-1)*diff(c(x),x);

$$J := Ds (K1 \ K2 \ c(x) - 1) \left(\frac{\partial}{\partial x} c(x) \right)$$

> degn:=J=0;

$$deqn := Ds (K1 \ K2 \ c(x) - 1) \left(\frac{\partial}{\partial x} c(x) \right) = 0$$

> csoln:=dsolve(deqn,c(x),explicit);

$$csoln := c(x) = \frac{1}{K1 \ K2}, c(x) = _C1$$

Note #1: There are apparently two "roots" to the solution for c(x). The first solution is obviously incorrect in that the concentration throughout the rod is defined in terms of the constants K1 and K2. The second solution is adopted as correct.

> c2:=unapply(simplify(op(2,csoln[2])),x);

$$c2 := x \rightarrow _C1$$

> c2(x);

 $_CI$

> J2:=unapply(simplify(Ds*(K1*K2*c2(x)-1)*diff(c2(x),x)));

J2 := 0

> J2(x);

0

Note #2: The above results show that the mass flux is identically zero, as it should be.

> e2:=x->_C2;

$$e2 := x \rightarrow _C2$$

> e2(x);

_*C*2

> ux2:=dsolve(A1*diff(u(x),x)+A2*c2(x)-A3-e2(x)=0,u(x));

$$ux2 := u(x) = -\frac{x A2 _C1 - x A3 - x _C2 - _C3 A1}{A1}$$

> u2:=unapply(simplify(op(2,ux2)),x);

$$u2 := x \rightarrow -\frac{x A2 _C1 - x A3 - x _C2 - _C3 A1}{A1}$$

> u2(x);

> S2:=x->simplify(B1*e2(x)-B2*c2(x)+B3);

$$S2 := x \rightarrow \text{simplify}(B1 \text{ e}2(x) - B2 \text{ c}2(x) + B3)$$

> S2(x);

$$B1 \ C2 - B2 \ C1 + B3$$

Note #3: At this point, we will introduce expressions for the constants and then simplify the results before quantifying the constants and determining the constants of integration.

> K:=(Em/(3*(1-2*nu)));

$$K := \frac{Em}{3 - 6 v}$$

> lambda:=(Em*nu)/((1+nu)*(1-2*nu));

$$\lambda := \frac{Em \, V}{(1+V)(1-2 \, V)}$$

> mu:=Em/(2*(1+nu));

$$\mu := \frac{Em}{2+2v}$$

> alpha:=(rho*Vs)/(3*MWs);

$$\alpha := \frac{1}{3} \frac{\rho \ Vs}{MWs}$$

> K1:=(Vs*K)/(R*T);

$$K1 := \frac{Vs Em}{(3 - 6 \text{ V}) R T}$$

> K2:=3*alpha*(1+traps);

 $K2 := \frac{\rho \ Vs \ (1 + traps)}{MWs}$

> K1*K2;

 $\frac{Vs^2 Em \rho (1 + traps)}{(3 - 6 v) R T MWs}$

> A1:=(1-2*nu);

A1 := 1 - 2 v

> A2:=2*(1+nu)*alpha*(1+traps);

 $A2 := \frac{2}{3} \frac{(1+v) \rho Vs (1+traps)}{MWs}$

> A3:=A2*c0;

 $A3 := \frac{2}{3} \frac{(1+v) \rho \ Vs (1 + traps) cO}{MWs}$

> B1:=3*K;

 $B1 := 3 \frac{Em}{3 - 6 v}$

> B2:=9*K*alpha*(1+traps);

 $B2 := 3 \frac{Em \rho Vs (1 + traps)}{(3 - 6 v) MWs}$

> B3:=B2*c0;

 $B3 := 3 \frac{Em \rho Vs (1 + traps) cO}{(3 - 6 v) MWs}$

> c2(x);

 $_C1$

> e2(x);

_C2

> collect(u2(x),x);

$$-\frac{\left(\frac{2}{3}\frac{(1+v)\rho Vs (1+traps)_{C1}}{MWs} - \frac{2}{3}\frac{(1+v)\rho Vs (1+traps)c0}{MWs} - _{C2}\right)x}{1-2v} + _{C3}$$

> S2(x);

 $\frac{Em\left(-_C2\ MWs + \rho\ Vs\ _C1 + \rho\ Vs\ _C1\ traps - \rho\ Vs\ c0 - \rho\ Vs\ c0\ traps\right)}{\left(-1 + 2\ \nu\right)\ MWs}$

Note #4: Now we will introduce numerical values for the material properties.

The values selected are typical for internal hydrogen as a solute in AISI 4340 steel.

Solute In Alsi 4340 stee	± •	
> Ds:=1e-9;		
> D310-3,		
	$Ds := .1 \ 10^{-8}$	
> Vs:=2.02e-6;		
	$Vs := .202 \ 10^{-5}$	
	VS .— .202 TO	
> traps:=499;		
	<i>traps</i> := 499	
> c0:=0.0;	1	
> co:=0.0;		
	c0 := 0	
> MWs:=0.00100797;		
•	MW - 00100707	
	MWs := .00100797	
> Em:=200e9;		
	$Em := .200 \ 10^{12}$	
0.0.	2	
> nu:=0.3;		
	v := .3	
> rho:=7800;		
	7000	
	$\rho := 7800$	
> T:=293;		
	T := 293	
	1 255	
> R:=8.31432;		
	R := 8.31432	
> r.,		
	$.1666666666666667 \ 10^{12}$	
> lambda;		
	.115384615384616 10 ¹²	
	.113384013384010 10	
> mu;		
	.769230769230769 1011	
> alpha;		
> aipiia,		
	5.21047253390476	
> Rh:=R/MWs;		
	<i>Rh</i> := 8248.57882675080	
	Mt 8248.37882073080	
> K1;		
	138.199274509089	
> K2;		
~ ILE,		
	7815.70880085715	
> K1*K2;		
	.108012528605276 10 ⁷	
	.100012320003270 10	

```
> 1/(K1*K2);
                                       .925818525788271 10-6
> A1;
                                                  .4
> A2;
                                         6773.61429407620
> A3;
                                                  0
> B1;
                                       .500000000000001 \ 10^{12}
> B2;
                                      .390785440042860\ 10^{16}
> B3;
                                                  0
  Note #6: Now let's solve for the integration constants: _C1, _C2, and _C3, using the
> c2(x);
                                                 \_C1
> _C1:=1e-7;
                                            C1 := .1 \ 10^{-6}
> c2(x);
                                               .1 10-6
> u2(x);
         -.00169340357351905 \ x + 2.500000000000000 \ x \ \_C2 + 1.0000000000000 \ \_C3
> S2(x);
                       .50000000000000110^{12} C2 - .39078544004286010^{9}
> _C2:=solve(S2(0.1)=0.0,_C2);
                                   \_C2 := .000781570880085718
> S2(x);
                                                  0
> u2(x);
                        .000260523626695245 x + 1.00000000000000 _C3
> _C3:=solve(u2(0.0)=0,_C3);
                                              _{C}3 := 0
> u2(x);
                                      .000260523626695245 x
> e2(x);
                                       .000781570880085718
```

1. MAPLE PROGRAM DIRECTORY AND FILE NAME:
/afs/nd.edu/user4/jthomas5/Maple/Transport/1DSSsol#2a.ms

2. ORIGINATOR:

Dr. James P. Thomas University of Notre Dame Department of Aerospace and Mechanical Engineering 374 Fitzpatrick Hall Notre Dame, IN 46556-5637 (219) 631-9371

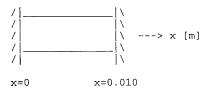
3. HISTORY:

Written: June-1994

Latest Revision: 13-Feb-1995

4. PROGRAM DESCRIPTION:

One-dimensional, steady-state analysis of the fully coupled solute transport and trapping equations using "plane stress" constitutive equations for stress. The particular problem solved is shown below:



Deformation variable boundary conditions: u(0)=0, u(0.10)=0Concentration variable boundary conditions: c(0)=0, c(0.10)=1e-6Reference concentration level: c0=0.0Trapping parameter: traps=19 (low trapping)

The equations used in the analysis are:

- dJ/dx=0 (steady-state diffusion equation)
- 2. J=Ds*(K1*c*de/dx-dc/dx) (mass flux)
- dS/dx=0 (steady-state deformation equation with zero body forces)
- 4. S=B1*e-B2*c+B3 (axial stress)
- 5. de/dx-K2*dc/dx=0 (dilational strain gradient)
- 6. e=A1*du/dx+A2*c-A3 (dilatational strain)

The constants used in the plane stress analysis are defined below:

Ds:=lattice diffusivity [m^2/sec]

K1 := (Vs*K) / (R*T)

```
Vs:= partial molar volume of solute [m^3/mol solute]
K:= bulk modulus=Em/(3*(1-2*nu)) [N/m<sup>2</sup>]
Em:= Modulus of Elasticity [N/m^2]
nu:= Poisson's ratio [1]
R:= universal gas constant=8.31432 [J/mol-K]
T:= temperature [K]
B1 := 3 * K
B2:=9*K*alpha*(1+traps)
B3:≃B2*co
traps:= Csr*Kr=trapping constant [1]
Csr:= saturation trap concentration [1]
Kr:= equilibrium trapping constant [1]
c0:= reference solute concentration [1]
K2:=3*alpha*(1+traps)
alpha:= (rho*Vs)/(3*MWs)=solute concentration expansion coefficient [m/m/Delc]
rho:= mass density of the solid [kg/m^3]
MWs:= molecular weight of the solute [kg/mol]
Delc:= c-c0 [1]
```

A1:=1-2*nu

A2:=2*(1+nu)*alpha*(1+traps)

A3:=A2*c0

5. REQUIRED INPUTS:

Ds:=lattice diffusivity [m^2/sec]

Vs:= partial molar volume of solute [m^3/mol solute]

traps:= Csr*Kr=trapping constant [1]

c0:= reference solute concentration [1]

MWs:= molecular weight of the solute [kg/mol]

Em: = Modulus of Elasticity [Pa]

nu:= Poisson's ratio [1]

rho:= mass density of the solid $[kg/m^3]$

T:= temperature [K]

6. SYMBOLIC ANALYSIS:

> Digits:=trunc(evalhf(Digits));

$$Digits := 15$$

> J:=Ds*(K1*K2*c(x)-1)*diff(c(x),x);

$$J := Ds (Kl \ K2 \ c(x) - 1) \left(\frac{\partial}{\partial x} c(x) \right)$$

> deqn:=diff(J,x)=0;

$$deqn := Ds K1 K2 \left(\frac{\partial}{\partial x} c(x)\right)^{2} + Ds (K1 K2 c(x) - 1) \left(\frac{\partial^{2}}{\partial x^{2}} c(x)\right) = 0$$

> csoln:=dsolve(deqn,c(x),explicit);

$$csoln := c(x) = -\frac{CI\left(-\frac{1}{CI} - \frac{\sqrt{1 + 2 KI K2 _CI x + 2 KI K2 _CI _C2}}{CI}\right)}{KI K2}$$

$$c(x) = -\frac{CI\left(-\frac{1}{CI} + \frac{\sqrt{1 + 2 KI K2 _CI x + 2 KI K2 _CI _C2}}{CI}\right)}{KI K2}$$

Note #1: There are apparently two "roots" to the solution for c(x). The boundary conditions will be used to select the proper one.

> c1:=unapply(simplify(op(2,csoln[1])),x);

$$c1 := x \to \frac{1 + \sqrt{1 + 2 KI K2 _CI x + 2 KI K2 _CI _C2}}{KI K2}$$

> c1(x);

$$\frac{1 + \sqrt{1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2}}{K1 K2}$$

> c2:=unapply(simplify(op(2,csoln[2])),x);

$$c2 := x \to -\frac{-1 + \sqrt{1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2}}{K1 K2}$$

> c2(x);

$$\frac{-1 + \sqrt{1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2}}{K1 K2}$$

> J1:=unapply(simplify(Ds*(K1*K2*c1(x)-1)*diff(c1(x),x),x));

$$J1 := () \rightarrow Ds _C1$$

> J1(x);

> J2:=unapply(simplify(Ds*(K1*K2*c2(x)-1)*diff(c2(x),x)));

$$J2 := () \rightarrow Ds \quad C1$$

> J2(x);

$$Ds _C1$$

Note #2: The above results show that both mass flux solutions are identical and equal to Ds*_C1 where _C1 is a constant of integration!

> ex1:=dsolve(diff(e(x),x)-K2*diff(c1(x),x)=0,e(x));

$$ex1 := e(x) = \frac{\sqrt{1 + 2 K1 K2 C1 x + 2 K1 K2 C1 C2} + C3 K1}{K1}$$

> e1:=unapply(simplify(op(2,ex1)),x);

$$e1 := x \to \frac{\sqrt{1 + 2 K1 K2 C1 x + 2 K1 K2 C1 C2} + C3 K1}{K1}$$

> e1(x);

$$\frac{\int_{1+2 K1 K2}_{C1 x+2 K1 K2}_{C1 C2} + C3 K1}{K1}$$

> ex2:=dsolve(diff(e(x),x)-K2*diff(c2(x),x)=0,e(x));

$$ex2 := e(x) = \frac{-\sqrt{1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2} + _C3 K1}{K1}$$

> e2:=unapply(simplify(op(2,ex2)),x);

$$e2 := x \to \frac{-\sqrt{1 + 2 \ K1 \ K2 \ _C1 \ x + 2 \ K1 \ K2 \ _C1 \ _C2} + _C3 \ K1}{K1}$$

> e2(x);

$$\frac{-\sqrt{1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2} + _C3 K1}{K1}$$

> ux1:=dsolve(A1*diff(u(x),x)+A2*c1(x)-A3-e1(x)=0,u(x));

$$uxl := u(x) = \frac{1}{3} \left(-3 \text{ A2 } x \text{ K1 } \text{ K2 } _\text{C1 - A2} \sqrt{\%1} - 2 \sqrt{\%1} \text{ A2 } \text{K1 } \text{K2 } _\text{C1 } x \right)$$

$$-2 \sqrt{\%1} \text{ A2 } \text{K1 } \text{K2 } _\text{C1 } _\text{C2} + 3 \text{ A3 } x \text{ K1}^2 \text{ K2}^2 _\text{C1} + \sqrt{\%1} \text{ K2} + 2 \sqrt{\%1} \text{ K2}^2 \text{ K1 } _\text{C1 } x$$

$$+2 \sqrt{\%1} \text{ K2}^2 \text{ K1 } _\text{C1 } _\text{C2} + 3 \text{ _C3 } x \text{ K1}^2 \text{ K2}^2 \text{ _C1} + 3 \text{ _C4 } \text{A1 } \text{K1}^2 \text{ K2}^2 \text{ _C1} \right) / (\text{A1 } \text{K1}^2 \text{ K2}^2 \text{ _C1})$$

 $%1 := 1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2$

> u1:=unapply(simplify(op(2,ux1)),x);

$$ul := x \rightarrow \frac{1}{3} \left(-3 \text{ A2 } x \text{ K1 K2 } _C1 - \text{A2} \sqrt{1 + 2 \text{ K1 K2 }} _C1 \text{ } x + 2 \text{ K1 K2 }} _C1 _C2 \right)$$

$$-2 \sqrt{1 + 2 \text{ K1 K2 }} _C1 \text{ } x + 2 \text{ K1 K2 }} _C1 _C2 \text{ A2 K1 K2 }} _C1 \text{ } x$$

$$-2 \sqrt{1 + 2 \text{ K1 K2 }} _C1 \text{ } x + 2 \text{ K1 K2 }} _C1 _C2 \text{ A2 K1 K2 }} _C1 _C2 + 3 \text{ A3 } x \text{ K1}^2 \text{ K2}^2 _C1$$

$$+ \sqrt{1 + 2 \text{ K1 K2 }} _C1 \text{ } x + 2 \text{ K1 K2 }} _C1 _C2 \text{ K2}$$

$$+2 \sqrt{1 + 2 \text{ K1 K2 }} _C1 \text{ } x + 2 \text{ K1 K2 }} _C1 _C2 \text{ K2}^2 \text{ K1 }} _C1 \text{ } x$$

$$+2 \sqrt{1 + 2 \text{ K1 K2 }} _C1 \text{ } x + 2 \text{ K1 K2 }} _C1 _C2 \text{ K2}^2 \text{ K1 }} _C1 _C2 + 3 _C3 \text{ } x \text{ K1}^2 \text{ K2}^2 _C1$$

$$+3 _C4 \text{ A1 K1}^2 \text{ K2}^2 _C1 \right) / (\text{A1 K1}^2 \text{ K2}^2 _C1)$$

> u1(x)

$$\frac{1}{3} \left(-3 A2 \times K1 K2 _C1 - A2 \sqrt{\%1} - 2 \sqrt{\%1} A2 K1 K2 _C1 \times - 2 \sqrt{\%1} A2 K1 K2 _C1 _C2 \right)$$

$$+ 3 A3 \times K1^{2} K2^{2} _C1 + \sqrt{\%1} K2 + 2 \sqrt{\%1} K2^{2} K1 _C1 \times + 2 \sqrt{\%1} K2^{2} K1 _C1 _C2$$

$$+ 3 _C3 \times K1^{2} K2^{2} _C1 + 3 _C4 A1 K1^{2} K2^{2} _C1 \right) / (A1 K1^{2} K2^{2} _C1)$$

 $%1 := 1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2$

> ux2:=dsolve(A1*diff(u(x),x)+A2*c2(x)-A3-e2(x)=0,u(x));

$$ux2 := u(x) = \frac{1}{3} \left(-3 A2 x K1 K2 _C1 + A2 \sqrt{\%1} + 2 \sqrt{\%1} A2 K1 K2 _C1 x \right)$$

$$+ 2 \sqrt{\%1} A2 K1 K2 _C1 _C2 + 3 A3 x K1^2 K2^2 _C1 - \sqrt{\%1} K2 - 2 \sqrt{\%1} K2^2 K1 _C1 x$$

$$- 2 \sqrt{\%1} K2^2 K1 _C1 _C2 + 3 _C3 x K1^2 K2^2 _C1 + 3 _C4 A1 K1^2 K2^2 _C1 \right) / (A1 K1^2 K2^2 _C1)$$

$$%1 := 1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2$$

> u2:=unapply(simplify(op(2,ux2)),x);

$$u2 := x \rightarrow \frac{1}{3} \left(-3 \text{ A2 } x \text{ K1 K2 _C1} + \text{A2} \sqrt{1 + 2 \text{ K1 K2 _C1} x + 2 \text{ K1 K2 _C1} _C2} \right.$$

$$+ 2 \sqrt{1 + 2 \text{ K1 K2 _C1} x + 2 \text{ K1 K2 _C1} _C2} \text{ A2 K1 K2 _C1} x$$

$$+ 2 \sqrt{1 + 2 \text{ K1 K2 _C1} x + 2 \text{ K1 K2} _C1} _C2 \text{ A2 K1 K2 _C1} _C2 + 3 \text{ A3 } x \text{ K1}^2 \text{ K2}^2 _C1}$$

$$- \sqrt{1 + 2 \text{ K1 K2} _C1 x + 2 \text{ K1 K2} _C1} _C2 \text{ K2}$$

$$- 2 \sqrt{1 + 2 \text{ K1 K2} _C1 x + 2 \text{ K1 K2} _C1} _C2 \text{ K2}^2 \text{ K1} _C1 x$$

$$- 2 \sqrt{1 + 2 \text{ K1 K2} _C1 x + 2 \text{ K1 K2} _C1} _C2 \text{ K2}^2 \text{ K1} _C1 _C2 + 3 _C3 x \text{ K1}^2 \text{ K2}^2 _C1}$$

$$+ 3 _C4 \text{ A1 K1}^2 \text{ K2}^2 _C1 / (\text{A1 K1}^2 \text{ K2}^2 _C1)$$

> u2(x);

$$\frac{1}{3} \left(-3 A2 x K1 K2 _C1 + A2 \sqrt{\%1} + 2 \sqrt{\%1} A2 K1 K2 _C1 x + 2 \sqrt{\%1} A2 K1 K2 _C1 _C2 + 3 A3 x K1^2 K2^2 _C1 - \sqrt{\%1} K2 - 2 \sqrt{\%1} K2^2 K1 _C1 x - 2 \sqrt{\%1} K2^2 K1 _C1 _C2 + 3 _C3 x K1^2 K2^2 _C1 + 3 _C4 A1 K1^2 K2^2 _C1 \right) / (A1 K1^2 K2^2 _C1)$$

$$%1 := 1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2$$

> S1:=x->simplify(B1*e1(x)-B2*c1(x)+B3);

$$SI := x \rightarrow \text{simplify}(BI \text{ el}(x) - B2 \text{ cl}(x) + B3)$$

> S1(x);

> S2:=x->simplify(B1*e2(x)-B2*c2(x)+B3);

$$S2 := x \rightarrow \text{simplify}(B1 \text{ e2}(x) - B2 \text{ c2}(x) + B3)$$

> S2(x);

$$(-B1 \ K2 \sqrt{1 + 2 \ K1 \ K2 \ _C1 \ x + 2 \ K1 \ K2 \ _C1 \ _C2} + B1 \ K2 \ _C3 \ K1 - B2 + B2 \sqrt{1 + 2 \ K1 \ K2 \ _C1 \ x + 2 \ K1 \ K2 \ _C1 \ _C2} + B3 \ K1 \ K2)/(K1 \ K2)$$

Note #3: At this point, we will introduce expressions for the constants and then simplify the results before quantifying the constants and determining the constants of integration.

> K:=(Em/(3*(1-2*nu)));

$$K := \frac{Em}{3 - 6 v}$$

> lambda:=(Em*nu)/((1+nu)*(1-2*nu));

$$\lambda := \frac{Em \, v}{(1+v)(1-2 \, v)}$$

> mu:=Em/(2*(1+nu));

$$\mu := \frac{Em}{2 + 2 \nu}$$

> alpha:=(rho*Vs)/(3*MWs);

$$\alpha := \frac{1}{3} \frac{\rho \ Vs}{MWs}$$

> K1:=(Vs*K)/(R*T);

$$K1 := \frac{Vs Em}{(3 - 6 v) R T}$$

> K2:=3*alpha*(1+traps);

$$K2 := \frac{\rho \ Vs (1 + traps)}{MWs}$$

> K1*K2;

$$\frac{Vs^2 Em \rho (1 + traps)}{(3 - 6 v) R T MWs}$$

> A1:=(1-2*nu);

$$AI := 1 - 2 v$$

> A2:=2*(1+nu)*alpha*(1+traps);

$$A2 := \frac{2}{3} \frac{(1+v) \rho Vs (1+traps)}{MWs}$$

> A3:=A2*c0;

$$A3 := \frac{2}{3} \frac{(1+v) \rho \ Vs (1+traps) \ c0}{MWs}$$

> B1:=3*K;

$$B1 := 3 \frac{Em}{3 - 6 v}$$

> B2:=9*K*alpha*(1+traps);

$$B2 := 3 \frac{Em \rho Vs (1 + traps)}{(3 - 6 v) MWs}$$

> B3:=B2*c0;

$$B3 := 3 \frac{Em \rho Vs (1 + traps) cO}{(3 - 6 v) MWs}$$

>et(w);
$$\left(1 + \sqrt{1 + 2 \frac{V_s^2 Em \rho (1 + traps) - CI x}{(3 - 6 \vee) R T MWs}} + 2 \frac{V_s^2 Em \rho (1 + traps) - CI - C2}{(3 - 6 \vee) R T MWs} \right) (3 - 6 \vee) R T$$

$$MWs/(Vs^2 Em \rho (1 + traps))$$
>e2(x);
$$-\left(-1 + \sqrt{1 + 2 \frac{Vs^2 Em \rho (1 + traps) - CI x}{(3 - 6 \vee) R T MWs}} + 2 \frac{Vs^2 Em \rho (1 + traps) - CI - C2}{(3 - 6 \vee) R T MWs} \right) (3 - 6 \vee) R T$$

$$MWs/(Vs^2 Em \rho (1 + traps))$$
>e1(x);
$$\left(\sqrt{1 + 2 \frac{Vs^2 Em \rho (1 + traps) - CI x}{(3 - 6 \vee) R T MWs}} + 2 \frac{Vs^2 Em \rho (1 + traps) - CI - C2}{(3 - 6 \vee) R T MWs} + \frac{-C3 Vs Em}{(3 - 6 \vee) R T} \right)$$

$$> e2(x);$$

$$\left(-\sqrt{1 + 2 \frac{Vs^2 Em \rho (1 + traps) - CI x}{(3 - 6 \vee) R T MWs}} + 2 \frac{Vs^2 Em \rho (1 + traps) - CI - C2}{(3 - 6 \vee) R T MWs} + \frac{-C3 Vs Em}{(3 - 6 \vee) R T} \right)$$

$$> e1(x);$$

$$\left(-\sqrt{1 + 2 \frac{Vs^2 Em \rho (1 + traps) - CI x}{(3 - 6 \vee) R T MWs}} + 2 \frac{Vs^2 Em \rho (1 + traps) - CI - C2}{(3 - 6 \vee) R T MWs} + \frac{-C3 Vs Em}{(3 - 6 \vee) R T} \right)$$

$$> o1(x);$$

$$\frac{1}{3} \left(-2 \frac{(1 + \nu) \rho^2 Vs^3 (1 + traps)^2 x Em - CI}{MWs^2 (3 - 6 \vee) R T} - \frac{2}{3} \frac{(1 + \nu) \rho Vs (1 + traps) \sqrt{\%1}}{MWs} \right)$$

$$- \frac{4 \sqrt{\%1} (1 + \nu) \rho^2 Vs^3 (1 + traps)^2 Em - CI x}{MWs^2 (3 - 6 \vee) R T}$$

$$+ 2 \frac{4 \sqrt{\%1} (1 + \nu) \rho^2 Vs^3 (1 + traps)^2 Em - CI - C2}{MWs^2 (3 - 6 \vee) R T} + 2 \frac{\sqrt{\%1} \rho Vs (1 + traps)}{MWs} = \frac{-C1 - C2}{MWs^2 (3 - 6 \vee) R T}$$

$$+ 2 \frac{\sqrt{\%1} \rho^2 Vs^3 (1 + traps)^2 Em - CI x}{MWs^2 (3 - 6 \vee) R T} + 2 \frac{\sqrt{\%1} \rho^2 Vs^3 (1 + traps)^2 Em - CI - C2}{MWs^2 (3 - 6 \vee) R T} + 2 \frac{\sqrt{\%1} \rho^2 Vs^3 (1 + traps)^2 Em - CI - C2}{MWs^2 (3 - 6 \vee) R T} + 2 \frac{-C3 Vs Em}{MWs^2 (3 - 6 \vee) R T} + 2 \frac{-C3 Vs Em}{MWs^2 (3 - 6 \vee) R T} + 2 \frac{-C3 Vs Em}{MWs^2 (3 - 6 \vee) R T} + 2 \frac{-C3 Vs Em}{MWs^2 (3 - 6 \vee) R T} + 2 \frac{-C3 Vs Em}{MWs^2 (3 - 6 \vee) R T} + 2 \frac{-C3 Vs Em}{MWs^2 (3 - 6 \vee) R T} + 2 \frac{-C3 Vs Em}{MWs^2 (3 - 6 \vee) R T} + 2 \frac{-C3 Vs Em}{MWs^2 (3 - 6 \vee) R T} + 2 \frac{-C3 Vs Em}{MWs^2 (3 - 6 \vee) R T} + 2 \frac{-C3 Vs Em}{MWs^2 (3 - 6 \vee) R T} + 2 \frac{-C3 Vs Em}{MWs^2 (3 - 6 \vee) R T} + 2 \frac{-C3 Vs Em}{MWs^2 (3 - 6 \vee) R T} + 2 \frac{-C3 Vs Em}{MWs^2 (3 - 6 \vee) R T} + 2 \frac{-C3 Vs Em}{MWs^2 (3 - 6 \vee) R T} + 2 \frac{-C3 Vs Em}{MWs^2 (3 - 6 \vee) R T} + 2 \frac$$

$$\%1 := 1 + 2 \frac{Vs^2 Em \rho (1 + traps) _C1 x}{(3 - 6 v) R T MWs} + 2 \frac{Vs^2 Em \rho (1 + traps) _C1 _C2}{(3 - 6 v) R T MWs}$$

> u2(x);
$$\frac{1}{3} \left(-2 \frac{(1+v) \rho^2 V s^3 (1 + traps)^2 x Em_C1}{MW s^2 (3 - 6 v) R T} + \frac{2}{3} \frac{(1+v) \rho V s (1 + traps) \sqrt{\%1}}{MW s} \right)$$

$$+ \frac{4}{3} \frac{\sqrt{\%1} (1+v) \rho^2 V s^3 (1 + traps)^2 Em_C1 x}{MW s^2 (3 - 6 v) R T}$$

$$+ \frac{4}{3} \frac{\sqrt{\%1} (1+v) \rho^2 V s^3 (1 + traps)^2 Em_C1 _C2}{MW s^2 (3 - 6 v) R T}$$

$$+ 2 \frac{(1+v) \rho^3 V s^5 (1 + traps)^3 cO x Em^2_C1}{MW s^3 (3 - 6 v)^2 R^2 T^2} - \frac{\sqrt{\%1} \rho V s (1 + traps)}{MW s}$$

$$- 2 \frac{\sqrt{\%1} \rho^2 V s^3 (1 + traps)^2 Em_C1 x}{MW s^2 (3 - 6 v) R T} - 2 \frac{\sqrt{\%1} \rho^2 V s^3 (1 + traps)^2 Em_C1 _C2}{MW s^2 (3 - 6 v) R T}$$

$$+ 3 \frac{-C3 x V s^4 Em^2 \rho^2 (1 + traps)^2 _C1}{(3 - 6 v)^2 R^2 T^2 MW s^2} + 3 \frac{-C4 (1 - 2 v) V s^4 Em^2 \rho^2 (1 + traps)^2 _C1}{(3 - 6 v)^2 R^2 T^2 MW s^2}$$

$$- \frac{(3 - 6 v)^2 R^2 T^2 MW s^2 / ((1 - 2 v) V s^4 Em^2 \rho^2 (1 + traps)^2 _C1}{(3 - 6 v) R T MW s} + 2 \frac{V s^2 Em \rho (1 + traps) _C1 _C2}{(3 - 6 v) R T MW s}$$

> S1(x);

$$-(MWs _C3 \ Vs \ Em - 3 \ R \ T \ MWs + 6 \ R \ T \ MWs \ v + Em \ \rho \ Vs^2 \ c0 + Em \ \rho \ Vs^2 \ c0 \ traps)/($$

$$(-1 + 2 \ v) \ Vs \ MWs)$$

> S2(x);

-
$$(MWs _C3 \ Vs \ Em - 3 \ R \ T \ MWs + 6 \ R \ T \ MWs \ v + Em \ \rho \ Vs^2 \ c0 + Em \ \rho \ Vs^2 \ c0 \ traps)/((-1 + 2 \ v) \ Vs \ MWs)$$

> S1(x)-S2(x);

0

Note #4: The stresses are same for each solution root, and do not depend on the position x! The stresses are therefore constant throughout the body. We define a new stress function, Sx.

> Sx:=S2(x);

$$Sx := -\left(MWs _C3 \ Vs \ Em - 3 \ R \ T \ MWs + 6 \ R \ T \ MWs \ v + Em \ \rho \ Vs^2 \ cO + Em \ \rho \ Vs^2 \ cO \ traps\right) / ($$

$$(-1 + 2 \ v) \ Vs \ MWs)$$

Note #5: Now we will introduce numerical values for the material properties. The values selected are typical for internal hydrogen as a solute in AISI 4340 steel.

> Ds:=1e-9;	
	$Ds := .1 \ 10^{-8}$
. V- 0.00 0	23110
> Vs:=2.02e-6;	
	$Vs := .202 \ 10^{-5}$
> traps:=19;	
•	tuana 10
	traps := 19
> c0:=0.0;	
	$c\theta := 0$
> MWs:=0.00100797;	
> WI 8 9 3 0.00 100 / 31,	
	MWs := .00100797
> Em:=200e9;	
	$Em := .200 \ 10^{12}$
	EIII .— .200 10
> nu:=0.3;	
	v := .3
> rho:=7800;	
	7000
	$\rho := 7800$
> T:=293;	
	T := 293
> R:=8.31432;	
7 III-0.0 I-102;	D 0.21/02
	R := 8.31432
> K;	
	.16666666666667 1012
January de	.10000000000110
> lambda;	
	.115384615384616 10 ¹²
> mu;	
	.769230769230769 1011
	.707430707430707 10
> alpha;	
	5.21047253390476
> Rh:=R/MWs;	
	DI . 0040 57000/75000
	Rh := 8248.57882675080
> K1;	
	138.199274509089
> K2;	
/ NZ,	
	312.628352034286
> K1*K2;	
	43205.0114421104
	13203,0117721107

```
> 1/(K1*K2):
                                      .0000231454631447068
> A1;
                                                  .4
> A2;
                                         270.944571763048
> A3:
                                                  0
> B1;
                                      .500000000000001 \ 10^{12}
> B2;
                                      .156314176017143\ 10^{15}
> B3:
                                                  0
  Note #6: Now let's solve for the integration constants: _C1, _C2, _C3, and _C4,
           using the boundary data.
> c1(x);
                .0000231454631447068 + .0000231454631447068
                      \sqrt{1 + 86410.0228842208} CI x + 86410.0228842208 CI C2
> c2(x);
                .0000231454631447068 - .0000231454631447068
                      \int 1 + 86410.0228842208 \ CI \ x + 86410.0228842208 \ CI \ C2
> evalf(c1(x),5);
                .000023146 + .000023146 \sqrt{1. + 86408} CI x + 86408 CI CZ
> evalf(c2(x),5);
                 .000023146 - .000023146 \sqrt{1. + 86408. \_C1 \ x + 86408. \_C1 \_C2}
> eqnset1:={0=c1(0),1e-6=c1(0.10)};
  eqnset1 := \begin{cases} .1 \ 10^{-5} = .0000231454631447068 + .0000231454631447068 \end{cases}
         \sqrt{1 + 8641.00228842208} CI + 86410.0228842208 CI C2, 0 = 
         .0000231454631447068 + .0000231454631447068 \sqrt{1 + 86410.0228842208 \_C1 \_C2}
> varset1:={_C1,_C2};
                                       varset1 := \{ \_C1, \_C2 \}
> solnset1:=solve(eqnset1,varset1);
                                             solnset1 :=
> eqnset2:={0=c2(0),1e-6=c2(0.10)};
  eqnset2 := \{0 =
         .0000231454631447068 - .0000231454631447068 \sqrt{1 + 86410.0228842208 \ \_C1 \ \_C2} \,,
```

 $.1\ 10^{-5} = .0000231454631447068$

- .0000231454631447068
$$\sqrt{1 + 8641.00228842208}$$
 _CI + 86410.0228842208 _CI _C2 }

> varset2:={_C1,_C2};

$$varset2 := \{ C1, C2 \}$$

> solnset2:=solve(eqnset2,varset2);

$$solnset2 := \{ _C2 = 0, _C1 = -.978397494278943 \ 10^{-5} \}$$

> assign(solnset2);

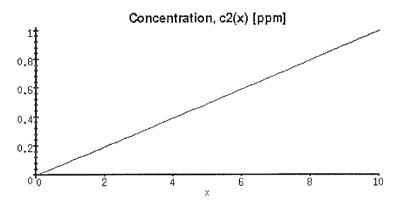
Note #7: The solution c1(x) is not capable of satisfying the boundary conditions, as evidenced by the lack of solution for the given boundary conditions. The concentration function c2(x) is the correct solution in this case!

> c2(x);

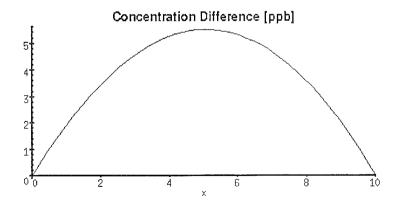
$$.0000231454631447068 - .0000231454631447068 \sqrt{1 - .845433498705078} x$$

> p1:=plot(1e6*c2(x/100),x=0..10,title='Concentration, c2(x) [ppm]'):

> p1;



> p2:=plot(1e9*(c2(0)+((c2(0.10)-c2(0))/0.10)*x/100-c2(x/100)), x=0..10, title='Concentration Difference [ppb]'): > p2; >



> u2(x);

$$-.015677843999998\ x + .00190196390663828\ \sqrt{1 - .845433498705078\ x}$$

$$- .00160798400000005\ \sqrt{1 - .845433498705078\ x}\ x + 2.49999999999998\ _C3\ x$$

$$+ .99999999999996\ _C4$$

> eqnset3:={0=u2(0),0=u2(0.10)};

$$eqnset3 := \{0 = .0000981540310014316 + .24999999999998 _C3 + .99999999999996 _C4, 0 = .00190196390663828 + .999999999999996 _C4\}$$

> varset3:={_C3,_C4};

$$varset3 := \{ _C3, _C4 \}$$

> solnset3:=solve(eqnset3,varset3);

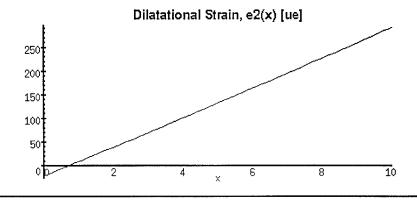
> assign(solnset3);

> e2(x);

$$-.0072359279999998\sqrt{1 - .845433498705078\,x} + .00721523950254741$$

> p3:=plot(1e6*e2(x/100),x=0..10,title='Dilatational Strain, e2(x) [ue]'):

> p3;

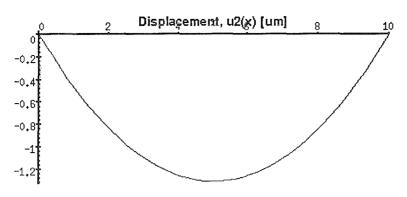


> u2(x);

.00236025475636850
$$x$$
 + .00190196390663828 $\sqrt{1}$ - .845433498705078 x - .00160798400000005 $\sqrt{1}$ - .845433498705078 x x - .00190196390663825

> p4:=plot(1e6*u2(x/100),x=0..10,title='Displacement, u2(x) [um]'):

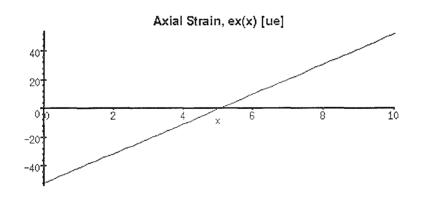
> p4;



> ex:=unapply(diff(u2(x),x),x);

$$ex := x \rightarrow .00236025475636850 - .00080399199999990 \frac{1}{\sqrt{1 - .845433498705078 \, x}} + .000679721769490914 \frac{x}{\sqrt{1 - .845433498705078 \, x}} - .00160798400000005 \sqrt{1 - .845433498705078 \, x}$$

> p5:=plot(1e6*ex(x/100),x=0..10,title='Axial Strain, ex(x) [ue]'): > p5; >



> Sx;

-.103442487262901 108

> J2(x);

-.978397494278943 10-14

> evalf(u2(x),5);

 $.0023588\ x + .0019015\ \sqrt{1. - .84542\ x}\ - .0016072\ \sqrt{1. - .84542\ x}\ x - .0019021$

> evalf(c2(x),5);

.000023146 - .000023146 $\sqrt{1. - .84542 \, x}$

> evalf(e2(x),5);

 $-.0072359\sqrt{1. - .84542 x} + .0072152$

> evalf(ex(x),5);

 $.0023603 - .00080399 \frac{1}{\sqrt{1. - .84543 \, x}} + .00067972 \frac{x}{\sqrt{1. - .84543 \, x}} - .0016080 \sqrt{1. - .84543 \, x}$

A-2a-15

1. MAPLE PROGRAM DIRECTORY AND FILE NAME: /afs/nd.edu/user4/jthomas5/Maple/Transport/1DSSsol#2b.ms

2. ORIGINATOR:

Dr. James P. Thomas University of Notre Dame Department of Aerospace and Mechanical Engineering 374 Fitzpatrick Hall Notre Dame; IN 46556-5637 (219) 631-9371

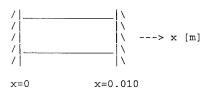
3. HISTORY:

Written: June-1994

Latest Revision: 13-Feb-1995

4. PROGRAM DESCRIPTION:

One-dimensional, steady-state analysis of the fully coupled solute transport and trapping equations using "plane stress" constitutive equations for stress. The particular problem solved is shown below:



Deformation variable boundary conditions: u(0)=0, u(0.10)=0Concentration variable boundary conditions: c(0)=0, c(0.10)=1e-7Reference concentration level: c0=0.0Trapping parameter: traps=19 (low trapping)

The equations used in the analysis are:

- 1. dJ/dx=0 (steady-state diffusion equation)
- 2. J=Ds*(K1*c*de/dx-dc/dx) (mass flux)
- 3. dS/dx=0 (steady-state deformation equation with zero body forces)
- 4. S=B1*e-B2*c+B3 (axial stress)
- 5. de/dx-K2*dc/dx=0 (dilational strain gradient)
- 6. e=A1*du/dx+A2*c-A3 (dilatational strain)

The constants used in the plane stress analysis are defined below:

Ds:=lattice diffusivity [m^2/sec]

K1 := (Vs * K) / (R * T)

```
Vs:= partial molar volume of solute [m^3/mol solute]
K:= bulk modulus=Em/(3*(1-2*nu)) [N/m^2]
Em:= Modulus of Elasticity [N/m^2]
nu:= Poisson's ratio [1]
R:= universal gas constant=8.31432 [J/mol-K]
T:= temperature [K]
B1:=3*K
B2:=9*K*alpha*(1+traps)
B3:=B2*co
traps:= Csr*Kr=trapping constant [1]
Csr:= saturation trap concentration [1]
Kr:= equilibrium trapping constant [1]
c0:= reference solute concentration [1]
K2:=3*alpha*(1+traps)
alpha:= (rho*Vs)/(3*MWs)=solute concentration expansion coefficient [m/m/Delc]
rho:= mass density of the solid [kg/m^3]
MWs:= molecular weight of the solute [kg/mol]
Delc:= c-c0 [1]
```

A1:=1-2*nu

A2:=2*(1+nu)*alpha*(1+traps)

A3:=A2*c0

5. REQUIRED INPUTS:

Ds:=lattice diffusivity [m^2/sec]

Vs:= partial molar volume of solute [m^3/mol solute]

traps:= Csr*Kr=trapping constant [1]

c0:= reference solute concentration [1]

MWs:= molecular weight of the solute [kg/mol]

Em: = Modulus of Elasticity [Pa]

nu:= Poisson's ratio [1]

rho:= mass density of the solid [kg/m^3]

T:= temperature [K]

6. SYMBOLIC ANALYSIS:

> Digits:=trunc(evalhf(Digits));

$$Digits := 15$$

> J:=Ds*(K1*K2*c(x)-1)*diff(c(x),x);

$$J := Ds (K1 \ K2 \ c(x) - 1) \left(\frac{\partial}{\partial x} c(x) \right)$$

> degn:=diff(J,x)=0;

$$deqn := Ds K1 K2 \left(\frac{\partial}{\partial x} c(x)\right)^{2} + Ds (K1 K2 c(x) - 1) \left(\frac{\partial^{2}}{\partial x^{2}} c(x)\right) = 0$$

> csoln:=dsolve(deqn,c(x),explicit);

$$csoln := c(x) = -\frac{_Cl\left(-\frac{1}{_Cl} - \frac{\sqrt{1 + 2 \ Kl \ K2 _Cl \ x + 2 \ Kl \ K2 _Cl \ _Cl}}{_Cl}\right)}{Kl \ K2}$$

$$c(x) = -\frac{_Cl\left(-\frac{1}{_Cl} + \frac{\sqrt{1 + 2 \ Kl \ K2 _Cl \ x + 2 \ Kl \ K2 \ _Cl \ _Cl}}{_Cl}\right)}{Kl \ K2}$$

Note #1: There are apparently two "roots" to the solution for $c(\mathbf{x})$. The boundary conditions will be used to select the proper one.

> c1:=unapply(simplify(op(2,csoln[1])),x);

$$c1 := x \rightarrow \frac{1 + \sqrt{1 + 2 \ K1 \ K2 \ _C1 \ x + 2 \ K1 \ K2 \ _C1 \ _C2}}{K1 \ K2}$$

> c1(x);

$$\frac{1 + \sqrt{1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2}}{K1 K2}$$

> c2:=unapply(simplify(op(2,csoln[2])),x);

$$c2 := x \to -\frac{-1 + \sqrt{1 + 2 KI K2 _CI x + 2 KI K2 _CI _C2}}{KI K2}$$

> c2(x);

$$\frac{-1 + \sqrt{1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2}}{K1 K2}$$

 \rightarrow J1:=unapply(simplify(Ds*(K1*K2*c1(x)-1)*diff(c1(x),x),x));

$$J1 := () \rightarrow Ds _C1$$

> J1(x);

> J2:=unapply(simplify(Ds*(K1*K2*c2(x)-1)*diff(c2(x),x)));

$$J2 := () \rightarrow Ds _C1$$

> J2(x);

$$Ds _C1$$

Note #2: The above results show that both mass flux solutions are identical and equal to Ds*_C1 where _C1 is a constant of integration!

> ex1:=dsolve(diff(e(x),x)-K2*diff(c1(x),x)=0,e(x));

$$ex1 := e(x) = \frac{\sqrt{1 + 2 K1 K2 C1 x + 2 K1 K2 C1 C2} + C3 K1}{K1}$$

> e1:=unapply(simplify(op(2,ex1)),x);

$$e1 := x \to \frac{\sqrt{1 + 2 K1 K2 C1 x + 2 K1 K2 C1 C2} + C3 K1}{K1}$$

> e1(x);

$$\frac{\int 1 + 2 \, KI \, K2 \, _CI \, x + 2 \, KI \, K2 \, _CI \, _C2}{KI}$$

> ex2:=dsolve(diff(e(x),x)-K2*diff(c2(x),x)=0,e(x));

$$ex2 := e(x) = -\frac{\sqrt{1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2} - _C3 K1}{K1}$$

> e2:=unapply(simplify(op(2,ex2)),x);

$$e2 := x \rightarrow \frac{-\sqrt{1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2} + _C3 K1}{K1}$$

> e2(x);

$$-\sqrt{1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2} + _C3 K1$$

> ux1:=dsolve(A1*diff(u(x),x)+A2*c1(x)-A3-e1(x)=0,u(x));

$$ux1 := u(x) = \frac{1}{3} \left(-3 \text{ A2 } x \text{ K1 } \text{ K2 } _\text{C1 - A2} \sqrt{\%1} - 2 \sqrt{\%1} \text{ A2 } \text{K1 } \text{ K2 } _\text{C1 } x \right)$$

$$-2 \sqrt{\%1} \text{ A2 } \text{K1 } \text{K2 } _\text{C1 } _\text{C2} + 3 \text{ A3 } x \text{ K1}^2 \text{ K2}^2 _\text{C1} + \sqrt{\%1} \text{ K2} + 2 \sqrt{\%1} \text{ K2}^2 \text{ K1 } _\text{C1 } x$$

$$+2 \sqrt{\%1} \text{ K2}^2 \text{ K1 } _\text{C1 } _\text{C2} + 3 \text{ _C3 } x \text{ K1}^2 \text{ K2}^2 \text{ _C1} + 3 \text{ _C4 } \text{A1 } \text{K1}^2 \text{ K2}^2 \text{ _C1} \right) / (\text{A1 } \text{K1}^2 \text{ K2}^2 \text{ _C1})$$

%1 := 1 + 2 K1 K2 C1 x + 2 K1 K2 C1 C2

> u1:=unapply(simplify(op(2,ux1)),x);

$$ul := x \rightarrow \frac{1}{3} \left(-3 \text{ A2 } x \text{ K1 K2 _C1 - A2} \sqrt{1 + 2 \text{ K1 K2 _C1 } x + 2 \text{ K1 K2 _C1 _C2}} \right.$$

$$- 2 \sqrt{1 + 2 \text{ K1 K2 _C1 } x + 2 \text{ K1 K2 _C1 _C2}} \text{ A2 K1 K2 _C1 } x$$

$$- 2 \sqrt{1 + 2 \text{ K1 K2 _C1 } x + 2 \text{ K1 K2 _C1 _C2}} \text{ A2 K1 K2 _C1 _C2 + 3 A3 } x \text{ K1}^2 \text{ K2}^2 \text{_C1}$$

$$+ \sqrt{1 + 2 \text{ K1 K2 _C1 } x + 2 \text{ K1 K2 _C1 _C2}} \text{ K2}$$

$$+ 2 \sqrt{1 + 2 \text{ K1 K2 _C1 } x + 2 \text{ K1 K2 _C1 _C2}} \text{ K2}^2 \text{ K1 _C1 } x$$

$$+ 2 \sqrt{1 + 2 \text{ K1 K2 _C1 } x + 2 \text{ K1 K2 _C1 _C2}} \text{ K2}^2 \text{ K1 _C1 _C2 + 3 _C3 } x \text{ K1}^2 \text{ K2}^2 \text{_C1}$$

$$+ 3 \text{_C4 A1 K1}^2 \text{ K2}^2 \text{_C1} / (\text{A1 K1}^2 \text{ K2}^2 \text{_C1})$$

> u1(x);

$$\frac{1}{3} \left(-3 A2 x K1 K2 _C1 - A2 \sqrt{\%1} - 2 \sqrt{\%1} A2 K1 K2 _C1 x - 2 \sqrt{\%1} A2 K1 K2 _C1 _C2 \right)$$

$$+ 3 A3 x K1^{2} K2^{2} _C1 + \sqrt{\%1} K2 + 2 \sqrt{\%1} K2^{2} K1 _C1 x + 2 \sqrt{\%1} K2^{2} K1 _C1 _C2$$

$$+ 3 _C3 x K1^{2} K2^{2} _C1 + 3 _C4 A1 K1^{2} K2^{2} _C1 \right) / (A1 K1^{2} K2^{2} _C1)$$

 $%1 := 1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2$

> ux2:=dsolve(A1*diff(u(x),x)+A2*c2(x)-A3-e2(x)=0,u(x));

$$ux2 := u(x) = \frac{1}{3} \left(-3 A2 x K1 K2 _C1 + A2 \sqrt{\%1} + 2 \sqrt{\%1} A2 K1 K2 _C1 x \right)$$

$$+ 2 \sqrt{\%1} A2 K1 K2 _C1 _C2 + 3 A3 x K1^2 K2^2 _C1 - \sqrt{\%1} K2 - 2 \sqrt{\%1} K2^2 K1 _C1 x$$

$$- 2 \sqrt{\%1} K2^2 K1 _C1 _C2 + 3 _C3 x K1^2 K2^2 _C1 + 3 _C4 A1 K1^2 K2^2 _C1 \right) / (A1 K1^2 K2^2 _C1)$$

$$%1 := 1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2$$

> u2:=unapply(simplify(op(2,ux2)),x);

$$u2 := x \rightarrow \frac{1}{3} \left(-3 A2 x K1 K2 _C1 + A2 \sqrt{1 + 2 K1 K2} _C1 x + 2 K1 K2 _C1 _C2 \right)$$

$$+ 2 \sqrt{1 + 2 K1 K2} _C1 x + 2 K1 K2 _C1 _C2 A2 K1 K2 _C1 x$$

$$+ 2 \sqrt{1 + 2 K1 K2} _C1 x + 2 K1 K2 _C1 _C2 A2 K1 K2 _C1 _C2 + 3 A3 x K1^2 K2^2 _C1 \right)$$

$$- \sqrt{1 + 2 K1 K2} _C1 x + 2 K1 K2 _C1 _C2 K2$$

$$- 2 \sqrt{1 + 2 K1 K2} _C1 x + 2 K1 K2 _C1 _C2 K2^2 K1 _C1 x$$

$$- 2 \sqrt{1 + 2 K1 K2} _C1 x + 2 K1 K2 _C1 _C2 K2^2 K1 _C1 x$$

$$+ 3 _C4 A1 K1^2 K2^2 _C1 \right) / (A1 K1^2 K2^2 _C1)$$

> u2(x);

$$\frac{1}{3} \left(-3 A2 x KI K2 _CI + A2 \sqrt{\%1} + 2 \sqrt{\%1} A2 KI K2 _CI x + 2 \sqrt{\%1} A2 KI K2 _CI _C2 \right)$$

$$+ 3 A3 x KI^{2} K2^{2} _CI - \sqrt{\%1} K2 - 2 \sqrt{\%1} K2^{2} KI _CI x - 2 \sqrt{\%1} K2^{2} KI _CI _C2$$

$$+ 3 _C3 x KI^{2} K2^{2} _CI + 3 _C4 AI KI^{2} K2^{2} _CI \right) / (AI KI^{2} K2^{2} _CI)$$

$$%1 := 1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2$$

> S1:=x->simplify(B1*e1(x)-B2*c1(x)+B3);

$$SI := x \rightarrow \text{simplify}(BI \text{ el}(x) - B2 \text{ cl}(x) + B3)$$

> S1(x);

$$\left(B1\ K2\ \sqrt{1+2\ K1\ K2\ _C1\ x+2\ K1\ K2\ _C1\ _C2} + B1\ K2\ _C3\ K1 - B2\right) \\ -B2\ \sqrt{1+2\ K1\ K2\ _C1\ x+2\ K1\ K2\ _C1\ _C2} + B3\ K1\ K2\right) / (K1\ K2)$$

> S2:=x->simplify(B1*e2(x)-B2*c2(x)+B3);

$$S2 := x \rightarrow \text{simplify}(B1 \text{ e2}(x) - B2 \text{ c2}(x) + B3)$$

> S2(x);

$$(-B1 K2 \sqrt{1 + 2 K1 K2 C1 x + 2 K1 K2 C1 C2 + B1 K2 C3 K1 - B2} + B2 \sqrt{1 + 2 K1 K2 C1 x + 2 K1 K2 C1 C2 + B3 K1 K2})/(K1 K2)$$

Note #3: At this point, we will introduce expressions for the constants and then simplify the results before quantifying the constants and determining the constants of integration.

> K:=(Em/(3*(1-2*nu)));

$$K := \frac{Em}{3-6v}$$

> lambda:=(Em*nu)/((1+nu)*(1-2*nu));

$$\lambda := \frac{Em \, v}{(1+v) (1-2 \, v)}$$

> mu:=Em/(2*(1+nu));

$$\mu := \frac{Em}{2 + 2v}$$

> alpha:=(rho*Vs)/(3*MWs);

$$\alpha := \frac{1}{3} \frac{\rho \ Vs}{MWs}$$

> K1:=(Vs*K)/(R*T);

$$K1 := \frac{Vs Em}{(3 - 6 \text{ V}) R T}$$

> K2:=3*alpha*(1+traps);

$$K2 := \frac{\rho \ Vs (1 + traps)}{MWs}$$

> K1*K2;

$$\frac{Vs^2 Em \rho (1 + traps)}{(3 - 6 v) R T MWs}$$

> A1:=(1-2*nu);

$$AI := 1 - 2 v$$

> A2:=2*(1+nu)*alpha*(1+traps);

$$A2 := \frac{2}{3} \frac{(1+v) \rho Vs (1+traps)}{MWs}$$

> A3:=A2*c0;

$$A3 := \frac{2}{3} \frac{(1+v) \rho \ Vs (1+traps) c\theta}{MWs}$$

> B1:=3*K;

$$BI := 3 \frac{Em}{3 - 6 v}$$

> B2:=9*K*alpha*(1+traps);

$$B2 := 3 \frac{Em \rho Vs (1 + traps)}{(3 - 6 \vee) MWs}$$

> B3:=B2*c0:

$$B3 := 3 \frac{Em \rho Vs (1 + traps) cO}{(3 - 6 v) MWs}$$

>e1(x);
$$\left(1 + \sqrt{1 + 2 \frac{Vs^2 \, Em \, \rho \, (1 + traps) \, _CI \, x}{(3 - 6 \, v) \, R \, T \, MWs}} + 2 \frac{Vs^2 \, Em \, \rho \, (1 + traps) \, _CI \, _C2}{(3 - 6 \, v) \, R \, T \, MWs} \right) (3 - 6 \, v) \, R \, T$$

$$\times \frac{MWs}{Vs^2 \, Em \, \rho \, (1 + traps)} \times \frac{CI \, x}{(3 - 6 \, v) \, R \, T \, MWs} + 2 \frac{Vs^2 \, Em \, \rho \, (1 + traps) \, _CI \, _C2}{(3 - 6 \, v) \, R \, T \, MWs} \right) (3 - 6 \, v) \, R \, T$$

$$\times \frac{MWs}{Vs^2 \, Em \, \rho \, (1 + traps) \, _CI \, x}{(3 - 6 \, v) \, R \, T \, MWs} + 2 \frac{Vs^2 \, Em \, \rho \, (1 + traps) \, _CI \, _C2}{(3 - 6 \, v) \, R \, T \, MWs} + 2 \frac{Vs^2 \, Em \, \rho \, (1 + traps) \, _CI \, _C2}{(3 - 6 \, v) \, R \, T \, MWs} + 2 \frac{C3 \, Vs \, Em}{(3 - 6 \, v) \, R \, T \, MWs} + 2 \frac{C3 \, Vs \, Em}{(3 - 6 \, v) \, R \, T \, MWs} + 2 \frac{C3 \, Vs \, Em}{(3 - 6 \, v) \, R \, T \, MWs} + 2 \frac{C3 \, Vs \, Em}{(3 - 6 \, v) \, R \, T \, MWs} + 2 \frac{C3 \, Vs \, Em}{(3 - 6 \, v) \, R \, T \, MWs} + 2 \frac{C3 \, Vs \, Em}{(3 - 6 \, v) \, R \, T \, MWs} + 2 \frac{C3 \, Vs \, Em}{(3 - 6 \, v) \, R \, T \, MWs} + 2 \frac{C3 \, Vs \, Em}{(3 - 6 \, v) \, R \, T \, MWs} + 2 \frac{C3 \, Vs \, Em}{(3 - 6 \, v) \, R \, T \, MWs} + 2 \frac{C3 \, Vs \, Em}{(3 - 6 \, v) \, R \, T \, MWs} + 2 \frac{C3 \, Vs \, Em}{(3 - 6 \, v) \, R \, T \, MWs} + 2 \frac{C3 \, Vs \, Em}{(3 - 6 \, v) \, R \, T \, MWs} + 2 \frac{C3 \, Vs \, Em}{(3 - 6 \, v) \, R \, T \, MWs} + 2 \frac{C3 \, Vs \, Em}{(3 - 6 \, v) \, R \, T \, MWs} + 2 \frac{C1 \, (1 + v) \, \rho^2 \, Vs^3 \, (1 + traps)^2 \, Em}{MWs^3 \, (3 - 6 \, v) \, R \, T} + 2 \frac{4 \, \sqrt{\%1 \, (1 + v) \, \rho^2 \, Vs^3 \, (1 + traps)^2 \, Em}_{CI \, x}}{MWs^3 \, (3 - 6 \, v) \, R \, T} + 2 \frac{\sqrt{\%1 \, \rho^2 \, Vs^3 \, (1 + traps)^2 \, Em}_{CI \, x}}{MWs^3 \, (3 - 6 \, v) \, R \, T} + 2 \frac{\sqrt{\%1 \, \rho^2 \, Vs^3 \, (1 + traps)^2 \, Em}_{CI \, x}}{MWs^3 \, (3 - 6 \, v) \, R \, T} + 2 \frac{\sqrt{\%1 \, \rho^2 \, Vs^3 \, (1 + traps)^2 \, Em}_{CI \, x}}{MWs^3 \, (3 - 6 \, v) \, R \, T} + 2 \frac{\sqrt{\%1 \, \rho^2 \, Vs^3 \, (1 + traps)^2 \, Em}_{CI \, x}}{MWs^3 \, (3 - 6 \, v)^2 \, R^2 \, T^2 \, MWs^2} + 2 \frac{\sqrt{\%1 \, \rho^2 \, Vs^3 \, (1 + traps)^2 \, Em}_{CI \, x}}{(3 - 6 \, v)^2 \, R^2 \, T^2 \, MWs^2} + 2 \frac{\sqrt{\%1 \, \rho^2 \, Vs^3 \, (1 + traps)^2 \, Em}_{CI \, x}}{(3 - 6 \, v)^2 \, R^2 \, T^2 \, MWs^2} + 2 \frac{\sqrt{\%1 \, \rho^2 \, Vs^3 \, (1 + traps)^2 \, Em}_{CI \, x}}{(3 - 6 \, v)^2 \, R^$$

$$\%1 := 1 + 2 \frac{Vs^2 Em \rho (1 + traps) _C1 x}{(3 - 6 v) R T MWs} + 2 \frac{Vs^2 Em \rho (1 + traps) _C1 _C2}{(3 - 6 v) R T MWs}$$

> S1(x);

-
$$(MWs _C3 \ Vs \ Em - 3 \ R \ T \ MWs + 6 \ R \ T \ MWs \ v + Em \ \rho \ Vs^2 \ c0 + Em \ \rho \ Vs^2 \ c0 \ traps)/($$
 $(-1 + 2 \ v) \ Vs \ MWs)$

> S2(x);

-
$$(MWs _C3 \ Vs \ Em - 3 \ R \ T \ MWs + 6 \ R \ T \ MWs \ v + Em \ \rho \ Vs^2 \ c0 + Em \ \rho \ Vs^2 \ c0 \ traps)/((-1 + 2 \ v) \ Vs \ MWs)$$

> S1(x)-S2(x);

0

Note #4: The stresses are same for each solution root, and do not depend on the position x! The stresses are therefore constant throughout the body. We define a new stress function, Sx.

> Sx:=S2(x);

$$Sx := -(MWs _C3 \ Vs \ Em - 3 \ R \ T \ MWs + 6 \ R \ T \ MWs \ v + Em \ \rho \ Vs^2 \ c0 + Em \ \rho \ Vs^2 \ c0 \ traps)/($$

$$(-1 + 2 \ v) \ Vs \ MWs)$$

Note #5: Now we will introduce numerical values for the material properties.

The values selected are typical for internal hydrogen as a solute in AISI 4340 steel.

> Ds:=1e-9;		
·	D 1.10-8	
	$Ds := .1 \ 10^{-8}$	
> Vs:=2.02e-6;		
	$Vs := .202 \ 10^{-5}$	
	V3202 10	
> traps:=19;		·
	<i>traps</i> := 19	
	maps: 19	
> c0:=0.0;		
	$c\theta := 0$	
- BANA/a:-0 00100707		
> MWs:=0.00100797;		
	MWs := .00100797	
> Em:=200e9;		
Z EIII20003,		
	$Em := .200 \ 10^{12}$	
> nu:=0.3;		
	2	
	v := .3	
> rho:=7800;		
	$\rho := 7800$	
	p .= 7000	
> T:=293;		
	T := 293	
Di0 21420		
> R:=8.31432;		
	R := 8.31432	
> K;		
> K,		
	.16666666666667 10 ¹²	
> lambda;		
,	115204615204616 1012	
	$.115384615384616\ 10^{12}$	
> mu;		
	.769230769230769 1011	
	(10)22010)23010) 10	
> alpha;		
	5.21047253390476	
> Rh:=R/MWs;		
> 1111.=11/11(175),		
	Rh := 8248.57882675080	
> K1;		
	120 100274500000	
	138.199274509089	
> K2;		
	312.628352034286	
******	J 1 2.020JJ 20J T 200	
> K1*K2;		
	43205.0114421104	

```
> 1/(K1*K2);
                                        .0000231454631447068
> A1;
                                                     .4
> A2;
                                           270.944571763048
> A3;
                                                     0
> B1;
                                         .500000000000001 \ 10^{12}
> B2;
                                         .156314176017143 1015
> B3:
                                                     0
  Note #6: Now let's solve for the integration constants: _C1, _C2, _C3, and _C4,
            using the boundary data.
> c1(x);
                 .0000231454631447068 + .0000231454631447068
                        \int 1 + 86410.0228842208 \ \_C1 \ x + 86410.0228842208 \ \_C1 \ \_C2
> c2(x);
                 .0000231454631447068 - .0000231454631447068
                       \int 1 + 86410.0228842208 \ C1 \ x + 86410.0228842208 \ C1 \ C2
> evalf(c1(x),5);
                 .000023146 + .000023146 \sqrt{1. + 86408. \_C1 x + 86408. \_C1 \_C2}
> evalf(c2(x),5);
                  .000023146 - .000023146 \sqrt{1. + 86408. \_C1 \ x + 86408. \_C1 \_C2}
> eqnset1:={0=c1(0),1e-7=c1(0.10)};
   eqnset1 := \begin{cases} .1 \ 10^{-6} = .0000231454631447068 + .0000231454631447068 \end{cases}
          \int 1 + 8641.00228842208 \ CI + 86410.0228842208 \ CI \ C2, 0 =
          .0000231454631447068 + .0000231454631447068 \sqrt{1 + 86410.0228842208 \ \_C1 \ \_C2} \right\}
> varset1:={_C1,_C2};
                                         varset1 := \{ \_C1, \_C2 \}
> solnset1:=solve(eqnset1,varset1);
                                               solnset1 :=
> eqnset2:={0=c2(0),1e-7=c2(0.10)};
   eqnset2 := \begin{cases} .1 \ 10^{-6} = .0000231454631447068 \end{cases}
          -.0000231454631447068 \sqrt{1 + 8641.00228842208} \ \_CI + 86410.0228842208 \ \_CI \ \_C2
```

$$, 0 =$$

 $.0000231454631447068 - .0000231454631447068 \sqrt{1 + 86410.0228842208 _C1 _C2}$ > varset2:={_C1,_C2};

$$varset2 := \{ _C1, _C2 \}$$

> solnset2:=solve(eqnset2,varset2);

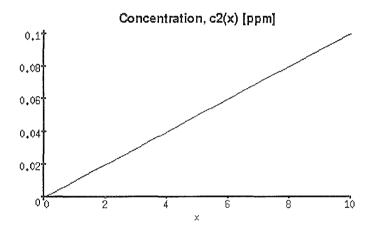
$$solnset2 := \{ C2 = 0, CI = -.997839749427893 \ 10^{-6} \}$$

> assign(solnset2);

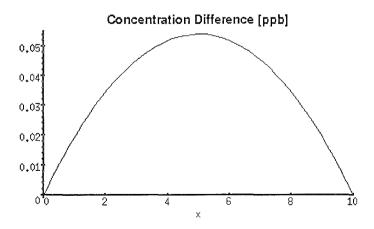
Note #7: The solution cl(x) is not capable of satisfying the boundary conditions, as evidenced by the lack of solution for the given boundary conditions. The concentration function cl(x) is the correct solution in this case!

> c2(x);

 $.0000231454631447068 - .0000231454631447068 \sqrt{1 - .0862233555828494 \ x} > p1:=plot(1e6*c2(x/100),x=0..10,title='Concentration, c2(x) [ppm]'): > p1;$



> p2:=plot(1e9*(c2(0)+((c2(0.10)-c2(0))/0.10)*x/100-c2(x/100)),x=0..10,title='Concentration Difference [ppb]'): > p2; > p2:



-.0156778440000000
$$x$$
 + .0186490538338527 $\sqrt{1}$ - .0862233555828494 x - .00160798400000002 $\sqrt{1}$ - .0862233555828494 x x + 2.49999999999999 _*C3* x + .9999999999999 *C4*

> eqnset3:={0=u2(0),0=u2(0.10)};

> varset3:={_C3,_C4};

$$varset3 := \{ _C3, _C4 \}$$

> solnset3:=solve(eqnset3,varset3);

$$solnset3 := \{ _C3 = .00723384531502524, _C4 = -.0186490538338529 \}$$

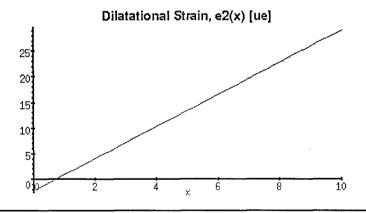
> assign(solnset3);

> e2(x);

$$-.0072359279999998 \sqrt{1 - .0862233555828494 x} + .00723384531502525$$

> p3:=plot(1e6*e2(x/100),x=0..10,title='Dilatational Strain, e2(x) [ue]'):

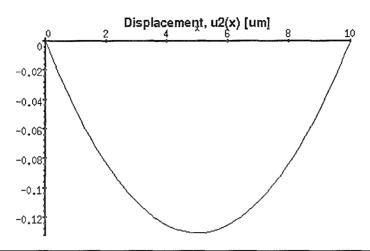
> p3;



> u2(x);

.00240676928756317
$$x$$
 + .0186490538338527 $\sqrt{1$ - .0862233555828494 x - .00160798400000002 $\sqrt{1}$ - .0862233555828494 x x - .0186490538338527

> p4:=plot(1e6*u2(x/100),x=0..10,title='Displacement, u2(x) [um]'): > p4; >



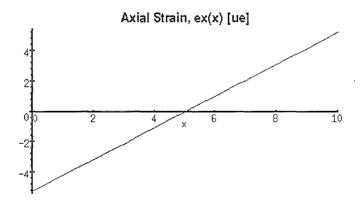
> ex:=unapply(diff(u2(x),x),x);

$$ex := x \rightarrow .00240676928756317 - .000803991999999991 \frac{1}{\sqrt{1 - .0862233555828494 \, x}}$$

$$+.0000693228881017671 \frac{x}{\sqrt{1 - .0862233555828494 \ x}}$$

$$-.00160798400000002\sqrt{1 -.0862233555828494}x$$

> p5:=plot(1e6*ex(x/100),x=0..10,title='Axial Strain, ex(x) [ue]'): > p5; >



 $-.104134248738053 \ 10^{7}$

> J2(x);

-.997839749427893 10⁻¹⁵

> evalf(u2(x),5);

$$.0024067 \ x + .018644 \ \sqrt{1. - .086222 \ x} - .0016072 \ \sqrt{1. - .086222 \ x} \ x - .018650$$

> evalf(c2(x),5);

$$.000023146 - .000023146 \sqrt{1. - .086222 x}$$

> evalf(e2(x),5);

$$-.0072359\sqrt{1.-.086222 x} + .0072338$$

> evalf(ex(x),5);

$$.0024068 - .00080399 \frac{1}{\sqrt{1. - .086223 \ x}} + .000069323 \frac{x}{\sqrt{1. - .086223 \ x}} - .0016080 \sqrt{1. - .086223 \ x}$$

1. MAPLE PROGRAM DIRECTORY AND FILE NAME: /afs/nd.edu/user4/jthomas5/Maple/Transport/1DSSsol#2c.ms

2. ORIGINATOR:

Dr. James P. Thomas University of Notre Dame Department of Aerospace and Mechanical Engineering 374 Fitzpatrick Hall Notre Dame, IN 46556-5637 (219) 631-9371

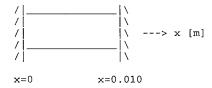
3. HISTORY:

Written: June-1994

Latest Revision: 13-Feb-1995

4. PROGRAM DESCRIPTION:

One-dimensional, steady-state analysis of the fully coupled solute transport and trapping equations using "plane stress" constitutive equations for stress. The particular problem solved is shown below:



Deformation variable boundary conditions: u(0)=0, u(0.10)=0Concentration variable boundary conditions: c(0)=0, c(0.10)=1e-7Reference concentration level: c0=0.0Trapping parameter: traps=499 (high trapping)

The equations used in the analysis are:

- dJ/dx=0 (steady-state diffusion equation)
- 2. J=Ds*(K1*c*de/dx-dc/dx) (mass flux)
- 3. dS/dx=0 (steady-state deformation equation with zero body forces)
- 4. S=B1*e-B2*c+B3 (axial stress)
- 5. de/dx-K2*dc/dx=0 (dilational strain gradient)
- 6. e=A1*du/dx+A2*c-A3 (dilatational strain)

The constants used in the plane stress analysis are defined below:

 $Ds:=lattice \ diffusivity \ [\texttt{m}^2/\texttt{sec}]$

Delc:= c-c0 [1]

K1 := (Vs*K) / (R*T)Vs:= partial molar volume of solute [m^3/mol solute] $K:= bulk modulus=Em/(3*(1-2*nu)) [N/m^2]$ Em:= Modulus of Elasticity [N/m^2] nu:= Poisson's ratio [1] R:= universal gas constant=8.31432 [J/mol-K] T:= temperature [K] B1:=3*K B2:=9*K*alpha*(1+traps) B3:=B2*co traps:= Csr*Kr=trapping constant [1] Csr:= saturation trap concentration [1] Kr:= equilibrium trapping constant [1] c0:= reference solute concentration [1] K2:=3*alpha*(1+traps) alpha:= (rho*Vs)/(3*MWs)=solute concentration expansion coefficient [m/m/Delc] rho:= mass density of the solid $[kg/m^3]$ MWs:= molecular weight of the solute [kg/mol]

A1:=1-2*nu A2:=2*(1+nu)*alpha*(1+traps) A3:=A2*c0

5. REQUIRED INPUTS:

Ds:=lattice diffusivity [m^2/sec]
Vs:= partial molar volume of solute [m^3/mol solute]
traps:= Csr*Kr=trapping constant [1]
c0:= reference solute concentration [1]
MWs:= molecular weight of the solute [kg/mol]

Em:= Modulus of Elasticity [Pa]
nu:= Poisson's ratio [1]
rho:= mass density of the solid [kg/m^3]

T:= temperature [K]

6. SYMBOLIC ANALYSIS:

> Digits:=trunc(evalhf(Digits));

$$Digits := 15$$

> J:=Ds*(K1*K2*c(x)-1)*diff(c(x),x);

$$J := Ds (KI \ K2 \ c(x) - 1) \left(\frac{\partial}{\partial x} c(x) \right)$$

> deqn:=diff(J,x)=0;

$$deqn := Ds KI K2 \left(\frac{\partial}{\partial x} c(x)\right)^2 + Ds (KI K2 c(x) - 1) \left(\frac{\partial^2}{\partial x^2} c(x)\right) = 0$$

> csoln:=dsolve(deqn,c(x),explicit);

$$csoln := c(x) = -\frac{CI\left(-\frac{1}{CI} - \frac{\sqrt{1 + 2 KI K2 _CI x + 2 KI K2 _CI _C2}}{CI}\right)}{KI K2}$$

$$c(x) = -\frac{CI\left(-\frac{1}{CI} + \frac{\sqrt{1 + 2 KI K2 _CI x + 2 KI K2 _CI _C2}}{CI}\right)}{KI K2}$$

Note #1: There are apparently two "roots" to the solution for $c(\mathbf{x})$. The boundary conditions will be used to select the proper one.

> c1:=unapply(simplify(op(2,csoln[1])),x);

$$c1 := x \to \frac{1 + \sqrt{1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2}}{K1 K2}$$

> c1(x);

$$\frac{1 + \sqrt{1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2}}{K1 K2}$$

> c2:=unapply(simplify(op(2,csoln[2])),x);

$$c2 := x \to -\frac{-1 + \sqrt{1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2}}{K1 K2}$$

> c2(x);

$$-\frac{-1 + \sqrt{1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2}}{K1 K2}$$

 \rightarrow J1:=unapply(simplify(Ds*(K1*K2*c1(x)-1)*diff(c1(x),x),x));

$$J1 := () \rightarrow Ds _C1$$

> J1(x);

> J2:=unapply(simplify(Ds*(K1*K2*c2(x)-1)*diff(c2(x),x)));

$$J2 := () \rightarrow Ds _C1$$

> J2(x);

Note #2: The above results show that both mass flux solutions are identical and equal to Ds*_C1 where _C1 is a constant of integration!

> ex1:=dsolve(diff(e(x),x)-K2*diff(c1(x),x)=0,e(x));

$$ex1 := e(x) = \frac{\sqrt{1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2} + _C3 K1}{K1}$$

> e1:=unapply(simplify(op(2,ex1)),x);

$$e1 := x \rightarrow \frac{\sqrt{1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2} + _C3 K1}{K1}$$

> e1(x);

$$\frac{\int 1 + 2 \ Kl \ K2 \ _Cl \ x + 2 \ Kl \ K2 \ _Cl \ _C2}{Kl} + _C3 \ Kl}$$

> ex2:=dsolve(diff(e(x),x)-K2*diff(c2(x),x)=0,e(x));

$$ex2 := e(x) = \frac{-\sqrt{1 + 2 K1 K2 C1 x + 2 K1 K2 C1 C2} + C3 K1}{K1}$$

> e2:=unapply(simplify(op(2,ex2)),x);

$$e2 := x \to -\frac{\sqrt{1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2} - _C3 K1}{K1}$$

> e2(x);

$$-\frac{\sqrt{1+2 \ K1 \ K2 \ _C1 \ x+2 \ K1 \ K2 \ _C1 \ _C2} - _C3 \ K1}{K1}$$

> ux1:=dsolve(A1*diff(u(x),x)+A2*c1(x)-A3-e1(x)=0,u(x));

$$uxI := u(x) = \frac{1}{3} \left(-3 \text{ A2 } x \text{ K1 } \text{ K2 } _\text{C1 - A2} \sqrt{\%1} - 2 \sqrt{\%1} \text{ A2 } \text{K1 } \text{ K2 } _\text{C1 } x \right)$$

$$- 2 \sqrt{\%1} \text{ A2 } \text{K1 } \text{K2 } _\text{C1 } _\text{C2} + 3 \text{ A3 } x \text{ K1}^2 \text{ K2}^2 _\text{C1} + \sqrt{\%1} \text{ K2} + 2 \sqrt{\%1} \text{ K2}^2 \text{ K1 } _\text{C1 } x$$

$$+ 2 \sqrt{\%1} \text{ K2}^2 \text{ K1 } _\text{C1 } _\text{C2} + 3 \text{ _C3 } x \text{ K1}^2 \text{ K2}^2 \text{ _C1} + 3 \text{ _C4 } \text{A1 } \text{K1}^2 \text{ K2}^2 \text{ _C1} \right) / (\text{A1 } \text{K1}^2 \text{ K2}^2 \text{ _C1})$$

 $%1 := 1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2$

> u1:=unapply(simplify(op(2,ux1)),x);

$$ul := x \rightarrow -\frac{1}{3} \left(3 \text{ A2 } x \text{ K1 K2 _C1} + \text{A2} \sqrt{1 + 2 \text{ K1 K2 _C1} x + 2 \text{ K1 K2 _C1 _C2}} \right.$$

$$+ 2 \sqrt{1 + 2 \text{ K1 K2 _C1} x + 2 \text{ K1 K2 _C1 _C2}} \text{ A2 K1 K2 _C1 } x$$

$$+ 2 \sqrt{1 + 2 \text{ K1 K2 _C1} x + 2 \text{ K1 K2 _C1 _C2}} \text{ A2 K1 K2 _C1 _C2 - 3 A3 } x \text{ K1}^2 \text{ K2}^2 \text{ _C1}}$$

$$- \sqrt{1 + 2 \text{ K1 K2 _C1} x + 2 \text{ K1 K2 _C1 _C2}} \text{ K2}$$

$$- 2 \sqrt{1 + 2 \text{ K1 K2 _C1} x + 2 \text{ K1 K2 _C1 _C2}} \text{ K2}^2 \text{ K1 _C1 } x$$

$$- 2 \sqrt{1 + 2 \text{ K1 K2 _C1} x + 2 \text{ K1 K2 _C1 _C2}} \text{ K2}^2 \text{ K1 _C1 _C2 - 3 _C3 } x \text{ K1}^2 \text{ K2}^2 \text{ _C1}}$$

$$- 3 \text{_C4 A1 K1}^2 \text{ K2}^2 \text{_C1} / (\text{A1 K1}^2 \text{ K2}^2 \text{_C1})$$

> u1(x);

$$-\frac{1}{3} \left(3 \text{ A2 x K1 K2 _C1} + \text{A2} \sqrt{\%1} + 2 \sqrt{\%1} \text{ A2 K1 K2 _C1 x} + 2 \sqrt{\%1} \text{ A2 K1 K2 _C1 _C2} \right.$$

$$-3 \text{ A3 x K1}^{2} \text{ K2}^{2} \text{ _C1} - \sqrt{\%1} \text{ K2} - 2 \sqrt{\%1} \text{ K2}^{2} \text{ K1 _C1 x} - 2 \sqrt{\%1} \text{ K2}^{2} \text{ K1 _C1 _C2}$$

$$-3 \text{ _C3 x K1}^{2} \text{ K2}^{2} \text{ _C1} - 3 \text{ _C4 A1 K1}^{2} \text{ K2}^{2} \text{ _C1} \right) / (\text{A1 K1}^{2} \text{ K2}^{2} \text{ _C1})$$

 $%1 := 1 + 2 KI K2 _CI x + 2 KI K2 _CI _C2$

> ux2:=dsolve(A1*diff(u(x),x)+A2*c2(x)-A3-e2(x)=0,u(x));

$$ux2 := u(x) = \frac{1}{3} \left(-3 A2 x K1 K2 _C1 + A2 \sqrt{\%1} + 2 \sqrt{\%1} A2 K1 K2 _C1 x \right)$$

$$+ 2 \sqrt{\%1} A2 K1 K2 _C1 _C2 + 3 A3 x K1^2 K2^2 _C1 - \sqrt{\%1} K2 - 2 \sqrt{\%1} K2^2 K1 _C1 x$$

$$- 2 \sqrt{\%1} K2^2 K1 _C1 _C2 + 3 _C3 x K1^2 K2^2 _C1 + 3 _C4 A1 K1^2 K2^2 _C1 \right) / (A1 K1^2 K2^2 _C1)$$

$$%1 := 1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2$$

> u2:=unapply(simplify(op(2,ux2)),x);

$$u2 := x \rightarrow \frac{1}{3} \left(-3 \text{ A2 } x \text{ K1 K2 _C1} + \text{A2} \sqrt{1 + 2 \text{ K1 K2 _C1} x + 2 \text{ K1 K2 _C1} _C2} \right.$$

$$+ 2 \sqrt{1 + 2 \text{ K1 K2 _C1} x + 2 \text{ K1 K2 _C1} _C2} \text{ A2 K1 K2 _C1 } x$$

$$+ 2 \sqrt{1 + 2 \text{ K1 K2 _C1} x + 2 \text{ K1 K2 _C1} _C2} \text{ A2 K1 K2 _C1} _C2 + 3 \text{ A3 } x \text{ K1}^2 \text{ K2}^2 _C1}$$

$$- \sqrt{1 + 2 \text{ K1 K2 _C1} x + 2 \text{ K1 K2} _C1} _C2 \text{ K2}$$

$$- 2 \sqrt{1 + 2 \text{ K1 K2} _C1} x + 2 \text{ K1 K2} _C1 _C2} \text{ K2}^2 \text{ K1} _C1 x$$

$$- 2 \sqrt{1 + 2 \text{ K1 K2} _C1} x + 2 \text{ K1 K2} _C1 _C2} \text{ K2}^2 \text{ K1} _C1 _C2 + 3 _C3 x \text{ K1}^2 \text{ K2}^2 _C1}$$

$$+ 3 _C4 \text{ A1 K1}^2 \text{ K2}^2 _C1 / (\text{A1 K1}^2 \text{ K2}^2 _C1)$$

> u2(x);

$$\frac{1}{3} \left(-3 A2 x K1 K2 _C1 + A2 \sqrt{\%1} + 2 \sqrt{\%1} A2 K1 K2 _C1 x + 2 \sqrt{\%1} A2 K1 K2 _C1 _C2 + 3 A3 x K1^2 K2^2 _C1 - \sqrt{\%1} K2 - 2 \sqrt{\%1} K2^2 K1 _C1 x - 2 \sqrt{\%1} K2^2 K1 _C1 _C2 + 3 _C3 x K1^2 K2^2 _C1 + 3 _C4 A1 K1^2 K2^2 _C1 \right) / (A1 K1^2 K2^2 _C1)$$

$$%1 := 1 + 2 K1 K2 _C1 x + 2 K1 K2 _C1 _C2$$

> S1:=x->simplify(B1*e1(x)-B2*c1(x)+B3);

$$S1 := x \rightarrow \text{simplify}(B1 \text{ e1}(x) - B2 \text{ c1}(x) + B3)$$

> S1(x);

$$\left(B1 \ K2 \sqrt{1 + 2 \ K1 \ K2 \ _C1 \ x + 2 \ K1 \ K2 \ _C1 \ _C2} + B1 \ K2 \ _C3 \ K1 - B2 - B2 \sqrt{1 + 2 \ K1 \ K2 \ _C1 \ x + 2 \ K1 \ K2 \ _C1 \ _C2} + B3 \ K1 \ K2 \right) / (K1 \ K2)$$

> S2:=x->simplify(B1*e2(x)-B2*c2(x)+B3);

$$S2 := x \rightarrow \text{simplify}(B1 \text{ e2}(x) - B2 \text{ c2}(x) + B3)$$

> S2(x);

$$- \left(B1 \ K2 \sqrt{1 + 2 \ K1 \ K2 \ _C1 \ x + 2 \ K1 \ K2 \ _C1 \ _C2} - B1 \ K2 \ _C3 \ K1 + B2 - B2 \sqrt{1 + 2 \ K1 \ K2 \ _C1 \ x + 2 \ K1 \ K2 \ _C1 \ _C2} - B3 \ K1 \ K2 \right) / (K1 \ K2)$$

Note #3: At this point, we will introduce expressions for the constants and then simplify the results before quantifying the constants and determining the constants of integration.

> K:=(Em/(3*(1-2*nu)));

$$K := \frac{Em}{3-6v}$$

> lambda:=(Em*nu)/((1+nu)*(1-2*nu));

$$\lambda := \frac{Em \, v}{(1+v)(1-2 \, v)}$$

> mu:=Em/(2*(1+nu));

$$\mu := \frac{Em}{2 + 2v}$$

> alpha:=(rho*Vs)/(3*MWs);

$$\alpha := \frac{1}{3} \frac{\rho \ Vs}{MWs}$$

> K1:=(Vs*K)/(R*T);

$$K1 := \frac{Vs Em}{(3 - 6 v) R T}$$

> K2:=3*alpha*(1+traps);

$$K2 := \frac{\rho \ Vs \ (1 + traps)}{MWs}$$

> K1*K2;

$$\frac{Vs^2 Em \rho (1 + traps)}{(3 - 6 v) R T MWs}$$

> A1:≈(1-2*nu);

$$AI := 1 - 2 v$$

> A2:=2*(1+nu)*alpha*(1+traps);

$$A2 := \frac{2}{3} \frac{(1+v) \rho Vs (1+traps)}{MWs}$$

> A3:=A2*c0;

$$A3 := \frac{2}{3} \frac{(1+v) \rho Vs (1+traps) cO}{MWs}$$

> B1:≈3*K;

$$BI := 3 \frac{Em}{3 - 6 v}$$

> B2:=9*K*alpha*(1+traps);

$$B2 := 3 \frac{Em \rho \ Vs (1 + traps)}{(3 - 6 \ v) \ MWs}$$

> B3:=B2*c0;

$$B3 := 3 \frac{Em \rho Vs (1 + traps) cO}{(3 - 6 v) MWs}$$

>et(x);
$$\left(1 + \sqrt{1 + 2 \frac{Vs^2 Em p (1 + traps) CI x}{(3 - 6 v) R T MWs}} + 2 \frac{Vs^2 Em p (1 + traps) CI C2}{(3 - 6 v) R T MWs} \right) (3 - 6 v) R T$$

$$MWs/(Vs^2 Em p (1 + traps)) > e2(x);$$

$$- \left(-1 + \sqrt{1 + 2 \frac{Vs^2 Em p (1 + traps) CI x}{(3 - 6 v) R T MWs}} + 2 \frac{Vs^2 Em p (1 + traps) CI C2}{(3 - 6 v) R T MWs} \right) (3 - 6 v) R T$$

$$MWs/(Vs^2 Em p (1 + traps)) > e1(x);$$

$$\left(\sqrt{1 + 2 \frac{Vs^2 Em p (1 + traps) CI x}{(3 - 6 v) R T MWs}} + 2 \frac{Vs^2 Em p (1 + traps) CI C2}{(3 - 6 v) R T MWs} + \frac{C3 Vs Em}{(3 - 6 v) R T} \right)$$

$$+ \frac{C3 Vs Em}{(3 - 6 v) R T MWs} + 2 \frac{Vs^2 Em p (1 + traps) CI C2}{(3 - 6 v) R T MWs} + \frac{C3 Vs Em}{(3 - 6 v) R T} \right)$$

$$+ \frac{Vs^2 Em p (1 + traps) CI x}{(3 - 6 v) R T MWs} + 2 \frac{Vs^2 Em p (1 + traps) CI C2}{(3 - 6 v) R T MWs} - \frac{C3 Vs Em}{(3 - 6 v) R T} \right)$$

$$+ \frac{1}{3} \left(2 \frac{(1 + v) p^2 Vs^3 (1 + traps)^2 x Em CI}{MWs^2 (3 - 6 v) R T} + \frac{2}{3} \frac{(1 + v) p Vs (1 + traps) \sqrt{\%1}}{MWs} \right)$$

$$+ \frac{4}{3} \frac{\sqrt{\%1} (1 + v) p^2 Vs^3 (1 + traps)^2 Em CI x}{MWs^2 (3 - 6 v) R T} + \frac{2}{3} \frac{(1 + v) p Vs (1 + traps) \sqrt{\%1}}{MWs}$$

$$+ \frac{4}{3} \frac{\sqrt{\%1} (1 + v) p^2 Vs^3 (1 + traps)^2 Em CI C2}{MWs^3 (3 - 6 v) R T} - 2 \frac{(1 + v) p^3 Vs^5 (1 + traps)^3 Co x Em^2 CI}{MWs^3 (3 - 6 v) R T} - 2 \frac{(1 + v) p^3 Vs^5 (1 + traps)^3 Em CI C2}{MWs^3 (3 - 6 v) R T} - 2 \frac{\sqrt{\%1} p^2 Vs^3 (1 + traps)^2 Em CI C2}{MWs^3 (3 - 6 v) R T} - 3 \frac{C3 x Vs^4 Em^2 p^2 (1 + traps)^2 CI}{(3 - 6 v)^2 R^2 T^2 MWs^2} - 3 \frac{C4 (1 - 2 v) Vs^4 Em^2 p^2 (1 + traps)^2 CI}{(3 - 6 v)^2 R^2 T^2 MWs^2} - 3 \frac{C4 (1 - 2 v) Vs^4 Em^2 p^2 (1 + traps)^2 CI}{(3 - 6 v)^2 R^2 T^2 MWs^2} - 3 \frac{C4 (1 - 2 v) Vs^4 Em^2 p^2 (1 + traps)^2 CI}{(3 - 6 v)^2 R^2 T^2 MWs^2} - 3 \frac{C4 (1 - 2 v) Vs^4 Em^2 p^2 (1 + traps)^2 CI}{(3 - 6 v)^2 R^2 T^2 MWs^2} - 3 \frac{C4 (1 - 2 v) Vs^4 Em^2 p^2 (1 + traps)^2 CI}{(3 - 6 v)^2 R^2 T^2 MWs^2} - 3 \frac{C4 (1 - 2 v) Vs^4 Em^2 p^2 (1 + traps)^2 CI}{(3 - 6 v)^2 R^2 T^2 MWs^2} - 3 \frac{C4 (1 - 2 v) Vs^4 Em^2 p^2 (1 + traps)^2 CI}{(3 - 6 v)^2 R^2 T^2 MWs^2} - 3 \frac{C4 (1 - 2 v) Vs^4 Em^2 p^2 (1 + traps)^2 CI}{(3 - 6 v)^2 R^2 T^2 MWs^2} - 3 \frac{C4 (1 -$$

> u2(x);
$$\frac{1}{3} \left(-2 \frac{(1+v) \rho^2 V s^3 (1 + traps)^2 x Em_CI}{MW s^2 (3 - 6v) R T} + \frac{2}{3} \frac{(1+v) \rho V s (1 + traps) \sqrt{\%1}}{MW s} \right) + \frac{4}{3} \frac{\sqrt{\%1} (1+v) \rho^2 V s^3 (1 + traps)^2 Em_CI x}{MW s^2 (3 - 6v) R T} + \frac{4}{3} \frac{\sqrt{\%1} (1+v) \rho^2 V s^3 (1 + traps)^2 Em_CI_C2}{MW s^2 (3 - 6v) R T} + 2 \frac{(1+v) \rho^3 V s^5 (1 + traps)^3 cO x Em^2_CI}{MW s^3 (3 - 6v)^2 R^2 T^2} - \frac{\sqrt{\%1} \rho V s (1 + traps)}{MW s} - 2 \frac{\sqrt{\%1} \rho^2 V s^3 (1 + traps)^2 Em_CI x}{MW s^2 (3 - 6v) R T} - 2 \frac{\sqrt{\%1} \rho^2 V s^3 (1 + traps)^2 Em_CI_C2}{MW s^2 (3 - 6v) R T} + 3 \frac{-C3 x V s^4 Em^2 \rho^2 (1 + traps)^2 _CI}{(3 - 6v)^2 R^2 T^2 MW s^2} + 3 \frac{-C4 (1 - 2v) V s^4 Em^2 \rho^2 (1 + traps)^2 _CI}{(3 - 6v)^2 R^2 T^2 MW s^2} + 2 \frac{V s^2 Em \rho (1 + traps)^2 _CI}{(3 - 6v) R T MW s}$$

$$\%1 := 1 + 2 \frac{V s^2 Em \rho (1 + traps)_CI x}{(3 - 6v) R T MW s} + 2 \frac{V s^2 Em \rho (1 + traps)_CI_C2}{(3 - 6v) R T MW s}$$

> S1(x);

$$- (MWs _C3 Vs Em - 3 R T MWs + 6 R T MWs v + Em \rho Vs^2 c0 + Em \rho Vs^2 c0 traps) / ((-1 + 2 v) Vs MWs)$$

> S2(x);

-
$$(MWs _C3 \ Vs \ Em - 3 \ R \ T \ MWs + 6 \ R \ T \ MWs \ v + Em \ \rho \ Vs^2 \ c0 + Em \ \rho \ Vs^2 \ c0 \ traps)/($$

$$(-1 + 2 \ v) \ Vs \ MWs)$$

> S1(x)-S2(x);

0

Note #4: The stresses are same for each solution root, and do not depend on the position x! The stresses are therefore constant throughout the body. We define a new stress function, Sx.

> Sx:=S2(x);

$$Sx := -(MWs _C3 \ Vs \ Em - 3 \ R \ T \ MWs + 6 \ R \ T \ MWs \ v + Em \ \rho \ Vs^2 \ c0 + Em \ \rho \ Vs^2 \ c0 \ traps)/($$

$$(-1 + 2 \ v) \ Vs \ MWs)$$

Note #5: Now we will introduce numerical values for the material properties.

The values selected are typical for internal hydrogen as a solute in AISI 4340 steel.

> Ds:=1e-9;		
	$Ds := .1 \ 10^{-8}$	
> Vs:=2.02e-6;		
	$Vs := .202 \ 10^{-5}$	
> traps:=499;		
	<i>traps</i> := 499	
> c0:=0.0;		
	$c\theta := 0$	
> MWs:=0.00100797;		
	MWs := .00100797	
> Em:=200e9;		
	$Em := .200 \ 10^{12}$	
> nu:=0.3;		
	v := .3	
> rho:=7800;		
	$\rho := 7800$	
> T:=293;		
	T := 293	
> R:=8.31432;		
	R := 8.31432	
> K;		
	.16666666666667 10 ¹²	
> lambda;		
> lambda;	$.115384615384616\ 10^{12}$	
> lambda; > mu;		
	.115384615384616 10 ¹² .769230769230769 10 ¹¹	
	.769230769230769 1011	
> mu;		
> mu;	.769230769230769 1011	
> mu; > alpha;	.769230769230769 1011	
> mu; > alpha;	.769230769230769 10 ¹¹ 5.21047253390476	
> mu; > alpha; > Rh:=R/MWs;	.769230769230769 10 ¹¹ 5.21047253390476	
> mu; > alpha; > Rh:=R/MWs;	.769230769230769 10 ¹¹ 5.21047253390476 Rh := 8248.57882675080	
> mu; > alpha; > Rh:=R/MWs; > K1;	.769230769230769 10 ¹¹ 5.21047253390476 Rh := 8248.57882675080	
> mu; > alpha; > Rh:=R/MWs; > K1;	$.769230769230769 10^{11}$ 5.21047253390476 $Rh := 8248.57882675080$ 138.199274509089 7815.70880085715	
> mu; > alpha; > Rh:=R/MWs; > K1; > K2;	$.769230769230769 \ 10^{11}$ 5.21047253390476 $Rh := 8248.57882675080$ 138.199274509089	
> mu; > alpha; > Rh:=R/MWs; > K1; > K2;	$.769230769230769 10^{11}$ 5.21047253390476 $Rh := 8248.57882675080$ 138.199274509089 7815.70880085715	

```
> A1;
                                                       .4
> A2;
                                             6773.61429407620
> A3;
                                                       0
> B1;
                                          .500000000000001 1012
> B2;
                                          .390785440042860\ 10^{16}
> B3:
                                                       0
  Note #6: Now let's solve for the integration constants: _C1, _C2, _C3, and _C4,
             using the boundary data.
> c1(x);
             .925818525788271\ 10^{-6} + .925818525788271\ 10^{-6}
                   \sqrt{1 + .216025057210552 \cdot 10^7} _C1 x + .216025057210552 10^7 _C1 _C2
> c2(x);
            .925818525788271\ 10^{-6} - .925818525788271\ 10^{-6}
                   \sqrt{1 + .216025057210552 \cdot 10^7 - CI \cdot x + .216025057210552 \cdot 10^7 - CI - C2}
> evalf(c1(x),5);
              .92584 10^{-6} + .92584 10^{-6}\sqrt{1. + .21602} \ 10^{7} _C1 x + .21602 10^{7} _C1 _C2
> evalf(c2(x),5);
              .92584 10<sup>-6</sup> - .92584 10<sup>-6</sup> \sqrt{1. + .21602 \ 10^7 \ \_C1 \ x + .21602 \ 10^7 \ \_C1 \ \_C2}
> egnset1:={0=c1(0),1e-7=c1(0.10)};
         eqnset1 := \{ 0 = .925818525788271 \ 10^{-6} \}
                 +.925818525788271\ 10^{-6}\sqrt{1+.216025057210552\ 10^{7}\ C1\ C2}, .1 10^{-6} =
                .925818525788271\ 10^{-6} + .925818525788271\ 10^{-6}
                 1 + 216025.057210552 \_CI + .216025057210552 10^7 \_CI \_C2
> varset1:={_C1,_C2};
                                          varset1 := \{ C2, C1 \}
> solnset1:=solve(eqnset1,varset1);
                                                solnset1 :=
> eqnset2:={0=c2(0),1e-7=c2(0.10)};
            eqnset2 := \{ .1 \ 10^{-6} = .925818525788271 \ 10^{-6} - .925818525788271 \ 10^{-6} \} 
                   \sqrt{1 + 216025.057210552} CI + .216025057210552 10^7 CI CZ, 0 =
```

$$.925818525788271\ 10^{-6}$$

$$-.925818525788271\ 10^{-6} \sqrt{1 + .216025057210552\ 10^7 \ C1\ C2}$$

> varset2:={_C1,_C2};

$$varset2 := \{ _C2, _C1 \}$$

> solnset2:=solve(eqnset2,varset2);

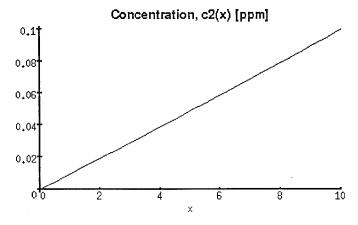
$$solnset2 := \{ _C2 = 0, _C1 = -.945993735697361 \ 10^{-6} \}$$

> assign(solnset2);

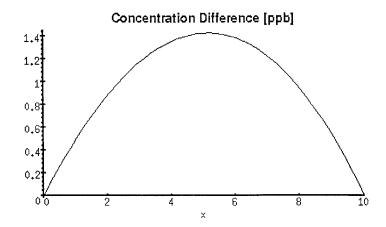
Note #7: The solution c1(x) is not capable of satisfying the boundary conditions, as evidenced by the lack of solution for the given boundary conditions. The concentration function c2(x) is the correct solution in this case!

> c2(x);

.925818525788271 10⁻⁶ - .925818525788271 10⁻⁶ $\sqrt{1 - 2.04358350874846 \ x}$ > p1:=plot(1e6*c2(x/100),x=0..10,title='Concentration, c2(x) [ppm]'): > p1;



 $> p2:=plot(1e9*(c2(0)+((c2(0.10)-c2(0))/0.10)*x/100-c2(x/100)), x=0..10, title='Concentration \ Difference \ [ppb]'): > p2;$



> u2(x);

 $> eqnset3:={0=u2(0),0=u2(0.10)};$

$$varset3 := \{ _C3, _C4 \}$$

> solnset3:=solve(eqnset3,varset3);

$$solnset3 := \{ _C4 = -.000786845261334466, _C3 = .00718481481807253 \}$$

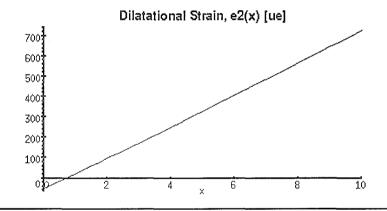
> assign(solnset3);

> e2(x);

$$-.00723592799999998\sqrt{1-2.04358350874846\,x} + .00718481481807253$$

> p3:=plot(1e6*e2(x/100),x=0..10,title='Dilatational Strain, e2(x) [ue]'):

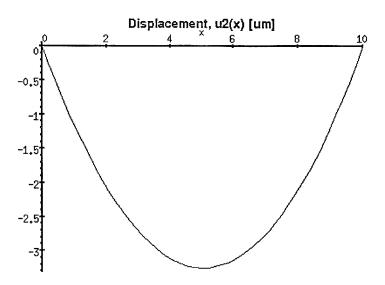
> p3;



> u2(x);

.00228419304518136
$$x$$
 + .000786845261334466 $\sqrt{1 - 2.04358350874846}$ x - .0016079839999998 $\sqrt{1 - 2.04358350874846}$ x - .000786845261334466

> p4:=plot(1e6*u2(x/100),x=0..10,title='Displacement, u2(x) [um]'): > p4; >

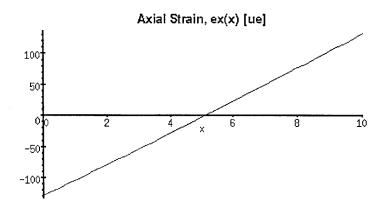


> ex:=unapply(diff(u2(x),x),x);

$$ex := x \to .00228419304518136 - .000803991999999993 \frac{1}{\sqrt{1 - 2.04358350874846 \, x}}$$

$$+ .00164302479236567 \frac{x}{\sqrt{1 - 2.04358350874846 \, x}}$$

 $-.0016079839999998 \int 1 - 2.04358350874846 x$ > p5:=plot(1e6*ex(x/100),x=0..10,title='Axial Strain, ex(x) [ue]'): > p5; >



-.255565909637320 108

> J2(x);

-.945993735697361 10⁻¹⁵

> evalf(u2(x),5);

 $.0022826\,x + .00078643\,\sqrt{1. - 2.0436\,x} - .0016069\,\sqrt{1. - 2.0436\,x}\,x - .00078689$

> evalf(c2(x),5);

.92584 10⁻⁶ - .92584 10⁻⁶ $\sqrt{1. - 2.0436 x}$

> evalf(e2(x),5);

 $-.0072359\sqrt{1.-2.0436x} + .0071848$

> evalf(ex(x),5);

 $.0022842 - .00080399 \frac{1}{\sqrt{1. - 2.0436 \, x}} + .0016430 \frac{x}{\sqrt{1. - 2.0436 \, x}} - .0016080 \sqrt{1. - 2.0436 \, x}$

1. MAPLE PROGRAM DIRECTORY AND FILE NAME: /afs/nd.edu/user4/jthomas5/Maple/Transport/1DSSsol#2d.ms

2. ORIGINATOR:

Dr. James P. Thomas University of Notre Dame Department of Aerospace and Mechanical Engineering 374 Fitzpatrick Hall Notre Dame, IN 46556-5637 (219) 631-9371

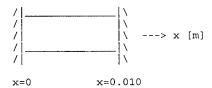
3. HISTORY:

Written: June-1994

Latest Revision: 13-Feb-1995

4. PROGRAM DESCRIPTION:

One-dimensional, steady-state analysis of the fully coupled solute transport and trapping equations using "plane stress" constitutive equations for stress. The particular problem solved is shown below:



Deformation variable boundary conditions: u(0)=0, u(0.10)=0 Concentration variable boundary conditions: J(0)=0, c(0.10)=1e-7 Reference concentration level: c0=0.0 Trapping parameter: traps=19 (low trapping)

The equations used in the analysis are:

- 1. dJ/dx=0 (steady-state diffusion equation)
- 2. J=Ds*(K1*c*de/dx-dc/dx) (mass flux)
- 3. dS/dx=0 (steady-state deformation equation with zero body forces)
- 4. S=B1*e-B2*c+B3 (axial stress)
- 5. de/dx-K2*dc/dx=0 (dilational strain gradient)
- 6. e=A1*du/dx+A2*c-A3 (dilatational strain)

The constants used in the plane stress analysis are defined below:

Ds:=lattice diffusivity [m^2/sec]

```
K1 := (Vs*K) / (R*T)
Vs:= partial molar volume of solute [m^3/mol solute]
K:= bulk modulus=Em/(3*(1-2*nu)) [N/m<sup>2</sup>]
Em:= Modulus of Elasticity [N/m^2]
nu:= Poisson's ratio [1]
R:= universal gas constant=8.31432 [J/mol-K]
T:= temperature [K]
B1:=3*K
B2:=9*K*alpha*(1+traps)
B3:=B2*co
traps:= Csr*Kr=trapping constant [1]
Csr:= saturation trap concentration [1]
Kr:= equilibrium trapping constant [1]
c0:= reference solute concentration [1]
K2:=3*alpha*(1+traps)
alpha:= (rho*Vs)/(3*MWs)=solute concentration expansion coefficient [m/m/Delc]
rho:= mass density of the solid [kg/m^3]
MWs:= molecular weight of the solute [kg/mol]
Delc:=c-c0 [1]
```

A1:=1-2*nu

A2:=2*(1+nu)*alpha*(1+traps)

A3:=A2*c0

5. REQUIRED INPUTS:

Ds:=lattice diffusivity [m^2/sec]

Vs:= partial molar volume of solute [m^3/mol solute]

traps:= Csr*Kr=trapping constant [1]

c0:= reference solute concentration [1]

MWs:= molecular weight of the solute [kg/mol]

Em: = Modulus of Elasticity [Pa]

nu:= Poisson's ratio [1]

rho:= mass density of the solid [kg/m^3]

T:= temperature [K]

6. SYMBOLIC ANALYSIS:

> Digits:=trunc(evalhf(Digits));

$$Digits := 15$$

 $\overline{> J:=Ds^*(K1^*K2^*c(x)-1)^*diff(c(x),x);}$

$$J := Ds (KI \ K2 \ c(x) - 1) \left(\frac{\partial}{\partial x} c(x) \right)$$

> deqn:=J=0;

$$deqn := Ds (K1 \ K2 \ c(x) - 1) \left(\frac{\partial}{\partial x} c(x) \right) = 0$$

> csoln:=dsolve(deqn,c(x),explicit);

$$csoln := c(x) = \frac{1}{K1 \ K2}, c(x) = _C1$$

Note #1: There are apparently two "roots" to the solution for c(x). The first solution is obviously incorrect in that the concentration throughout the rod is defined in terms of the constants K1 and K2. The second solution is adopted as correct.

> c2:=unapply(simplify(op(2,csoln[2])),x);

$$c2 := x \rightarrow C1$$

> c2(x);

 $_{C}1$

> J2:=unapply(simplify(Ds*(K1*K2*c2(x)-1)*diff(c2(x),x)));

J2 := 0

> J2(x);

0

Note #2: The above result shows that the mass flux is identically zero, as it should be.

Note #3: Since the concentration is constant, we have from the third governing equation that e(x) equals a constant.

> e2:=x->_C2;

$$e2 := x \rightarrow C2$$

> e2(x);

_C2

> ux2:=dsolve(A1*diff(u(x),x)+A2*c2(x)-A3-e2(x)=0,u(x));

$$ux2 := u(x) = \frac{-x A2 _C1 + x A3 + x _C2 + _C3 A1}{A1}$$

> u2:=unapply(simplify(op(2,ux2)),x);

$$u2 := x \rightarrow -\frac{x A2 _C1 - x A3 - x _C2 - _C3 A1}{A1}$$

> u2(x);

> S2:=x->simplify(B1*e2(x)-B2*c2(x)+B3);

$$S2 := x \rightarrow \text{simplify}(B1 \text{ e2}(x) - B2 \text{ c2}(x) + B3)$$

> S2(x);

$$B1 C2 - B2 C1 + B3$$

Note #4: At this point, we will introduce expressions for the constants and then simplify the results before quantifying the constants and determining the constants of integration.

> K:=(Em/(3*(1-2*nu)));

$$K := \frac{Em}{3-6v}$$

> lambda:=(Em*nu)/((1+nu)*(1-2*nu));

$$\lambda := \frac{Em \, v}{(1+v)(1-2 \, v)}$$

> mu:=Em/(2*(1+nu));

$$\mu := \frac{Em}{2 + 2v}$$

> alpha:=(rho*Vs)/(3*MWs);

$$\alpha := \frac{1}{3} \frac{\rho \ Vs}{MWs}$$

> K1:=(Vs*K)/(R*T);

$$KI := \frac{Vs Em}{(3 - 6 v) R T}$$

> K2:=3*alpha*(1+traps);

$$K2 := \frac{\rho \ Vs \ (1 + traps)}{MWs}$$

> K1*K2;

$$\frac{Vs^2 Em \rho (1 + traps)}{(3 - 6 v) R T MWs}$$

> A1:=(1-2*nu);

$$A1 := 1 - 2 v$$

> A2:=2*(1+nu)*alpha*(1+traps);

$$A2 := \frac{2}{3} \frac{(1+v) \rho Vs (1+traps)}{MWs}$$

> A3:=A2*c0;

$$A3 := \frac{2}{3} \frac{(1+v) \rho Vs (1+traps) c\theta}{MWs}$$

> B1:=3*K:

$$BI := 3 \frac{Em}{3 - 6 v}$$

> B2:=9*K*alpha*(1+traps);

$$B2 := 3 \frac{Em \rho Vs (1 + traps)}{(3 - 6 \vee) MWs}$$

> B3:=B2*c0:

$$B3 := 3 \frac{Em \rho Vs (1 + traps) cO}{(3 - 6 v) MWs}$$

> c2(x);

 $_{CI}$

> e2(x);

_C2

> collect(u2(x),x);

$$-\frac{\left(\frac{2}{3}\frac{(1+v)\rho Vs (1+traps)_C1}{MWs} - \frac{2}{3}\frac{(1+v)\rho Vs (1+traps)c0}{MWs} - _C2\right)x}{1-2v} + _C3$$

> S2(x);

$$\frac{Em\left(-_C2\ MWs + \rho\ Vs\ _C1 + \rho\ Vs\ _C1\ traps - \rho\ Vs\ c0 - \rho\ Vs\ c0\ traps\right)}{\left(-1 + 2\ v\right)\ MWs}$$

Note #5: Now we will introduce numerical values for the material properties.

The values selected are typical for internal hydrogen as a solute in AISI 4340 steel.

> Ds:=1e-9;		
	$Ds := .1 \ 10^{-8}$	
> Vs:=2.02e-6;		
, 102.020 0,	$Vs := .202 \ 10^{-5}$	
	VS .= .202 10	
> traps:=19;		
	traps := 19	
> c0:=0.0;		
	$c\theta := 0$	
> MWs:=0.00100797;	,	
,,	MWs := .00100797	
	141 1413 . — .00100797	
> Em:=200e9;	10	
	$Em := .200 \ 10^{12}$	
> nu:=0.3;		
	v := .3	
> rho:=7800;		
,,	$\rho := 7800$	
T 000	μ.= 7000	
> T:=293;		
	T := 293	
> R:=8.31432;		
	R := 8.31432	
> K;		
•	.166666666666667 1012	
Laurelinda	.1000000000007 10	
> lambda;	117001617001616101012	
	$.115384615384616\ 10^{12}$	
> mu;		
	.769230769230769 1011	
> alpha;		
	5.21047253390476	
> Rh:=R/MWs;	3.210 (72333) 0 17 0	
> nii.=n/wws,	DI 0040 57000 (75000	
	Rh := 8248.57882675080	
> K1;		
	138.199274509089	
> K2;		
	312.628352034286	
> K1*K2;	0.12.02002-00.1200	
C 131 136y	42205 0114421104	
	43205.0114421104	

```
> 1/(K1*K2);
                                .0000231454631447068
> A1;
                                          .4
> A2;
                                   270.944571763048
> A3;
                                          0
> B1;
                                .500000000000001\ 10^{12}
> B2;
                                .156314176017143\ 10^{15}
> B3;
                                          0
 Note #6: Now let's solve for the integration constants: _C1, _C2, _C3, and _C4,
         using the boundary data.
> c2(x);
                                         C1
> _C1:=1e-7;
                                     C1 := .1 \ 10^{-6}
> c2(x);
                                        .1\ 10^{-6}
> u2(x);
      -.0000677361429407620 \ x + 2.500000000000000 \ x \ \_C2 + 1.00000000000000 \ \_C3
> eqnset1:=\{0.0=u2(0.0), 0.0=u2(0.10)\};
    > varset1:={_C2,_C3};
                                varset1 := \{ \_C2, \_C3 \}
> solnset1:=solve(eqnset1,varset1);
                  solnset1 := \{ C3 = 0, C2 = .0000270944571763048 \}
> assign(solnset1);
```

> c2(x);		
	.1 10-6	
> e2(x);		
	.0000270944571763048	
> u2(x);		
	0	
> S2(x);		
	208418901356190 10 ⁷	

1. MAPLE PROGRAM DIRECTORY AND FILE NAME: /afs/nd.edu/user4/jthomas5/Maple/Transport/1DSSsol#2e.ms

2. ORIGINATOR:

Dr. James P. Thomas University of Notre Dame Department of Aerospace and Mechanical Engineering 374 Fitzpatrick Hall Notre Dame, IN 46556-5637 (219) 631-9371

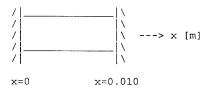
3. HISTORY:

Written: June-1994

Latest Revision: 13-Feb-1995

4. PROGRAM DESCRIPTION:

One-dimensional, steady-state analysis of the fully coupled solute transport and trapping equations using "plane stress" constitutive equations for stress. The particular problem solved is shown below:



Deformation variable boundary conditions: u(0)=0, u(0.10)=0Concentration variable boundary conditions: J(0)=0, c(0.10)=1e-7Reference concentration level: c0=0.0Trapping parameter: traps=499 (high trapping)

The equations used in the analysis are:

- 1. dJ/dx=0 (steady-state diffusion equation)
- 2. J=Ds*(K1*c*de/dx-dc/dx) (mass flux)
- 3. dS/dx=0 (steady-state deformation equation with zero body forces)
- 4. S=B1*e-B2*c+B3 (axial stress)
- 5. de/dx-K2*dc/dx=0 (dilational strain gradient)
- 6. e=A1*du/dx+A2*c-A3 (dilatational strain)

MWs:= molecular weight of the solute [kg/mol]

Delc:= c-c0 [1]

The constants used in the plane stress analysis are defined below:

Ds:=lattice diffusivity [m^2/sec]

K1 := (Vs * K) / (R * T)Vs:= partial molar volume of solute [m^3/mol solute] $K:= bulk modulus=Em/(3*(1-2*nu)) [N/m^2]$ Em:= Modulus of Elasticity [N/m^2] nu:= Poisson's ratio [1] R:= universal gas constant=8.31432 [J/mol-K] T:= temperature [K] B1:=3*K B2:=9*K*alpha*(1+traps) B3:=B2*co traps:= Csr*Kr=trapping constant [1] Csr:= saturation trap concentration [1] Kr:= equilibrium trapping constant [1] c0:= reference solute concentration [1] K2:=3*alpha*(1+traps) alpha:= (rho*Vs)/(3*MWs)=solute concentration expansion coefficient [m/m/Delc] rho:= mass density of the solid [kg/m^3]

A1:=1-2*nu A2:=2*(1+nu)*alpha*(1+traps)A3:=A2*c0

5. REQUIRED INPUTS:

Ds:=lattice diffusivity [m^2/sec] Vs:= partial molar volume of solute [m^3/mol solute] traps:= Csr*Kr=trapping constant [1] c0:= reference solute concentration [1] MWs:= molecular weight of the solute [kg/mol]

Em: = Modulus of Elasticity [Pa]

nu:= Poisson's ratio [1] rho:= mass density of the solid [kg/m^3]

T:= temperature [K]

6. SYMBOLIC ANALYSIS:

> Digits:=trunc(evalhf(Digits));

$$Digits := 15$$

 $> J:=Ds^*(K1^*K2^*c(x)-1)^*diff(c(x),x);$

$$J := Ds (KI \ K2 \ c(x) - 1) \left(\frac{\partial}{\partial x} c(x) \right)$$

> deqn:=J=0;

$$deqn := Ds (K1 \ K2 \ c(x) - 1) \left(\frac{\partial}{\partial x} c(x) \right) = 0$$

> csoln:=dsolve(deqn,c(x),explicit);

$$csoln := c(x) = \frac{1}{K1 \ K2}, c(x) = _C1$$

Note #1: There are apparently two "roots" to the solution for c(x). The first solution is obviously incorrect in that the concentration throughout the rod is defined in terms of the constants K1 and K2. The second solution is adopted as correct.

> c2:=unapply(simplify(op(2,csoln[2])),x);

$$c2 := x \rightarrow C1$$

> c2(x);

_C1

> J2:=unapply(simplify(Ds*(K1*K2*c2(x)-1)*diff(c2(x),x)));

J2 := 0

> J2(x);

0

Note #2: The above result shows that the mass flux is identically zero, as it should be.

Note #3: Since the concentration is constant, we have from the third governing equation that e(x) equals a constant.

> e2:=x->_C2;

$$e2 := x \rightarrow _C2$$

> e2(x);

$$_{C2}$$

> ux2:=dsolve(A1*diff(u(x),x)+A2*c2(x)-A3-e2(x)=0,u(x));

$$ux2 := u(x) = \frac{-x A2 _C1 + x A3 + x _C2 + _C3 A1}{A1}$$

> u2:=unapply(simplify(op(2,ux2)),x);

$$u2 := x \to \frac{-x A2 _C1 + x A3 + x _C2 + _C3 A1}{A1}$$

> u2(x);

$$\frac{-x A2 \ _C1 + x A3 + x \ _C2 + \ _C3 \ A1}{A1}$$

> S2:=x->simplify(B1*e2(x)-B2*c2(x)+B3);

$$S2 := x \rightarrow \text{simplify}(B1 \text{ e2}(x) - B2 \text{ c2}(x) + B3)$$

> S2(x);

Note #4: At this point, we will introduce expressions for the constants and then simplify the results before quantifying the constants and determining the constants of integration.

> K:=(Em/(3*(1-2*nu)));

$$K := \frac{Em}{3-6v}$$

> lambda:=(Em*nu)/((1+nu)*(1-2*nu));

$$\lambda := \frac{Em \, V}{(1+V)(1-2 \, V)}$$

> mu:=Em/(2*(1+nu));

$$\mu := \frac{Em}{2+2v}$$

> alpha:=(rho*Vs)/(3*MWs);

$$\alpha := \frac{1}{3} \frac{\rho \ Vs}{MWs}$$

> K1:=(Vs*K)/(R*T);

$$K1 := \frac{Vs Em}{(3-6 \text{ V}) R T}$$

> K2:=3*alpha*(1+traps);

$$K2 := \frac{\rho \ Vs (1 + traps)}{MWs}$$

> K1*K2;

$$\frac{Vs^2 Em \rho (1 + traps)}{(3 - 6 v) R T MWs}$$

> A1:≃(1-2*nu);

$$A1 := 1 - 2 \nu$$

> A2:=2*(1+nu)*alpha*(1+traps);

$$A2 := \frac{2}{3} \frac{(1+v) \rho Vs (1+traps)}{MWs}$$

> A3:=A2*c0;

$$A3 := \frac{2}{3} \frac{(1+v) \rho Vs (1+traps) cO}{MWs}$$

> B1:=3*K;

$$B1 := 3 \frac{Em}{3 - 6 \text{ y}}$$

> B2:=9*K*alpha*(1+traps);

$$B2 := 3 \frac{Em \rho Vs (1 + traps)}{(3 - 6 \vee) MWs}$$

> B3:=B2*c0;

$$B3 := 3 \frac{Em \rho Vs (1 + traps) c0}{(3 - 6 v) MWs}$$

> c2(x);

__C1

> e2(x);

 $_C2$

> collect(u2(x),x);

$$\frac{\left(-\frac{2}{3}\frac{(1+v)\rho Vs (1+traps)_{-}C1}{MWs} + \frac{2}{3}\frac{(1+v)\rho Vs (1+traps)_{-}c0}{MWs} + _{-}C2\right)x}{1-2v} + _{-}C3$$

> S2(x);

$$-\frac{Em\left(_C2\ MWs - \rho\ Vs\ _C1 - \rho\ Vs\ _C1\ traps + \rho\ Vs\ c0 + \rho\ Vs\ c0\ traps\right)}{\left(-1 + 2\ v\right)MWs}$$

Note #5: Now we will introduce numerical values for the material properties.

The values selected are typical for internal hydrogen as a solute in AISI 4340 steel.

> Ds:=1e-9;	
· = - 7	n 1108
	$Ds := .1 \ 10^{-8}$
> Vs:=2.02e-6;	
	$Vs := .202 \ 10^{-5}$
•	VS := .202 10°
> traps:=499;	
	traps := 499
	11 aps 499
> c0:=0.0;	
	$c\theta := 0$
> MWs:=0.00100797;	
	MWs := .00100797
> Em:=200e9;	
	$Em := .200 \ 10^{12}$
> nu:=0.3;	
, iidi	_
	v := .3
> rho:=7800;	
,	7000
	$\rho := 7800$
> T:=293;	
	T := 293
	1 293
> R:=8.31432;	
	R := 8.31432
	1 0.31132
> K;	
	.166666666666667 10 ¹²
	.100000000000007 10
> lambda;	
	.115384615384616 10 ¹²
> mu;	
	.769230769230769 10 ¹¹
> alpha;	
• ,	£ 010.450£0000.456
	5.21047253390476
> Rh:=R/MWs;	
7	01. 0240 57002675000
Γ	Rh := 8248.57882675080
> K1;	
	138.199274509089
	130.177274307007
> K2;	
	7815.70880085715
1/4+1/0	, 5 2 5 . 7 0 0 0 0 0 0 7 2 0
> K1*K2;	
	$.108012528605276\ 10^{7}$

```
> 1/(K1*K2);
                            .925818525788271\ 10^{-6}
> A1;
                                     .4
> A2;
                              6773.61429407620
> A3;
                                     0
> B1;
                            .500000000000001\ 10^{12}
> B2;
                            .390785440042860\ 10^{16}
> B3;
                                     0
 Note #6: Now let's solve for the integration constants: _C1, _C2, _C3, and _C4,
        using the boundary data.
> c2(x);
                                    \_CI
>_C1:=1e-7;
                                C1 := .1 \ 10^{-6}
> c2(x);
                                   .1 10-6
> u2(x);
      > eqnset1:={0.0=u2(0.0), 0.0=u2(0.10)};
    > varset1:={_C2,_C3};
                            varset1 := \{ \_C3, \_C2 \}
> solnset1:=solve(eqnset1,varset1);
                solnset1 := \{ C3 = 0, C2 = .000677361429407620 \}
> assign(solnset1);
```

> c2(x); $.1 10^{-6}$ > e2(x); .000677361429407620 > u2(x); 0 > s2(x); $-.521047253390490 10^{8}$

1. MAPLE PROGRAM DIRECTORY AND FILE NAME:
/afs/nd.edu/user4/jthomas5/Maple/Transport/1DSSsol#3a.ms

2. ORIGINATOR:

Dr. James P. Thomas University of Notre Dame Department of Aerospace and Mechanical Engineering 374 Fitzpatrick Hall Notre Dame, IN 46556-5637 (219) 631-9371

3. HISTORY:

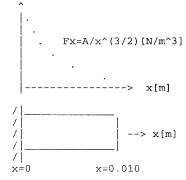
Written: 12-Nov-1994

Latest Revision: 13-Feb-1995

4. PROGRAM DESCRIPTION:

One-dimensional, steady-state analysis of the fully coupled solute transport and trapping equations using "plane stress" constitutive equations for stress. The particular problem to be solved is shown below. It is the 1-D SS transport problem, but with a $(1/x)^{(3/2)}$ singular body force applied. This is meant to "simulate" the stress occurring at the tip of a crack:

Body Force, Fx



Deformation variable boundary conditions: u(0)=0, S(0.10)=0 Concentration variable boundary conditions: J(0)=0, c(0.10)=1e-7 Reference concentration level: c0=0.0 Trapping parameter: ltrap=19 (low trapping); htrap=499 (high trapping) Body Force: $A=15\times10^6$ [N/m^(3/2)]

The equations used in the analysis are:

- 1. dJ/dx=0 (steady-state diffusion equation)
- 2. J=Ds*(K1*c*de/dx-dc/dx) (mass flux)
- 3. dS/dx+Fx=0 (steady-state deformation equation with applied axial body force, Fx)
- 4. S=B1*e-B2*c+B3 (axial stress)
- 5. de/dx-K2*dc/dx+Fx/B1=0 (dilational strain gradient)
- 6. e=A1*du/dx+A2*c-A3 (dilatational strain)

The constants used in the plane stress analysis are defined below:

Ds:=lattice diffusivity [m^2/sec]

K1 := (Vs*K) / (R*T)

Vs:= partial molar volume of solute [m^3/mol solute]

 $K:= bulk modulus=Em/(3*(1-2*nu)) [N/m^2]$

Em:= Modulus of Elasticity [N/m^2]

nu:= Poisson's ratio [1]

R:= universal gas constant=8.31432 [J/mol-K]

T:= temperature [K]

B1:=3*K B2:=9*K*alpha*(1+traps) B3:=B2*co (l or h)traps:= Csr*Kr=trapping constant [1] Csr:= saturation trap concentration [1] Kr:= equilibrium trapping constant [1] c0:= reference solute concentration [1] K2:=3*alpha*(1+traps) alpha:= (rho*Vs)/(3*MWs)=solute concentration expansion coefficient [m/m/Delc] rho:= mass density of the solid [kg/m^3] MWs:= molecular weight of the solute [kg/mol] Delc:= c-c0 [1] A1:=1-2*nu A2:=2*(1+nu)*alpha*(1+traps)A3:=A2*c0 5. REQUIRED INPUTS: Ds:=lattice diffusivity [m^2/sec] Vs:= partial molar volume of solute [m^3/mol solute] (1 and h)traps:= Csr*Kr=trapping constant [1] c0:= reference solute concentration [1] MWs:= molecular weight of the solute [kg/mol] Em: = Modulus of Elasticity [Pa] nu:= Poisson's ratio [1] rho:= mass density of the solid [kg/m^3] T:= temperature [K] ______ 6. SYMBOLIC ANALYSIS:

> restart;

> Digits:=trunc(evalhf(Digits));

$$Digits := 15$$

> s:=x->B1*e(x)-B2*c(x)+B3;

$$s := x \rightarrow BI \ e(x) - B2 \ c(x) + B3$$

> diff(s(x),x)+fx(x);

$$BI\left(\frac{\partial}{\partial x}e(x)\right) - B2\left(\frac{\partial}{\partial x}c(x)\right) + fx(x)$$

> dedx:=x->K2*diff(c(x),x)-fx(x)/B1;

$$dedx := x \rightarrow K2 \operatorname{diff}(c(x), x) - \frac{\operatorname{fx}(x)}{B1}$$

> j:=Ds*(K1*c(x)*dedx(x)-diff(c(x),x));

$$j := Ds\left(K1 \ c(x) \left(K2 \left(\frac{\partial}{\partial x} c(x)\right) - \frac{fx(x)}{B1}\right) - \left(\frac{\partial}{\partial x} c(x)\right)\right)$$

 $> J := unapply(Ds^*((K1^*K2^*c(x)-1)^*diff(c(x),x)-(K1/B1)^*c(x)^*F(x)),x);\\$

$$J := x \to Ds \left((Kl \ K2 \ c(x) - 1) \ diff(c(x), x) - \frac{Kl \ c(x) \ F(x)}{Bl} \right)$$

> J(x);

$$Ds\left((KI\ K2\ c(x)-1)\left(\frac{\partial}{\partial x}c(x)\right)-\frac{KI\ c(x)\ F(x)}{BI}\right)$$

 $> F:=x->A/x^{(3/2)};$

$$F := x \to \frac{A}{x^{3/2}}$$

Note #1: The governing differential equation in this case is J=constant because dJ/dx=0, and we are specifying the value of J(0)=0. Setting J(0)=constant produces a DE that is not easily solved.

> deqn1:=collect(expand(J(x)/(Ds*K1*K2)),diff(c(x),x))=0;

$$deqn1 := \left(c(x) - \frac{1}{K1 \ K2}\right) \left(\frac{\partial}{\partial x} c(x)\right) - \frac{c(x) A}{K2 \ B1 \ x^{3/2}} = 0$$

> soln1:=dsolve(deqn1,c(x));

$$soln1 := c(x) - \frac{\ln(c(x))}{K1 \ K2} + 2 \frac{A}{\sqrt{x} \ K2 \ B1} = _C1$$

Note #2: We will solve the above equation for c(x) and use this to define a new function with $\{x, A, K2, and _C1\}$ as the argument variables.

$> c:=(x,A,K2,C1)-> solve(c-(1/(K1*K2))*In(c)+(2*A)/(K2*B1*x^(1/2)) = C1,c);$

$$c := (x, A, K2, CI) \rightarrow \text{solve} \left(c - \frac{\ln(c)}{KI \ K2} + 2 \frac{A}{K2 \ BI \ \sqrt{x}} = CI, c \right)$$

> c(x,A,K2,C1);

$$\frac{W\left(-K1\ K2\ e^{\left(\frac{K1\left(2\sqrt{x}\ A-C1\ K2\ B1\ x\right)}{x\ B1}\right)}\right)}{K1\ K2}$$

Note #3a: The above W() function is the "Lambert" function W(). See the Maple help discussion of this function for further details.

Note #3b: The expression for the classical stress assisted diffusion (SAD) concentration is given by setting taking the limit of c(x) as K2->0.

\rightarrow csad:=(x,A,C1)->limit(c(x,A,K2,C1),K2=0);

$$csad := (x, A, C1) \rightarrow \lim_{K2 \rightarrow 0} c(x, A, K2, C1)$$

> csad(x,A,C1);

$$e^{\left(2\frac{KIA}{\sqrt{x}BI}\right)}$$

> dcdx1:=simplify(diff(c(x,A,K2,C1),x));

$$dcdx1 := \frac{W\left(-K1 \ K2 \ e^{\left(\frac{K1\left(2\sqrt{x} \ A - C1 \ K2 \ B1 \ x\right)}{x \ B1}\right)\right)}A}{\left(1 + W\left(-K1 \ K2 \ e^{\left(\frac{K1\left(2\sqrt{x} \ A - C1 \ K2 \ B1 \ x\right)}{x \ B1}\right)\right)}\right)}x^{3/2} \ B1 \ K2$$

> diff(W(f(x)),x);

$$\frac{W(f(x))\left(\frac{\partial}{\partial x}f(x)\right)}{(1+W(f(x)))f(x)}$$

> int(W(x),x);

$$W(x) x - x + \frac{x}{W(x)}$$

 $> x:=(c,A,K2,C1)-> solve(c-(1/(K1*K2))*ln(c)+(2*A)/(K2*B1*x^(1/2)) = C1,x);$

$$x := (c, A, K2, C1) \rightarrow \text{solve} \left(c - \frac{\ln(c)}{K1 \ K2} + 2 \frac{A}{K2 \ B1 \ \sqrt{x}} = C1, x \right)$$

> x(c,A,K2,C1);

$$4 \frac{A^2 K I^2}{(c K1 K2 B1 - \ln(c) B1 - C1 K1 K2 B1)^2}$$

> dxdc:=(c,A,K2,C1)->diff(x(c,A,K2,C1),c);

$$dxdc := (c, A, K2, C1) \rightarrow diff(x(c, A, K2, C1), c)$$

> factor(dxdc(c,A,K2,C1));

$$8 \frac{A^2 K I^2 (c K I K 2 - 1)}{B I^2 (-c K I K 2 + \ln(c) + C I K I K 2)^3 c}$$

> factor(1/dxdc(c,A,K2,C1));

$$\frac{1}{8} \frac{BI^{2} \left(-c \ KI \ K2 + \ln(c) + CI \ KI \ K2\right)^{3} c}{A^{2} \ KI^{2} \left(c \ KI \ K2 - 1\right)}$$

Note #4a: dx/dc->0 as c->1/K1*K2, or conversely, dc/dx->infinity as c->1/K1*K2! This is apparently a critical point in the mathematical solution for the concentration. The concentration becomes multivalued at this point.

Note #4b: We will now determine the value of the constant $_{C1}$ in terms of the concentration at the boundary c(0.1)=c1.

> C1:=solve(x(cl,A,K2,C1)=0.1,C1);

C1 := -.5000000000000000

$$\frac{-2. BI^2 cl KI^2 K2^2 + 2. BI^2 \ln(cl) KI K2 + 12.6491106406735 BI KI^2 K2 A}{BI^2 KI^2 K2^2}, -\frac{1}{2}$$

.5000000000000000

$$\frac{-2.\ BI^2\ cl\ KI^2\ K2^2 + 2.\ BI^2\ ln(cl)\ KI\ K2 - 12.6491106406735\ BI\ KI^2\ K2\ A}{BI^2\ KI^2\ K2^2}$$

> C1a:=(cl,A,K2)->expand(-.5000000000000000B1^2/K1^2/K2^2*(-2.*B1^2*cl*K1^2*K2^2+2.*B1^2*ln(cl)*K1*K2+12.6491106 406735*B1*K1^2*K2*A));

$$\frac{-2. Bl^{2} cl Kl^{2} K2^{2} + 2. Bl^{2} \ln(cl) Kl K2 + 12.6491106406735 Bl Kl^{2} K2 A}{Bl^{2} Kl^{2} K2^{2}}$$

> C1b:=(cl,A,K2)->expand(-.5000000000000000B1^2/K1^2/K2^2*(-2.*B1^2*cl*K1^2*K2^2+2.*B1^2*In(cl)*K1*K2-12.6491106 406735*B1*K1^2*K2*A));

> C1a(cl,A,K2);

> C1b(cl,A,K2);

Note #5: The quadratic nature of the solution for the constant _C1 yields two solutions, C1a & C1b, each of which results in a different solution for x(c).

> xa:=(c,A,K2,cl)->x(c,A,K2,C1a(cl,A,K2));

$$xa := (c, A, K2, cl) \rightarrow x(c, A, K2, C1a(cl, A, K2))$$

> xa(c,A,K2,cl);

> xb:=(c,A,K2,cl)->x(c,A,K2,C1b(cl,A,K2));

$$xb := (c, A, K2, cl) \rightarrow x(c, A, K2, C1b(cl, A, K2))$$

> xb(c,A,K2,cl);

Note #6: Next we will define the constants, and simplify the two concentration expressions for the fully coupled theory and classical stress assisted diffusion theory.

> K:=(Em/(3*(1-2*nu)));

$$K := \frac{Em}{3 - 6 \text{ V}}$$

> lambda:=(Em*nu)/((1+nu)*(1-2*nu));

$$\lambda := \frac{Em \, V}{(1+V)(1-2 \, V)}$$

> mu:=Em/(2*(1+nu));

$$\mu := \frac{Em}{2 + 2v}$$

> alpha:=(rho*Vs)/(3*MWs);

$$\alpha := \frac{1}{3} \frac{\rho \ Vs}{MWs}$$

> K1:=(Vs*K)/(R*T);

$$KI := \frac{Vs Em}{(3 - 6 v) R T}$$

> K2:=3*alpha*(1+traps);

$$K2 := \frac{\rho \ Vs (1 + traps)}{MWs}$$

> A1:=(1-2*nu);

$$A1 := 1 - 2 v$$

> A2:=2*(1+nu)*alpha*(1+traps);

$$A2 := \frac{2}{3} \frac{(1+v) \rho Vs (1+traps)}{MWs}$$

> A3:=A2*c0;

$$A3 := \frac{2}{3} \frac{(1+v) \rho Vs (1 + traps) cO}{MWs}$$

> B1:=3*K;

$$B1 := 3 \frac{Em}{3 - 6 v}$$

> B2:=9*K*alpha*(1+traps);

$$B2 := 3 \frac{Em \rho Vs (1 + traps)}{(3 - 6 \vee) MWs}$$

> B3:=B2*c0;

$$B3 := 3 \frac{Em \rho Vs (1 + traps) cO}{(3 - 6 v) MWs}$$

> Ds:=1e-9;

$$Ds := .1 \ 10^{-8}$$

> Vs:=2.02e-6;

$$Vs := .202 \cdot 10^{-5}$$

> ltraps:=19;

$$ltraps := 19$$

> htraps:=499;

$$htraps := 499$$

> c0:=0; c0 := 0> MWs:=0.00100797; MWs := .00100797> Em:=200e9; $Em := .200 \ 10^{12}$ > nu:=0.3; v := .3> rho:=7800; $\rho := 7800$ > T:=293; T := 293> R:=8.31432; R := 8.31432> K; $.1666666666666667 \ 10^{12}$ > lambda; $.115384615384616\ 10^{12}$ > mu; $.769230769230769\ 10^{11}$ > alpha; 5.21047253390476 > Rs:=R/MWs; Rs := 8248.57882675080> K1; 138.199274509089 > IK2:=3*alpha*(1+ltraps); *lK2* := 312.628352034285 > hK2:=3*alpha*(1+htraps); hK2 := 7815.70880085714> K1*lK2; 43205.0114421103 > 1/(K1*IK2); .0000231454631447068 > K1*hK2; $.108012528605276\ 10^{7}$ > 1/(K1*hK2); $.925818525788272\ 10^{-6}$

> A1;

.4

```
> A2;
                           13.5472285881524 + 13.5472285881524 traps
> A3;
                                                   0
> B1:
                                       .500000000000001 \ 10^{12}
> B2;
                      .781570880085717\ 10^{13} + .781570880085717\ 10^{13}\ traps
> B3;
                                                   0
  Note #7: Concentration for the low trapping situation (i.e., traps=19):
> traps:=19;
                                              traps := 19
> IA2:=A2;
                                      1A2 := 270.944571763048
> IB2:=B2;
                                   lB2 := .156314176017143 \ 10^{15}
> xal:=(c,A,K2,cl)->xa(c,A,K2,cl);
                                               xal := xa
> xal(c,A,K2,cl);
      .163705496009499 10^{-27} A^2/(c - .0000231454631447068 \ln(c) - 1.000000000000001 cl
             + .0000231454631447068 \ln(cl) + .404605358354904 \left.10^{-13} \, A\right)^2
> xb1:=(c,A,K2,c1)->xb(c,A,K2,c1);
                                               xbl := xb
> xbl(c,A,K2,cl);
      .163705496009499 10-27 A^2/(c - .0000231454631447068 \ln(c) - 1.00000000000001 cl
             +.0000231454631447068 \ln(cl) -.404605358354904 \ln^{-13}A)^2
  Note #8: Concentration for the high trapping situation (i.e., traps=499):
> traps:=499;
                                             traps := 499
> hA2:=A2;
                                     hA2 := 6773.61429407620
> hB2:=B2;
                                  hB2 := .390785440042860 \ 10^{16}
> xah:=(c,A,K2,cl)->xa(c,A,K2,cl);
                                              xah := xa
```

> xah(c,A,K2,cl);

 $.261928793615197\ 10^{-30}\ A^2\Big/(c-.925818525788271\ 10^{-6}\ \ln(c)-1.000000000000001\ cl\\ +.925818525788275\ 10^{-6}\ \ln(cl)+.161842143341962\ 10^{-14}\ A\Big)^2$

> xbh:=(c,A,K2,cl)->xb(c,A,K2,cl);

$$xbh := xb$$

> xbh(c,A,K2,cl);

 $.261928793615197 \ 10^{-30} A^2 / (c - .925818525788271 \ 10^{-6} \ln(c) - 1.000000000000001 \ cl \\ + .925818525788275 \ 10^{-6} \ln(cl) - .161842143341962 \ 10^{-14} A)^2$

Note #9: Concentration for SAD:

> csad(x,A,cl)->-1.*ln(cl)+.5516800588e-9*A/x^(1/2);

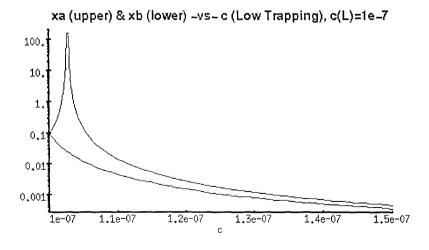
$$\operatorname{csad}(x, A, cl) \to -1. \ln(cl) + .5516800588 \ 10^{-9} \frac{A}{\sqrt{x}}$$

> Csad1(x,A,cl);

> with(plots);

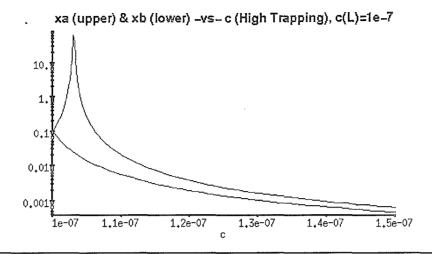
[animate, animate3d, conformal, contourplot, cylinderplot, densityplot, display, display3d, fieldplot, fieldplot3d, gradplot, gradplot3d, implicitplot, implicitplot3d, loglogplot, logplot, matrixplot, odeplot, pointplot, polarplot, polygonplot, polygonplot3d, polyhedraplot, replot, setoptions, setoptions3d, spacecurve, sparsematrixplot, sphereplot, surfdata, textplot, textplot3d, tubeplot]

```
> logpiot({xal(c,15e6,lK2,1e-7),
> xbl(c,15e6,lK2,1e-7)},c=1e-7..1.5e-7,
> title='xa (upper) & xb (lower) -vs- c (Low Trapping), c(L)=1e-7');
```



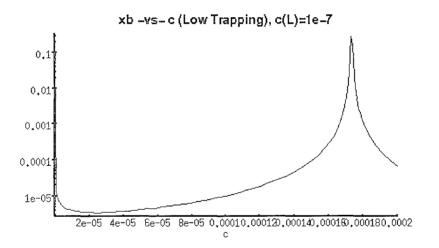
> logplot({xah(c,15e6,hK2,1e-7), > xbh(c,15e6,hK2,1e-7)},c=1e-7..1.5e-7,

> title='xa (upper) & xb (lower) -vs- c (High Trapping), c(L)=1e-7');

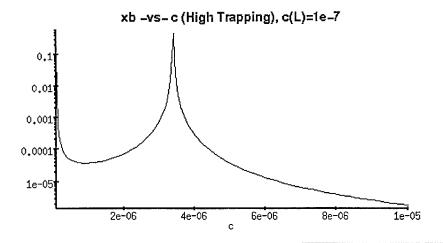


Note #11: The above plots show that the C1b parameter is clearly correct for c(L)=1e-7. The xa(c) solution predicts a singularity at the minimum concentration level on the rod, which is of no physical use. We must now determine, however, whether the xb(c) solution is valid for all c if c(L)=1e-7.

> logplot(xbl(c,15e6,lK2,1e-7), > c=1e-7..2e-4,title='xb -vs- c (Low Trapping), c(L)=1e-7');



> logplot(xbh(c,15e6,hK2,1e-7), > c=1e-7..1e-5,title='xb -vs- c (High Trapping), c(L)=1e-7');



Note #12a: THE ABOVE PLOTS DO NOT HAVE THE SAME CONCENTRATION AXES. While the second plot appears to show that the singularity shifts toward a higher concentration level, this is only caused by a change in the axis scaling -- the singularity shift is toward lower concentration levels.

Note #12b: Both xa(c) & xb(c) become multi-valued at some critical value of c which is dependent on the level of trapping present in the model, but independent of the left-hand concentration. We must determine the value of x where this critical value of concentration occurs decreases from L=0.1 to 0.0.

as x

> cbl_cr:=solve(diff(xbl(c,15e6,lK2,cl),c)=0,c);

 $cbl_cr := .0000231454631447068$

> xblmin:=xbl(cbl_cr,15e6,lK2,1e-9);

 $xblmin := .834723037961544 \cdot 10^{-6}$

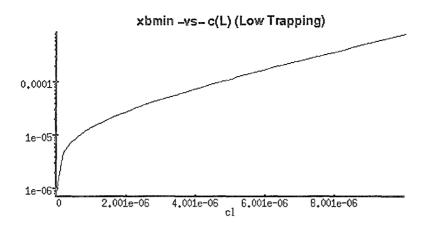
> xblmin:=xbl(cbl_cr,15e6,IK2,1e-7);

 $xblmin := .343354181482430 \ 10^{-5}$

Note #13: The limit concentration is independent of c(L), but the value of x(c) which corresponds to this limit concentration is obviously highly dependent on c(L). We now examine the nature of this dependence.

> logplot(xbl(cbl_cr,15e6,1K2,cl),cl=1e-9..1e-5,title='xbmin -vs- c(L) (Low Trapping)');

>



> cbh_cr:=solve(diff(xbh(c,15e6,hK2,cl),c)=0,c);

 $cbh_cr := .925818525788272 \ 10^{-6}$

> xbhmin:=xbh(cbh_cr,15e6,hK2,1e-9);

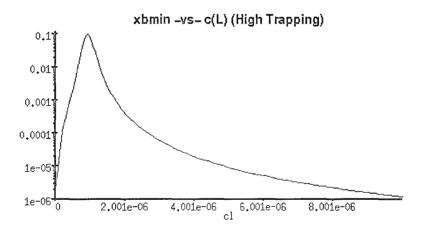
 $xbhmin := .200363349240182 \ 10^{-5}$

> xbhmin:=xbh(cbh_cr,15e6,hK2,1e-7);

xbhmin := .0000371878414183777

> logplot(xbh(cbh_cr,15e6,hK2,cl),cl=1e-9..1e-5,title='xbmin -vs- c(L) (High Trapping)');

>



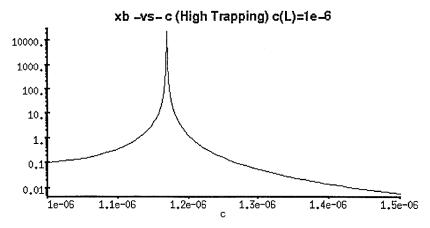
Note #14: The threshold level of c(L) at which xb(c) is no longer valid and xa(c) becomes the physically meaningful solution may also be determined.

> solve(diff(xbh(cbh_cr,15e6,hK2,cl),cl)=0,cl);

 $.925818525788271\ 10^{-6}$

> logplot(xbh(c,15e6,hK2,1e-6),c=1e-6..1.5e-6,title='xb -vs- c (High Trapping) c(L)=1e-6');

>



> logplot(xah(c,15e6,hK2,1e-6),c=1e-6..1.5e-6,title='xa -vs- c (High Trapping), c(L)=1e-6');

xa -vs- c (High Trapping), c(L)=1e-6

0.1

0.05

0.01

0.005

Note #15a: The solution is divided, then, into two regimes. The first, with c(L) < c(L) critical, requires the xb solution, while the second, with c(L) > c(L) critical, requires the xa solution. The exact value of c(L) critical is dependent on the level of trapping present in the problem.

1,2e-06

Note #15b: To directly compare the concentration predictions generated by variations in trapping and c(L), we define new concentration functions.

> cb:=(x,A,K2,cl)->c(x,A,K2,C1b(cl,A,K2));

1e-06

1.1e-06

$$cb := (x, A, K2, cl) \rightarrow c(x, A, K2, C1b(cl, A, K2))$$

> cb(x,A,IK2,cl);

 $-.0000231454631447068 \ \text{W} \big(-43205.0114421103 \ e^{\big(-.432050114421103 \ 10^{-23} \big)} \big) + 1.0000231454631447068 \ \text{W} \big(-43205.0114421103 \ e^{\big(-.432050114421103 \ 10^{-23} \big)} \big) + 1.0000231454631447068 \ \text{W} \big(-43205.0114421103 \ e^{\big(-.432050114421103 \ 10^{-23} \big)} \big) + 1.0000231454631447068 \ \text{W} \big(-43205.0114421103 \ e^{\big(-.432050114421103 \ 10^{-23} \big)} \big) + 1.0000231454631447068 \ \text{W} \big(-43205.0114421103 \ e^{\big(-.432050114421103 \ 10^{-23} \big)} \big) + 1.0000231454631447068 \ \text{W} \big(-43205.0114421103 \ e^{\big(-.432050114421103 \ 10^{-23} \big)} \big) + 1.0000231454631447068 \ \text{W} \big(-43205.0114421103 \ e^{\big(-.432050114421103 \ 10^{-23} \big)} \big) + 1.00002314421103 \ \text{W} \big(-.432050114421103 \ \text{W} \big) + 1.00002314421103 \ \text{W} \big(-.432050114421103 \ \text{W} \big) + 1.00002314421103 \ \text{W} \big(-.432050114421103 \ \text{W} \big) + 1.00002314421103 \ \text{W} \big(-.432050114421103 \ \text{W} \big) + 1.00002314421103 \ \text{W} \big(-.432050114421103 \ \text{W} \big) + 1.00002314421103 \ \text{W} \big(-.432050114421103 \ \text{W} \big) + 1.0000231421103 \ \text{W} \big(-.432050114421103 \ \text{W} \big) + 1.00002314421103 \ \text{W} \big(-.432050114421103 \ \text{W} \big) + 1.0000231421103 \ \text{W} \big(-.432050114421103 \ \text{W} \big) + 1.0000231421103 \ \text{W} \big(-.432050114421103 \ \text{W} \big) + 1.0000231421103 \ \text{W} \big(-.432050114421103 \ \text{W} \big) + 1.0000231421103 \ \text{W} \big(-.432050114421103 \ \text{W} \big) + 1.0000231421103 \ \text{W} \big(-.432050114421103 \ \text{W} \big) + 1.0000231421103 \ \text{W} \big(-.432050114421103 \ \text{W} \big) + 1.0000231421103 \ \text{W} \big(-.432050114421103 \ \text{W} \big) + 1.0000231421103 \ \text{W} \big(-.432050114421103 \ \text{W} \big) + 1.0000231421103 \ \text{W} \big(-.432050114421103 \ \text{W} \big) + 1.0000231421103 \ \text{W} \big(-.432050114421103 \ \text{W} \big) + 1.0000231421103 \ \text{W} \big(-.432050114421103 \ \text{W} \big) + 1.000023141103 \ \text{W} \big(-.432050114421103 \ \text{W} \big) + 1.000023141103 \ \text{W} \big(-.432050114421103 \ \text{W} \big) + 1.000023141103 \ \text{W} \big(-.432050114421103 \ \text{W} \big) + 1.000023141103 \ \text{W} \big(-.432050114421103 \ \text{W} \big) + 1.000023141103 \ \text{W} \big(-.432050114421103 \ \text{W} \big) + 1.000023141103 \ \text{W} \big(-$

 $\left(.404605358354905\ 10^{15}\ A\ x-.231454631447069\ 10^{24}\ln(cl)\ x+.1000000000000000\ 10^{29}\ cl\ x-.127947448591013\ 10^{15}\ \sqrt{x}\ A\right)/x\right)$

1.3e-06

1,4e-06

1.5e-06

> K2:='K2';

$$K2 := K2$$

> csad1:=(x,A,cl)->csad(x,A,C1b(cl,A,K2));

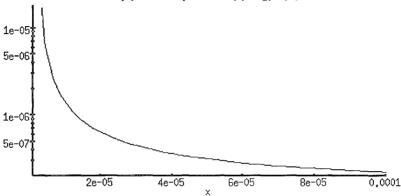
$$csad1 := (x, A, cl) \rightarrow csad(x, A, C1b(cl, A, K2))$$

> simplify(expand(csad1(x,A,cl)));

```
-.221118839214542\ 10^{-21}\ \frac{A\left(.7905694150421\ 10^{13}\sqrt{x}-.2500000000000010^{13}\right)}{2}
```

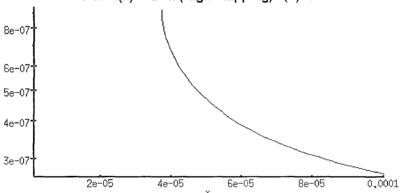
> p4:=logplot(cb(x,15e6,1K2,1e-7),x=1e-6..1e-4,title='FC:cb(x) -vs- x (Low Trapping) c(L)=1e-7'): > p4;

FC:cb(x) -ys-x (Low Trapping) c(L)=1e-7



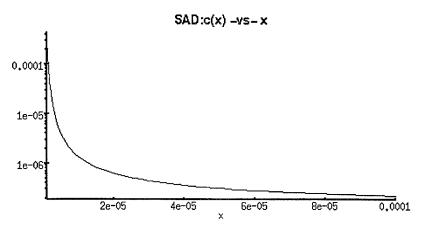
> p5:=logplot(cb(x,15e6,hK2,1e-7),x=1e-6..1e-4,resolution=100000, > title='FC:cb(x) -vs- x (High Trapping) c(L)=1e-7'): > p5;

FC:cb(x) -vs-x (High Trapping) c(L)=1e-7

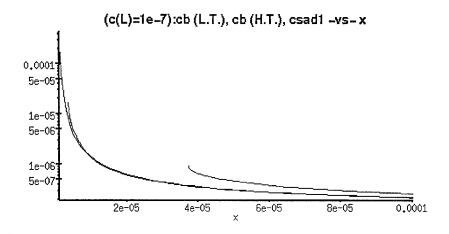


> p6:=logplot(csad1(x,15e6,1e-7),x=1e-6..1e-4, > resolution=10000,title='SAD:c(x) -vs- x'):

> p6;

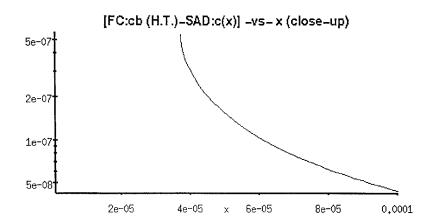


> display([p4,p5,p6],title='(c(L)=1e-7):cb (L.T.), cb (H.T.), csad1 -vs- x');

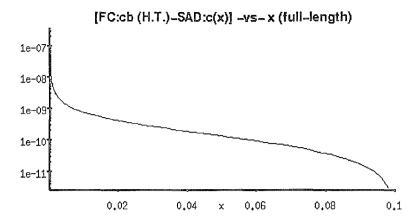


Note #16: The above plots indicate that the difference between SAD and Fully Coupled hydrogen concentration is strongly dependent on the degree of trapping.

> logplot(cb(x,15e6,hK2,1e-7)-csad1(x,15e6,1e-7),x=1e-6..1e-4, > resolution=10000,title='[FC:cb (H.T.)-SAD:c(x)] -vs- x (close-up)');

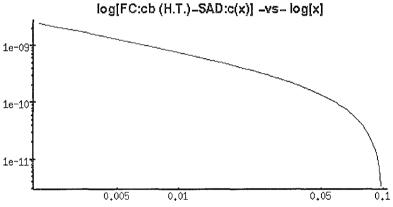


> logplot(cb(x,15e6,hK2,1e-7)-csad1(x,15e6,1e-7),x=1e-6..1e-1, > resolution=10000,title='[FC:cb (H.T.)-SAD:c(x)] -vs- x (full-length)');



> loglogplot([x,cb(x,15e6,hK2,1e-7)-csad1(x,15e6,1e-7),x=1e-6..1e-1], > resolution=10000,title='log[FC:cb (H.T.)-SAD:c(x)] -vs- log[x]');

> 150 1 (1.7.)



Note #17a: Calculate the stress, strain and displacements:

Note #17b: Stress is calculated by integration of the body force with the boundary condition that the right end of the rod is stress free.

> int(-A/x^(3/2),x);

$$2\frac{A}{\sqrt{x}}$$

 $> C2:=(x,A)->-(2*A)/x^{(1/2)};$

$$C2 := (x, A) \to -2 \frac{A}{\sqrt{x}}$$

> C2(0.1,15e6);

-.948683298050514 10⁸

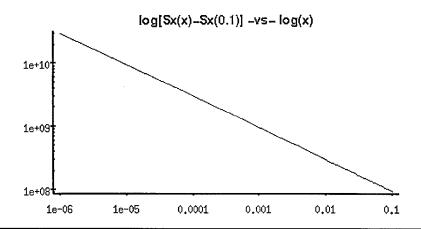
 $> Sx:=(x)->(2*15e6)/x^(1/2)+C2(0.1,15e6);$

$$Sx := x \to .30 \ 10^8 \frac{1}{\sqrt{x}} + C2(.1, .15 \ 10^8)$$

> Sx(x);

$$.30\ 10^8 \frac{1}{\sqrt{x}} - .948683298050514\ 10^8$$

> loglogplot([x,(Sx(x)-C2(0.1,15e6)),x=1e-6..0.1], title=`log[Sx(x)-Sx(0.1)] - vs-log(x)`);



Note #18: Strain is calculated from the constitutive relationship, Eq.(4).

Without displacements due to concentration, strain may be calculated from the standard linear-elastic constitutive relationship for comparison.

> eh:=x->Sx(x)/(3*K)+3*alpha*500*cb(x,15e6,hK2,1e-7);

$$eh := x \rightarrow \frac{1}{3} \frac{Sx(x)}{K} + 1500 \alpha cb(x, .15 \ 10^8, hK2, .1 \ 10^{-6})$$

> eh(x);

$$W\left(-.108012528605276\ 10^{7}\ e^{\left(-.200903303205813\ 10^{-14}\frac{.808962789010000\ 10^{16}\ x-.4127337051323\ 10^{13}\sqrt{x}}{x}\right)\right)$$

> e0:=x->Sx(x)/(3*K);

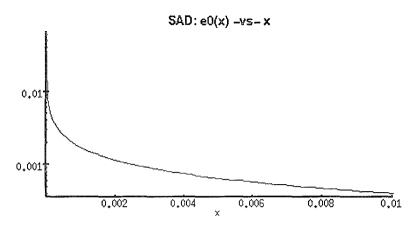
$$e0 := x \to \frac{1}{3} \frac{Sx(x)}{K}$$

> e0(x);

> p7:=logplot(e0(x),x=1e-6..0.01,title='SAD: e0(x) -vs- x'):

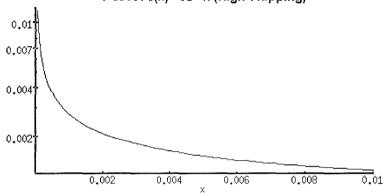
> p7;

>



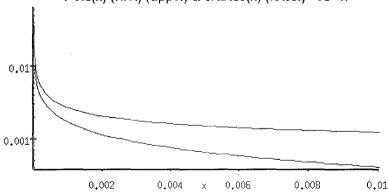
> p8:=logplot(eh(x),x=1e-6..0.01,title='FC500: e(x) -vs- x (High Trapping)'): > p8; >

FC500: e(x) -ys-x (High Trapping)

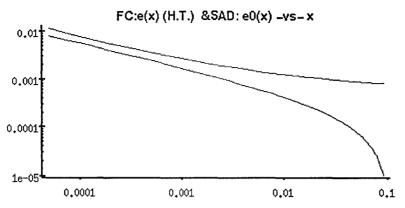


> display([p7,p8],title='FC:e(x) (H.T.) (upper) & SAD:e0(x) (lower) -vs- x');

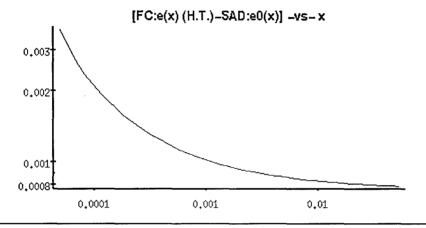
FC:e(x) (H.T.) (upper) & SAD:e0(x) (lower) -vs-x



```
> p9:=loglogplot([x,e0(x),x=5e-5..0.09]):
> p10:=loglogplot([x,eh(x),x=5e-5..0.09]):
> p10;
> display([p9,p10], title='FC:e(x) (H.T.) &SAD: e0(x) -vs- x');
```



> loglogplot([x,eh(x)-e0(x),x=5e-5..0.09],title='[FC:e(x)(H.T.)-SAD:e0(x)]-vs-x');



Note #19: Axial displacement is calculated by integrating the strain-displacement relation Eq.(6) over the length of the rod.

> ux:=x->(15e6/Em)*(4*sqrt(x)-6.32456*x)+alpha*500* > int(c(x,15e6,hK2,C1b(1e-7,15e6,hK2)),x=0..'x');

$$ux := x \to .15 \ 10^8 \frac{4 \ \text{sqrt}(x) - 6.32456 \ x}{Em}$$

$$+500 \text{ } \alpha \text{ int}(c(x, .15\ 10^8, hK2, C1b(.1\ 10^{-6}, .15\ 10^8, hK2)), x = 0 ... 'x')$$

> ux(x);

 $.925818525788272\ 10^{-6}$

$$W\left(-.108012528605276\ 10^{7}\ e^{\left(-.200903303205813\ 10^{-14}\ \frac{.808962789010000\ 10^{16}\ x-.4127337051323\ 10^{13}\ \sqrt{x}}{x}\right)\right)dx}$$

APPENDIX B: User Element Subroutine

This Appendix contains a copy of the Fortran user element subroutine for solving fully coupled deformation-diffusion problems in one-dimension. A listing of the subroutine is given first, and is followed by an annotated listing.

NOTE: This User Element Subroutine is used with ABAQUS Standard, Version 5.3.1, and is run on a Sun SPARC 10 workstation with the following software configuration: SUN OS 4.1:3; FORTRAN level V2.0.1; C level SC2.0.1 (libansia © Sun Microsystems, Inc.); X Windows level X11 Release 4.

<u>DISCLAIMER</u>: This ABAQUS "user element" subroutine is provided free of charge as a courtesy to the technical community. The authors and the University of Notre Dame accept no responsibility for any decisions based upon the use of this subroutine. Each user is advised that the subroutine has been tested against a limited number of steady-state deformation-diffusion problems, and has been found to give accurate answers to these particular test problems. The accuracy of this subroutine cannot be guaranteed in any particular future analysis.

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ABAOUS User Element Subroutine for Coupled Deformation-Diffusion Problems: Fortran 77 Code

```
subroutine UEL(RHS, AMATRX, SVARS, ENERGY, NDOFEL, NRHS, NSVARS, PROPS,
     1 NPROPS, COORDS, MCRD, NNODE, U, DU, V, A, JTYPE, TIME, DTIME, KSTEP, KINC,
     2 JELEM, PARAMS, NDLOAD, JDLTYP, ADLMAG, PREDEF, NPREDF, LFLAGS,
     3 MLVARX, DDLMAG, MDLOAD, PNEWDT)
C
         include 'ABA_PARAM.INC'
C
        implicit double precision (A-H,O-Z) <= input from included program
C
      dimension RHS(MLVARX, *), AMATRX(NDOFEL, NDOFEL), SVARS(NSVARS),
     1 ENERGY(8), PROPS(*), COORDS(MCRD, NNODE),
     2 U(NDOFEL), DU(MLVARX, *), V(NDOFEL), A(NDOFEL), TIME(2),
     3 PARAMS(3), JDLTYP(MDLOAD, *), ADLMAG(MDLOAD, *),
     4 DDLMAG (MDLOAD, *), PREDEF (NPREDF, NNODE, 2), LFLAGS (4)
C
      double precision AREA, XNU, BULK, ALPHA, D, XKON1, FUN,
     1 FNU(2), F(2), CREF, RHO, XMW, TEMP, RS, GW(2), GR(2), E, A1, A2, A3,
     2 B1, B2, B3, XJI(2), DJ(2), X(3), XGAUS(2), XKC(2,2), XKU(3,3),
     3 XKUC(3,2), CAP(2,2), E1, E2, DE, R(5), DEFF, TRAPS
      integer KFLAG
      double precision XJAC, DETJ, G1, G2, DG1, DG2, H1, H2, H3, DH1, DH2, DH3
      external XJAC, DETJ, G1, G2, DG1, DG2, H1, H2, H3, DH1, DH2, DH3
C
      * 1-D, 3-NODE ISOPARAMETRIC DISPLACEMENT/2-NODE SUPERPARAMETRIC
C
      * CONCENTRATION USER ELEMENT FOR MUTUALLY COUPLED
C
С
      * STATIC/TRANSIENT STRESS-ASSISTED DIFFUSION PROBLEMS
С
      * (CONSTANT CROSS-SECTIONAL AREA)
С
С
                                      2
С
                                                       3
            Node: 1
        /
            C
С
             Pos: x1----L/2----x2----L/2----x3
            Conc: c1
Disp: u1
С
                                                       c2
C
C
C
С
                      SOLUTION DEPENDENT STATE VARIABLES
            SVARS(1) = DILATATIONAL STRAIN AT INTEGRATION POINT GR(1) [1]
С
            SVARS(2) = DILATATIONAL STRAIN AT INTEGRATION POINT GR(2) [1]
С
           SVARS(3) = AXIAL STRESS AT INTEGRATION POINT GR(1)
SVARS(4) = AXIAL STRESS AT INTEGRATION POINT GR(2)
С
                                                                         [Pal
C
                                                                         [Pa]
С
            SVARS(5) = AXIAL STRESS AT LEFT END OF ELEMENT
            SVARS(6) = AXIAL STRESS AT RIGHT END OF ELEMENT
                                                                         [Pa]
           SVARS(7) = MASS FLUX AT THE CENTER OF EACH ELEMENT
C
                                                                        [m/s]
           SVARS(8) = X LOCATION OF INTEGRATION POINT GR(1)
SVARS(9) = X LOCATION OF INTEGRATION POINT GR(2)
С
                                                                          [m]
C
                                                                          [m]
C
С
                 BASED ON THEORY AND EQUATIONS BY DR. JAMES P. THOMAS
С
С
                 CODED BY CHARLES E. P. CHOPIN, 25 AUGUST 1994
                 LAST MODIFIED BY CHARLES E. P. CHOPIN, 13 MARCH 1995
C
C
                 DEPARTMENT OF AEROSPACE AND MECHANICAL ENGINEERING
C
                 UNIVERSITY OF NOTRE DAME, NOTRE DAME, INDIANA
С
С
                 DIRECT INQUIRIES REGARDING THIS USER ELEMENT
C
                 SUBROUTINE TO:
С
                             James. P. Thomas. 66@nd.edu
С
C
С
C
        DEGREES OF FREEDOM FOR MODEL IN ORDER:
С
                {C1.U1.U2.C2.U3}
C
C
        C
                         NODAL MASS FRACTION CONCENTRATION OF
С
                               SOLUTE
                                              [kgSOLUTE/kgSOLID]
                         NODAL DISPLACEMENT
                                                                         [ m ]
С
        KFLAG =
                         CASE FLAG FOR PLANE STRESS OR PLANE STRAIN
                                                                           [1]
                         CROSS-SECTIONAL AREA OF THE ROD
```

[m^2]

AREA

```
C
C
         E
                         YOUNG'S MODULUS FOR THE SOLID
                                                                            [Pa]
         XNU
                         POISSON'S RATIO FOR THE SOLID
                                                                             [1]
С
         D
                         DIFFUSION COEFFICIENT FOR SOLUTE IN SOLID [m^2/s]
         ALPHA
                         SOLUTE EXPANSION COEFFICIENT
00000000
                                                                             [11]
                          =(1/3)*(RHO*(PARTIAL MOLAR VOLUME OF SOLUTE))/XMW
         CREE
                =
                         REFERENCE CONCENTRATION
                                                            [kgSOLUTE/kgSOLID]
         RHO
                         DENSITY OF SOLID MIXTURE
                                                                        [kg/m^3]
         WMX
                         MOLECULAR WEIGHT FOR SOLUTE SPECIES
                                                                        [kg/mol]
         TEMP
                         AMBIENT TEMPERATURE (ASSUMED CONSTANT)
                                                                             [K]
         TRAPS
                         TRAP SITES CONSTANT (CSRKR IN (1+CSRKR) TERM) [1]
       KFLAG
               =int(PROPS(1))
       AREA
               =PROPS(2)
               =PROPS(3)
       Ε
       XNU
               =PROPS(4)
       D
                =PROPS(5)
       ALPHA
               =PROPS(6)
       CREF
               =PROPS(7)
       RHO
               =PROPS(8)
       WMX
                =PROPS(9)
       TEMP
               =PROPS(10)
       TRAPS = PROPS(11)
C
         2-POINT GAUSSIAN INTEGRATION LOCATIONS AND WEIGHTS
С
      data GR/-0.577350269189626D0, 0.577350269189626D0/,
            С
С
         RS
                          GAS CONSTANT FOR SOLUTE SPECIES
         BULK
                          BULK MODULUS FOR THE SOLID
\begin{smallmatrix} C & C & C & C \\ C & C & C & C \end{smallmatrix}
                          NON-DIMENSIONAL COEFFICIENT FOR (C*DE/DX) TERM OF
         XKON1
                          MASS FLUX EQUATION
                          (J=-DEFF*DC/DX + DEFF*XKON1*C*DE/DX)
         DEFF
                          EFFECTIVE DIFFUSION COEFFICIENT
С
                = 8.31432D0/XMW
                = E/(3.0D0*(1.0D0-2.0D0*XNU))
       BULK
       XKON1
               = 3.0D0*BULK*ALPHA/(RHO*RS*TEMP)
                = D/(1.0D0+TRAPS)
C
C
         SET CONSTANTS FOR PLANE STRESS OR PLANE STRAIN CASE
С
         1=> PLANE STRESS, FULLY COUPLED
C
         2=> PLANE STRAIN, FULLY COUPLED
         3=> PLANE STRESS, WEAKLY COUPLED
4=> PLANE STRAIN, WEAKLY COUPLED
С
C
C
        if(KFLAG.eq.1) then
                A1 = (1.0D0 - 2.0D0 * XNU)
                A2 = (2.0D0*(1.0D0+XNU)*ALPHA)*(1.0D0+TRAPS)
                A3 = (A2 * CREF)
                B1 = (3.0D0*BULK)
                B2 = (9.0D0*BULK*ALPHA)*(1.0D0+TRAPS)
                B3 = (B2 * CREF)
        elseif(KFLAG.eq.2) then
                A1=1.0D0
                A2 = 0.000
                A3 = 0.0D0
                B1 = (1.0D0 - XNU) *E/((1.0D0 + XNU) * (1.0D0 - 2.0D0 * XNU))
                B2=(3.0D0*BULK*ALPHA)*(1.0D0+TRAPS)
                B3 = (B2 *CREF)
        elseif(KFLAG.eq.3) then
                A1 = (1.0D0 - 2.0D0 * XNU)
                A2 = 0.0D0
                A3 = 0.0D0
                B1=(3.0D0*BULK)
                B2 = 0.0D0
                B3 = 0.0D0
        elseif(KFLAG.eq.4) then
                A1 = 1.0D0
                A2 = 0.000
                A3 = 0.0D0
                B1 = (1.0D0 - XNU) *E/((1.0D0 + XNU) * (1.0D0 - 2.0D0 * XNU))
                B2 = 0.0D0
                B3 = 0.000
```

```
endif
C
С
        SET X COORDINATES FOR NODES ON CURRENT ELEMENT
C
        X(1) = COORDS(1,1) + U(2)
        X(2) = COORDS(1,2) + U(3)
        X(3) = COORDS(1,3) + U(5)
C
С
        SET GLOBAL COORDINATES FOR GAUSS POINTS ON CURRENT ELEMENT
C
        XGAUS(1) = H1(GR(1)) *X(1) + H2(GR(1)) *X(2) + H3(GR(1)) *X(3)
        XGAUS(2) = H1(GR(2)) * X(1) + H2(GR(2)) * X(2) + H3(GR(2)) * X(3)
C
С
        CALCULATE INVERSE JACOBIAN AND DETERMINANT FOR ISOPARAMETRIC
С
        TRANSFORMATION
С
         do 20 I = 1.2
              XJI(I) = 1.0D0/XJAC(GR(I), X(1), X(2), X(3))
              DJ(I) = DETJ(GR(I), X(1), X(2), X(3))
  2.0
         if(LFLAGS(1).ne.71)then
C
С
        С
                = AREA*(G1(GR(1))*G1(GR(1))*DJ(1)*GW(1)
       CAP(1,1)
                  + G1(GR(2))*G1(GR(2))*DJ(2)*GW(2))
                = AREA*(G1(GR(1))*G2(GR(1))*DJ(1)*GW(1)
                  + G1(GR(2))*G2(GR(2))*DJ(2)*GW(2))
                 = AREA*(G2(GR(1))*G1(GR(1))*DJ(1)*GW(1)
       CAP(2,1)
                  + G2(GR(2))*G1(GR(2))*DJ(2)*GW(2))
                 = AREA*(G2(GR(1))*G2(GR(1))*DJ(1)*GW(1)
       CAP(2,2)
                  + G2(GR(2))*G2(GR(2))*DJ(2)*GW(2))
       endif
С
C
С
        С
С
       CALCULATE de/dr FROM PREVIOUS TIME STEP FOR NON-LINEAR TERM
C
       E1=SVARS(1)
       E2=SVARS(2)
       DE = (E2-E1)/(GR(2)-GR(1))
C
       COEFFICIENTS OF CONCENTRATION D.O.F. IN MASS BALANCE EQUATION
     XKC(1,1) = AREA*DEFF*(XJI(1)*(DG1(GR(1))*DG1(GR(1))
                   -XKON1*DG1(GR(1))*DE*G1(GR(1)))*GW(1)
    2
                     +XJI(2)*(DG1(GR(2))*DG1(GR(2))
                  -XKON1*DG1(GR(2))*DE*G1(GR(2)))*GW(2))
    3
      XKC(1,2) = AREA*DEFF*(XJI(1)*(DG1(GR(1))*DG2(GR(1)))
                  -XKON1*DG1(GR(1))*DE*G2(GR(1)))*GW(1)
    2
                    + XJI(2)*(DG1(GR(2))*DG2(GR(2))
    3
                  -XKON1*DG1(GR(2))*DE*G2(GR(2)))*GW(2))
      XKC(2,1) = AREA*DEFF*(XJI(1)*(DG2(GR(1))*DG1(GR(1))
                  -XKON1*DG2(GR(1))*DE*G1(GR(1)))*GW(1)
    2
                    + XJI(2)*(DG2(GR(2))*DG1(GR(2))
                  -XKON1*DG2(GR(2))*DE*G1(GR(2)))*GW(2))
     XKC(2,2) = AREA*DEFF*(XJI(1)*(DG2(GR(1))*DG2(GR(1))
                  -XKON1*DG2(GR(1))*DE*G2(GR(1)))*GW(1)
                     + XJI(2)*(DG2(GR(2))*DG2(GR(2))
                  -XKON1*DG2(GR(2))*DE*G2(GR(2)))*GW(2))
C
C
C
        COEFFICIENTS OF CONCENTRATION IN LINEAR MOMENTUM BALANCE EQUATION
C
      XKUC(1,1) = AREA*(B1*A2-B2)*((DH1(GR(1))*G1(GR(1)))*GW(1)
                          + (DH1(GR(2))*G1(GR(2)))*GW(2))
```

```
XKUC(1,2) = AREA*(B1*A2-B2)*((DH1(GR(1))*G2(GR(1)))*GW(1)
                           + (DH1(GR(2))*G2(GR(2)))*GW(2))
     XKUC(2,1) = AREA*(B1*A2-B2)*((DH2(GR(1))*G1(GR(1)))*GW(1)
                           + (DH2(GR(2))*G1(GR(2)))*GW(2))
     XKUC(2,2) = AREA*(B1*A2-B2)*((DH2(GR(1))*G2(GR(1)))*GW(1)
     1
                           + (DH2(GR(2))*G2(GR(2)))*GW(2))
     XKUC(3,1) = AREA*(B1*A2-B2)*((DH3(GR(1))*G1(GR(1)))*GW(1)
                           + (DH3(GR(2))*G1(GR(2)))*GW(2))
     XKUC(3,2) = AREA*(B1*A2-B2)*((DH3(GR(1))*G2(GR(1)))*GW(1)
                           + (DH3(GR(2))*G2(GR(2)))*GW(2))
С
С
С
        COEFFICIENTS OF DISPLACEMENT IN LINEAR MOMENTUM BALANCE EQUATION
     XKU(1,1) = AREA*B1*A1*(XJI(1)*(DH1(GR(1))*DH1(GR(1)))*GW(1)
                      + XJI(2)*(DH1(GR(2))*DH1(GR(2)))*GW(2))
     XKU(1,2) = AREA*B1*A1*(XJI(1)*(DH1(GR(1))*DH2(GR(1)))*GW(1)
     1
                      + XJI(2)*(DH1(GR(2))*DH2(GR(2)))*GW(2))
     XKU(1,3) = AREA*B1*A1*(XJI(1)*(DH1(GR(1))*DH3(GR(1)))*GW(1)
                      + XJI(2)*(DH1(GR(2))*DH3(GR(2)))*GW(2))
     XKU(2,1) = AREA*B1*A1*(XJI(1)*(DH2(GR(1))*DH1(GR(1)))*GW(1)
     1
                      + XJI(2)*(DH2(GR(2))*DH1(GR(2)))*GW(2))
     XKU(2,2) = AREA*B1*A1*(XJI(1)*(DH2(GR(1))*DH2(GR(1)))*GW(1)
                      + XJI(2)*(DH2(GR(2))*DH2(GR(2)))*GW(2))
      XKU(2,3) = AREA*B1*A1*(XJI(1)*(DH2(GR(1))*DH3(GR(1)))*GW(1)
                      + XJI(2)*(DH2(GR(2))*DH3(GR(2)))*GW(2))
      XKU(3,1) = AREA*B1*A1*(XJI(1)*(DH3(GR(1))*DH1(GR(1)))*GW(1)
                      + XJI(2)*(DH3(GR(2))*DH1(GR(2)))*GW(2))
      XKU(3,2) = AREA*B1*A1*(XJI(1)*(DH3(GR(1))*DH2(GR(1)))*GW(1)
                       + XJI(2)*(DH3(GR(2))*DH2(GR(2)))*GW(2))
      XKU(3,3) = AREA*B1*A1*(XJI(1)*(DH3(GR(1))*DH3(GR(1)))*GW(1)
                       + XJI(2)*(DH3(GR(2))*DH3(GR(2)))*GW(2))
C
C
       if(LFLAGS(1).eq.71) then
C
C
        ******* STEADY STATE DIFFUSION ************
C
С
       if (LFLAGS(3).eq.1.or.LFLAGS(3).eq.2) then
C
         <><<< CALCULATE AMATRX = [K] >>>>>>>
         WHERE [K] IS
                [XKC(1,1) 0]
                                  0
                                         XKC(1,2) 0
                [XKUC(1,1) XKU(1,1) XKU(1,2) XKUC(1,2) XKU(1,3)]
               [XKUC(2,1) XKU(2,1) XKU(2,2) XKUC(2,2) XKU(2,3)]
               [XKC(2,1) 0]
C
                                         XKC(2,2) 0
С
               [XKUC(3,1) XKU(3,1) XKU(3,2) XKUC(3,2) XKU(3,3)]
       AMATRX(1,1) = XKC(1,1)
       AMATRX(1,4) = XKC(1,2)
       AMATRX(2,1) = XKUC(1,1)
       AMATRX(2,2) = XKU(1,1)
       AMATRX(2,3) = XKU(1,2)
       AMATRX(2,4) = XKUC(1,2)
       AMATRX(2,5) = XKU(1,3)
       AMATRX(3,1) = XKUC(2,1)
       AMATRX(3,2) = XKU(2,1)
       AMATRX(3,3) = XKU(2,2)
       AMATRX(3,4) = XKUC(2,2)
       AMATRX(3,5) = XKU(2,3)
       AMATRX(4,1) = XKC(2,1)
```

```
AMATRX(4,4) = XKC(2,2)
       AMATRX(5,1) = XKUC(3,1)
       AMATRX(5,2) = XKU(3,1)
       AMATRX(5,3) = XKU(3,2)
       AMATRX(5,4) = XKUC(3,2)
       AMATRX(5,5) = XKU(3,3)
       endif
C
       if((LFLAGS(3).eq.1).or.(LFLAGS(3).eq.5)) then
C
С
С
        <><< CALCULATE {RHS} = {R}-[K]{U} >>>>>>>
c
              THIS USER ELEMENT ASSUMES THAT ALL OF THE
         NB ·
               NEUMANN BOUNDARY DATA ARE HOMOGENEOUS (i.e., ZERO).
C
               INHOMOGENEOUS (NON-ZERO) NEUMANN BOUNDARY DATA SUCH
С
               AS NON-ZERO BOUNDARY TRACTIONS AND/OR SOLUTE FLUXES
С
               ARE AUTOMATICALLY INCORPORATED INTO THE COMPUTATIONS
С
               BY ABAQUS. INHOMOGENEOUS DATA SHOULD BE ENTERED IN
С
               THE INPUT DECK AS CONCENTRATED NODAL FORCES.
С
C
       THE FOLLOWING SECTION CALCULATES THE TOTAL USER SPECIFIED
C
       DISTRIBUTED LOADS (e.g. BODY FORCES) AT THE GAUSS POINTS
C
       ON THE ROD FOR USE IN FINDING THE RHS VECTOR.
С
      FUN=0.0D0
       do 31 I=1,2
          F(I) = 0.0D0
          FNU(I) = 0.0D0
  31
       continue
       do 41 K1=1, MDLOAD
             if(JDLTYP(K1,1).gt.0)then
                 FUN=FUN+ADLMAG(K1,1)
             elseif(JDLTYP(K1,1).lt.0)then
                 call KFCALC(JDLTYP(K1,1),F,XGAUS,TIME(2))
                 do 51 I=1,2
                    FNU(I) = FNU(I) + F(I)
  51
                 continue
             endif
  41
        continue
        do 61 I=1,2
           FNU(I)=FNU(I)+FUN
  61
        continue
С
С
С
       R(1) = 0.0D0
\overline{C}
       R(2) = AREA*((H1(GR(1))*FNU(1)
     1
              -(B3-B1*A3)*XJI(1)*DH1(GR(1)))*DJ(1)*GW(1)
     2
              +(H1(GR(2))*FNU(2)
              -(B3-B1*A3)*XJI(2)*DH1(GR(2)))*DJ(2)*GW(2))
     3
C
       R(3) = AREA*((H2(GR(1))*FNU(1)
     1
             -(B3-B1*A3)*XJI(1)*DH2(GR(1)))*DJ(1)*GW(1)
     2
             +(H2(GR(2))*FNU(2)
             -(B3-B1*A3)*XJI(2)*DH2(GR(2)))*DJ(2)*GW(2))
С
       R(4) = 0.0D0
C
       R(5) = AREA*((H3(GR(1))*FNU(1)
     1
              -(B3-B1*A3)*XJI(1)*DH3(GR(1)))*DJ(1)*GW(1)
     2
             +(H3(GR(2))*FNU(2)
              -(B3-B1*A3)*XJI(2)*DH3(GR(2)))*DJ(2)*GW(2))
C
       RHS(1,1) = R(1) - XKC(1,1) *U(1) - XKC(1,2) *U(4)
С
       RHS(2,1) = R(2) - XKUC(1,1) * U(1) - XKU(1,1) * U(2) - XKU(1,2) * U(3)
     1
                        -XKUC(1,2)*U(4)-XKU(1,3)*U(5)
C
       RHS(3,1) = R(3) - XKUC(2,1) *U(1) - XKU(2,1) *U(2) - XKU(2,2) *U(3)
     1
                        -XKUC(2,2)*U(4)-XKU(2,3)*U(5)
С
       RHS(4,1) = R(4) - XKC(2,1) *U(1) - XKC(2,2) *U(4)
С
       RHS(5,1) = R(5) - XKUC(3,1) *U(1) - XKU(3,1) *U(2) - XKU(3,2) *U(3)
```

```
1
                           -XKUC(3,2)*U(4)-XKU(3,3)*U(5)
С
С
        endif
       elseif((LFLAGS(1).eq.72).or.(LFLAGS(1).eq.73)) then
C
c
c
         ******* TRANSIENT DIFFUSION ************
С
С
       if (LFLAGS(3).eq.1.or.LFLAGS(3).eq.2) then
С
С
           <><< CALCULATE AMATRX = [[K]+[C]/DTIME] >>>>>>>>>
С
          WHERE [K] IS
С
                  [XKC(1,1)
                                       0
                                                XKC(1,2) 0
C
                  [XKUC(1,1) XKU(1,1) XKU(1,2) XKUC(1,2) XKU(1,3)]
[XKUC(2,1) XKU(2,1) XKU(2,2) XKUC(2,2) XKU(2,3)]
C
                  [XKC(2,1) 0]
                                       0
                                               XKC(2,2) 0
                  [XKUC(3,1) XKU(3,1) XKU(3,2) XKUC(3,2) XKU(3,3)]
C
C
C
С
          AND [C] IS
00000
                  [CAP(1,1) 0
                                      0
                                                CAP(1,2) 0]
                  0]
                                      0
                                                0
                                                          0]
                                                n
                                                          0]
                  f O
                                      0
                             Ω
                  [CAP(2,1) 0]
                                       0
                                                CAP(2,2) 0]
                                                0
                                                          0]
С
C
       \mathtt{AMATRX}(1,1) \ = \ \mathtt{XKC}(1,1) \ + \ \mathtt{CAP}(1,1) \ / \mathtt{DTIME}
       AMATRX(1,4) = XKC(1,2) + CAP(1,2)/DTIME
       AMATRX(2,1) = XKUC(1,1)
       AMATRX(2,2) = XKU(1,1)
       AMATRX(2,3) = XKU(1,2)
       AMATRX(2,4) = XKUC(1,2)
       AMATRX(2,5) = XKU(1,3)
       AMATRX(3,1) = XKUC(2,1)
       AMATRX(3,2) = XKU(2,1)
       AMATRX(3,3) = XKU(2,2)
       AMATRX(3,4) = XKUC(2,2)
       AMATRX(3,5) = XKU(2,3)
       AMATRX(4,1) = XKC(2,1) + CAP(2,1)/DTIME
       \begin{array}{lll} {\rm AMATRX}\,(4,4) &=& {\rm XKC}\,(2,2) \,+\, {\rm CAP}\,(2,2)\,/{\rm DTIME} \\ {\rm AMATRX}\,(5,1) &=& {\rm XKUC}\,(3,1) \end{array}
       AMATRX(5,2) = XKU(3,1)
       AMATRX(5,3) = XKU(3,2)
       AMATRX(5,4) = XKUC(3,2)
       AMATRX(5,5) = XKU(3,3)
       endif
С
С
С
       if((LFLAGS(3).eq.1).or.(LFLAGS(3).eq.5)) then
С
С
          <><< CALCULATE {RHS} = {R}-[K]{U}-([C]/DTIME){DU}>>>>>
С
С
         THE FOLLOWING SECTION CALCULATES THE TOTAL USER SPECIFIED
        DISTRIBUTED LOADS (e.g. BODY FORCES) AT THE GAUSS POINTS
С
        ON THE ROD FOR USE IN FINDING THE RHS VECTOR.
C
C
        FUN=0.0D0
         do 131 I=1,2
            F(I) = 0.0D0
            FNU(I) = 0.0D0
  131
         continue
         do 141 K1=1, MDLOAD
              if(JDLTYP(K1,1).gt.0)then
                   FUN=FUN+ADLMAG(K1,1)
               elseif(JDLTYP(K1,1).lt.0)then
                   call KFCALC(JDLTYP(K1,1),F,XGAUS,TIME(2))
                   do 151 I=1.2
                      FNU(I) = FNU(I) + F(I)
  151
                   continue
               endif
```

```
141
        continue
        do 161 I=1,2
           FNU(I)=FNU(I)+FUN
 161
        continue
С
С
       R(1) = 0.0D0
С
       R(2) = AREA*((H1(GR(1))*FNU(1)
     1
             -(B3-B1*A3)*XJI(1)*DH1(GR(1)))*DJ(1)*GW(1)
             + (H1 (GR(2)) *FNU(2)
     3
             -(B3-B1*A3)*XJI(2)*DH1(GR(2)))*DJ(2)*GW(2))
С
       R(3) = AREA*((H2(GR(1))*FNU(1))
             -(B3-B1*A3)*XJI(1)*DH2(GR(1)))*DJ(1)*GW(1)
     1
             +(H2(GR(2))*FNU(2)
     2
             -(B3-B1*A3)*XJI(2)*DH2(GR(2)))*DJ(2)*GW(2))
С
       R(4) = 0.0D0
C
       R(5) = AREA*((H3(GR(1))*FNU(1)
             -(B3-B1*A3)*XJI(1)*DH3(GR(1)))*DJ(1)*GW(1)
     7
             +(H3(GR(2))*FNU(2)
     2
             -(B3-B1*A3)*XJI(2)*DH3(GR(2)))*DJ(2)*GW(2))
C
C
       RHS(1,1) = R(1) - XKC(1,1) *U(1) - XKC(1,2) *U(4)
                       -1.0D0/DTIME*(CAP(1,1)*DU(1,1)+CAP(1,2)*DU(4,1))
C
       RHS(2,1) = R(2) - XKUC(1,1) *U(1) - XKU(1,1) *U(2) - XKU(1,2) *U(3)
                       -XKUC(1,2)*U(4)-XKU(1,3)*U(5)
C
       С
       RHS(4,1) = R(4) - XKC(2,1) *U(1) - XKC(2,2) *U(4)
                       -1.0D0/DTIME*(CAP(2,1)*DU(1,1)+CAP(2,2)*DU(4,1))
С
       RHS(5,1) = R(5) - XKUC(3,1) * U(1) - XKU(3,1) * U(2) - XKU(3,2) * U(3)
     1
                       -XKUC(3,2)*U(4)-XKU(3,3)*U(5)
0
С
       endif
С
C
     <><<< CHECK THAT PROCEDURE FLAG SENT MATCHES PROCEDURE PERFORMED >>>
C
      elseif((LFLAGS(1).ne.71).and.
         (LFLAGS(1).ne.72).and.(LFLAGS(1).ne.73)) then
      write(*,*) 'ERROR IN THE VALUE OF LFLAGS(1) = ',LFLAGS(1)
        endif
C
      if(LFLAGS(3).eq.1) then
С
      <><<<<< CALCULATE STRESSES AND STRAINS AT THE GAUSS POINTS >>>
C
         DILATATIONAL STRAINS:
         SVARS(1) = A1*XJI(1)*(DH1(GR(1))*U(2)
                           +DH2(GR(1))*U(3)+DH3(GR(1))*U(5))
     2
                    +A2*(G1(GR(1))*U(1)+G2(GR(1))*U(4))-A3
         SVARS(2) = A1*XJI(2)*(DH1(GR(2))*U(2)+
                            DH2 (GR(2))*U(3)+DH3 (GR(2))*U(5))
                    +A2*(G1(GR(2))*U(1)+G2(GR(2))*U(4))-A3
C
         GAUSS POINT STRESSES:
         SVARS(3) = B1*SVARS(1) - B2*(G1(GR(1))*U(1)+G2(GR(1))*U(4))+B3
         SVARS(4) = B1*SVARS(2)-B2*(G1(GR(2))*U(1)+G2(GR(2))*U(4))+B3
C
         OUTER NODE STRESSES:
         DE = (SVARS(2)-SVARS(1))/(GR(2)-GR(1))
         SVARS(5) = B1*(SVARS(1)+(-1.0D0-GR(1))*DE)-B2*U(1)+B3
         SVARS(6) = B1*(SVARS(2) + (1.0D0-GR(2))*DE)-B2*U(4)+B3
         CENTROIDAL SOLUTE MASS FLUX
C
         SVARS(7) = D*(1.0D0/XJAC(0.0D0,X(1),X(2),X(3)))
```

```
*(-(DG1(0.0D0)*U(1)+DG2(0.0D0)*U(4))+
                   XKON1*(G1(0.0D0)*U(1)+G2(0.0D0)*U(4))*DE)
С
      GLOBAL GAUSS POINTS LOCATIONS
         SVARS(8)=XGAUS(1)
         SVARS(9)=XGAUS(2)
С
С
     endif
     return
     end
С
C '
С
        ***********
С
С
     double precision function XJAC(R,X1,X2,X3)
      double precision R, X1, X2, X3
С
      CALCULATE JACOBIAN OF ISOPARAMETRIC TRANSFORMATION
C
          (D/DR=XJAC^{(-1)*D/DX)
            XJAC=(R-1.0D0/2.0D0)*X1-2.0D0*R*X2+(R+1.0D0/2.0D0)*X3
      return
      end
С
Ċ
Č
С
      double precision function DETJ(R,X1,X2,X3)
      double precision R,X1,X2,X3
      CALCULATE DETERMINANT OF JACOBIAN OF ISOPARAMETRIC TRANSFORMATION
С
C
      (DR=DETJ*DX)
           DETJ=(R-1.0D0/2.0D0)*X1-2.0D0*R*X2+(R+1.0D0/2.0D0)*X3
      return
      end
С
С
Ċ
C
      LINEAR INTERPOLATION FUNCTIONS FOR CONCENTRATION
      double precision function G1(R)
      double precision R
            G1 = (1.0D0/2.0D0) * (1.0D0-R)
      return
      end
С
      double precision function G2(R)
      double precision R
            G2 = (1.0D0/2.0D0) * (1.0D0+R)
      return
      end
С
С
С
С
       QUADRATIC INTERPOLATION FUNCTIONS FOR DISPLACEMENT
С
      double precision function H1(R)
      double precision R
            H1=1.0D0/2.0D0*(R**2-R)
      return
      end
С
      double precision function H2(R)
      double precision R
            H2=1.0D0-R**2
      return
      end
С
С
      double precision function H3(R)
      double precision R
            H3=1.0D0/2.0D0*(R**2+R)
      return
      end
С
```

```
C
C
      DERIVATIVES OF LINEAR INTERPOLATIONS FOR CONCENTRATION
С
      double precision function DG1(R)
      double precision R
            DG1 = (-1.0D0/2.0D0)
      return
      end
С
^{\circ}
      double precision function DG2(R)
      double precision R
            DG2 = (1.0D0/2.0D0)
      return
      end
C
С
      DERIVATIVES OF QUADRATIC INTERPOLATIONS FOR DISPLACEMENT
Ċ
      double precision function DH1(R)
      double precision R
            DH1 = (R-1.0D0/2.0D0)
      return
      end
С
      double precision function DH2(R)
      double precision R
            DH2 = -2.0D0*R
      return
      end
С
C
      double precision function DH3(R)
      double precision R
            DH3 = (R+1.0D0/2.0D0)
      return
      end
С
C
С
      DISTRIBUTED LOAD SPECIFICATION ROUTINE
      subroutine KFCALC(JDLTYP,F,XGAUS,TTIME)
      double precision F(*), XGAUS(*), TTIME
      integer JDLTYP, I
      double precision A, EPS
С
      THIS LOAD ROUTINE CALCULATES THE BODY FORCE AT THE
C
      GAUSS POINTS FOR USE IN FINDING THE RHS VECTOR
      IF (JDLTYP.EQ.-1) THEN
С
         LOAD TYPE 1 INDICATES A DISTRIBUTED BODY FORCE OF THE FORM F(X) = A/(X+EPS)^3/2
С
С
         GRAVITATIONAL BODY FORCE FOR 4340 STEEL IS ~80.0D3 [N/m^3]
C
          'A' SHOULD BE SPECIFIED IN UNITS OF [FORCE/LENGTH^3/2]
         A=15.0D6
         EPS=1.0D-18
        DO 100 I=1,2
  100
       F(I) = A/sqrt((abs(XGAUS(I))+EPS)**3.0D0)
      ENDIF
      return
      end
```

ABAQUS User Element Subroutine for Coupled Deformation-Diffusion Problems: Annotated Description

DISCLAIMER

This ABAQUS "user element" subroutine is provided free of charge as a courtesy to the technical community. The authors and the University of Notre Dame accept no responsibility for any decisions based upon the use of this subroutine. Each user is advised that the subroutine has been tested against a limited number of steady-state deformation-diffusion problems, and has been found to give accurate answers to these particular test problems. The accuracy of this subroutine cannot be guaranteed in any particular future analysis.

Format of the Coding Notes

The left hand side of the page contains the actual Fortran coding of the ABAQUS User Element subroutine and the right hand side of the page contains explanations of the coding and assumptions, and justifications for certain coding techniques. Blank lines have been added to the subroutine in this listing for ease of readability.

This one-dimensional subroutine is primarily intended as a stepping-stone to a two-dimensional subroutine. A clear understanding of the one-dimensional formulation, which uses 5×5 capacitance and stiffness matrices, will help simplify the task of understanding the corresponding two-dimensional model which requires 20×20 matrices.

The notes on the right side of the page are intended to be as complete as is possible without repeating the comments already in the code. Familiarity with the standard ABAQUS user element variables is assumed. Additional information concerning these variables can be found in the ABAQUS User's Manual (Book II; Section 5.2.16). All further references to the ABAQUS User's Manual will be denoted by AUM.

BOLD, UNDERLINED, AND CAPITALIZED type. An attempt has been made to refer to all Fortran variables in COURIER CAPITALIS and any variables which relate directly to the analysis and derivation of this code in variable script.

Description of the Subroutine

This "User Element" subroutine is useful for analyzing steady-state and transient one-dimensional deformation-diffusion problems. By changing certain parameters in the ABAQUS input deck, the user may choose between plane-stress or plane-strain constitutive behavior, and fully-coupled, weakly-coupled (Stress Assisted Diffusion), or uncoupled behavior of the solute-solid system.

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subroutine UEL (RHS, AMATRX, SVARS, ENERGY, NDOFEL, NRHS, NSVARS, PROPS, 1 NPROFS, COORDS, MCRD, NNODE, U, DU, V, A, JTYPE, TIME, DTIME, KSTEP, KINC, 2 JELEM, PARAMS, NDLOAD, JDLTYP, ADLMAG, PREDEF, NPREDF, LFLAGS, MLVARX, DDLMAG, MDLOAD, PNEWDT)

U υv

include 'ABA_PARAM.INC'

implicit double precision (A-H, 0-Z) <= input from included program

dimension RHS(MLVARX,*), AMATRX(NDOFEL,NDOFEL), SVARS(NSVARS), 1 ENERGY(8), PROPS(*), COORDS(MCRD, NNODE),
2 U(NDOFEL), DU(MLVARX, *), V(NDOFEL), A(NDOFEL), TIME(2),
3 PARAMS(3), JDLIYP(MDLOAD, *), ADLMAG(MDLOAD, *),
4 DDLMAG(MDLOAD, *), PREDEF(NPREDF, NNODE, 2), LFLAGS(4)

O

integer KFLAG

double precision XJAC, DETJ, G1, G2, DG1, DG2, H1, H2, H3, DH1, DH2, DH3 external XJAC, DETJ, G1, G2, DG1, DG2, H1, H2, H3, DH1, DH2, DH3

in the AUM (II; 5.2.16-1). These three statements are the same for all User Elements. ABAQUS uses implicit type definition of variables. The program 'ABA_PARAM. INC' specifies whether the implicit variables are REAL or DOUBLE PRECISION based on the configuration of the The initial subroutine declaration, include statement, and dimension statement are provided computer used in the analysis.

DECLARE all local double precision variables.

DECLARE all local double precision arrays.

EXTERNAL FUNCTIONS are defined and used by subroutine UEL. DECLARE functions used by subroutine UEL as double precision.

NOTE: ABAQUS uses implicit declaration of variables to simplify the transition between computers using double and single precision variable defaults. All local variables in this routine are explicitly declared double precision, but the variables are named to take advantage of ABAQUS implicit declarations found in 'ABA_PARAM. INC' should this be necessary or desired.

SVARS(1)= DILATATIONAL STRAIN AT INTEGRATION POINT GR(1) [1]
SVARS(2)= DILATATIONAL STRAIN AT INTEGRATION POINT GR(2) [1]
SVARS(3)= AXIAL STRESS AT INTEGRATION POINT GR(1)
SVARS(4)= AXIAL STRESS AT INTEGRATION POINT GR(2)
SVARS(5)= AXIAL STRESS AT LEFT END OF ELEMENT
SVARS(5)= AXIAL STRESS AT LIFT END OF ELEMENT
SVARS(6)= AXIAL STRESS AT RIGHT END OF ELEMENT
SVARS(6)= AXIAL STRESS AT RIGHT END OF ELEMENT
[Pa]
SVARS(7)= MASS FLUX AT THE CENTER OF EACH ELEMENT
[Pa] EE 1-D, 3-NODE ISOPARAMETRIC DISPLACEMENT/2-NODE SUPERPARAMETRIC CONCENTRATION USER ELEMENT FOR MUTUALLY COUPLED STATIC/TRANSIENT STRESS-ASSISTED DIFFUSION PROBLEMS BASED ON THEORY AND EQUATIONS BY DR. JAMES P. THOMAS CODED BY CHARLES E. P. CHOPIN, 25 AUGUST 1994 LAST MODIFIED BY CHARLES E. P. CHOPIN, 13 MARCH 1995 DEPARTMENT OF AEROSPACE AND MECHANICAL ENGINEERING DIRECT E-MAIL INQUIRIES REGARDING THIS USER ELEMENT DEPARTMENT OF AEROSPACE AND MECHANICAL ENGINES UNIVERSITY OF NOTRE DAME, NOTRE DAME, INDIANA SVARS(8) = X LOCATION OF INTEGRATION POINT GR(1) SVARS(9) = X LOCATION OF INTEGRATION POINT GR(2)SOLUTION DEPENDENT STATE VARIABLES Node: 1 2 3 x1----L/2----x3 c2 u3 James.P. Thomas66@nd.edu (CONSTANT CROSS-SECTIONAL AREA) u2 SUBROUTINE TO: c_1 ul Disp: Pos:

The user element has three nodes. Concentration degrees-of-freedom (dof) are assigned at the two outer nodes, and axial displacement dof are assigned at all three nodes. An isoparametric element formulation is used in this code. That is, the element calculations are carried out in terms of a local space coordinate, r, which has values of -1,0, and +1 at the left, center, and right-hand nodes, respectively.

The concentration interpolation functions are linear, and the displacement interpolation functions are quadratic. The resulting finite element is therefore superparametric in the concentration and isoparametric in displacement.

STATE VARIABLES, SVARS for dilatational strain are used in calculation of the ABAQUS "Jacobian" matrix AMATRX. State variables for stress, mass flux, and the gauss point locations are calculated for output purposes only. Some additional notes regarding the calculation of these state variables can be found near the end of the main body of the code.

```
[1]
[m^2]
                                                                                                                                                                   =(1/3)*(RHO*(PARTIAL MOLAR VOLUME OF SOLUTE))/XMW
                                                                                                                                                                                                                         MOLECULAR WEIGHT FOR SOLUTE SPECIES [Kg/mol]
AMBIENT TEMPERATURE (ASSUMED CONSTANT) [K]
TRAP SITES CONSTANT (CSRKR IN (1+CSRKR) [1]
                                                                                             [Pa]
                                                                                                                              DIFFUSION COEFFICIENT FOR SOLUTE IN SOLID [m^2/s] SOLUTE EXPANSION COEFFICIENT
                                                                                                                                                                                                          kg/m^3]
                  [kgsolute/kgsolib]
                                                                                                                                                                                        [kgSolid/kgSolute]
                                                      FLAG FOR PLANE STRESS OR PLANE STRAIN
NODAL MASS FRACTION CONCENTRATION OF
                                                                      CROSS-SECTIONAL AREA OF THE ROD
YOUNG'S MODULUS FOR THE SOLID
POISSON'S RATIO FOR THE SOLID
                                                                                                                                                                                                          DENSITY OF SOLID MIXTURE
                                                                                                                                                                                        REFERENCE CONCENTRATION
                                   NODAL DISPLACEMENT
                                                                                                                                                                                                                                                                                                      =int(PROPS(1))
                                                                                                                                                                                                                                                                                                                      =PROPS(2)
=PROPS(3)
                                                                                                                                               ALPHA
                                                                                                                                                                                                                                                               TRAPS
                                                      KFLAG
                                                                        AREA
                                                                                                                                                                                        CREF
                                                                                                                                                                                                         RHO
XMW
TEMP
                                                                                                                                                                                                                                                                                                      KFLAG
                                                                                                            XNU
                                                                                                                                                                                                                                                                                                                        AREA
```

=PROPS(4) =PROPS(5) =PROPS(6) =PROPS(7) =PROPS(10) =PROPS (11 =PROPS(8) = PROPS (9) ALPHA XNU

2-POINT GAUSSIAN INTEGRATION LOCATIONS AND WEIGHTS data (

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NON-DIMENSIONAL COEFFICIENT FOR (C*DE/DX) TERM OF (J=-DEFF*DC/DX + DEFF*XKON1*C*DE/DX) EFFECTIVE DIFFUSION COEFFICIENT GAS CONSTANT FOR SOLUTE SPECIES BULK MODULUS FOR THE SOLID MASS FLUX EQUATION R R H RS BULK XKON1 DEFF

000000000

= E/(3.0D0*(1.0D0-2.0D0*XNU)) = 3.0D0*BULK*ALPHA/(RHO*RS*TEMP) = D/(1.0D0+TRAPS) = 8.31432D0/XMWRS BULK XKON1

The nodal dof are stored in the array U (NDOF) in the UEL subroutine:

$$[U(1), U(2), U(3), U(4), U(5)] = [c_1, u_1, u_2, c_2, u_3]$$

This ordering is in agreement with standard ABAQUS and finite element conventions.

PROPERTIES for the element include material constants for the SOLID and pertinent SOLUTE deck and passed via the PROPS (NPROPS) array. Physical dimensions are specified here in SI data. These properties are specified in the "UEL PROPERTIES" option of the ABAQUS input units, but the program can be used with any dimensionally consistent system of units. KFLAG represents the only non-physical "property" specified and indicates the desired linear elastic and coupling case to be run.

3 ⇒ Plane Stress, Weakly Coupled KFLAG values: 1 ⇒ Plane Stress, Fully Coupled 2 ⇒ Plane Strain, Fully Coupled

(Uncoupled analysis is achieved by setting ALPHA=0 in the input deck) 4 ⇒ Plane Strain, Weakly Coupled

GAUSS POINTS are standard two-point Gaussian locations and weights, GW(i) = 1.0 $GR(1) = \pm \frac{1}{\sqrt{3}},$

DEFINITIONS of calculated constants for the model.

XMM RS=

 $\mathbb{R} \equiv \text{universal gas constant} = 8.31432 [J/mol.^{\circ}K]$

BULK MODULUS is calculated using the elastic modulus and Poisson's ratio; it is used to simplify the coefficients (B1, B2, B3) in the constitutive equation for the stress.

DEFF is the effective diffusion coefficient defined by:

$$D_{eff} \equiv \frac{D}{(1 + c_R^s K_R)}$$

```
B1=(1.0D0-XNU)*E/((1.0D0+XÑU)*(1.0D0-2.0D0*XNU))
B2=(3.0D0*BULK*ALPHA)*(1.0D0+TRAPS)
B3=(B2*CREF)
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                       B1 = (1.0D0 - XNU) *E/((1.0D0 + XNU) * (1.0D0 - 2.0D0 * XNU))
              CONSTANTS FOR PLANE STRESS OR PLANE STRAIN CASE
                                                                                                                                                                    A2=(2.0D0*(1.0D0+XNU)*ALPHA)*(1.0D0+TRAPS)
                                                                                                                                                                                                                            B2=(9.0D0*BULK*ALPHA)*(1.0D0+TRAPS)
              SET CONSTANTS FOR PLANE STRESS UK
1=> PLANE STRESS, FULLY COUPLED
3=> PLANE STRAIN, FULLY COUPLED
3=> PLANE STRESS, WEAKLY COUPLED
4=> PLANE STRAIN, WEAKLY COUPLED
                                                                                                                                 if(KFLAG.eq.1) then
A1=(1.0D0-2.0D0*XNU)
                                                                                                                                                                                                                                                                                                                                                                                                   elseif(KFLAG.eq.3) then
A1=(1.0D0-2.0D0*XNU)
                                                                                                                                                                                                                                                                 elseif(KFLAG.eq.2) then A1=1.0D0
                                                                                                                                                                                                                                                                                                                                                                                                                                    A2= 0.0D0
A3= 0.0D0
B1=(3.0D0*BULK)
B2= 0.0D0
B3= 0.0D0
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                 elseif(KFLAG.eq.4) then
                                                                                                                                                                                                           B1=(3.0D0*BULK)
                                                                                                                                                                                       A3=(A2*CREF)
                                                                                                                                                                                                                                               B3=(B2*CREF)
                                                                                                                                                                                                                                                                                                      A2=0.0D0
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                        A2=0.0D0
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                         A3=0.0D0
                                                                                                                                                                                                                                                                                                                         A3=0.0D0
0000000
```

CALCULATION of NODAL COORDINATES is necessary for calculating the Jacobian of the coordinate transformation for the deformed configuration. ABAQUS passes in the COORDS (MCRD, NNODE) array which contains the original global coordinates of the nodes on the element. The displacement at each node must then be added to the original coordinate to give the nodal position in the deformed configuration.

CALCULATION of GAUSS POINT COORDINATES in terms of the global coordinate system is necessary for later calculations of the distributed load functions and for storage in the state variable array. This is performed using the quadratic interpolation functions.

IACOBIAN AND DETERMINANT OF TRANSFORMATION for the 1-D model are both scalar functions of the local element coordinate, r. The product of the inverse Jacobian and the determinant of the Jacobian is unity, and this product is directly simplified wherever it occurs in order to reduce the number of computations.

CALCULATE INVERSE JACOBIAN AND DETERMINANT FOR ISOPARAMETRIC

FRANSFORMATION

0 0 0 0

DJ(I) = DETJ(GR(I),X(1),X(2),X(3))

continue

20

SET GLOBAL COORDINATES FOR GAUSS POINTS ON CURRENT ELEMENT

000

SET GLOBAL COORDINATES FOR NODES ON CURRENT ELEMENT

000

X(1) = COORDS(1,1) +U(2) X(2) = COORDS(1,2) +U(3) X(3) = COORDS(1,3) +U(5) The inverse Jacobian will be denoted as $[J^{-1}]$, or $1/J(r_k)$ if Gauss integration is indicated, and the determinant will be denoted by |J|.

CASE DETERMINATION is made possible by the fact that the hydrostatic strain and the stress for the 1-D case may be written using a set of defined coefficients.

$$e = A_1 \frac{\partial u}{\partial x} + A_2 c - A_3$$
$$\sigma = B_1 e - B_2 c + B_3$$

The values of A_i and B_i are assigned based on the value of KFLAG set in the input deck:

KFLAG = $1 \Rightarrow \text{Plane Stress, Fully Coupled}$

KFLAG = $2 \Rightarrow$ Plane Strain, Fully Coupled

KFLAG = $3 \Rightarrow$ Plane Stress, Weakly Coupled

KFLAG = 4 \Rightarrow Plane Strain, Weakly Coupled

(Uncoupled analysis is achieved by setting ALPHA=0 in the input deck)

```
0 0 0 0 0 0 0
```

```
CALCULATE de/dr USING VALUES FROM THE PREVIOUS TIME STEP
<<<<<<< CALCULATE CAPACITANCE MATRIX >>>>>>>
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                          <<<<<<< CONTROL CAPERTY CONTROL CAPERTY CONTROL CONTROL CONTROL CAPERTY CONTROL CAPERTY CONTROL CAPERTY CONTROL CAPERTY CAPERT
                                                                                                                                                                                                                                                                                    = AREA*(G1(GR(1))*G1(GR(1))*DJ(1)*GW(1)
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                   = AREA*(G1(GR(1))*G2(GR(1))*DJ(1)*GW(1)
+ G1(GR(2))*G2(GR(2))*DJ(2)*GW(2))
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                = AREA*(G2(GR(1))*G1(GR(1))*DJ(1)*GW(1)
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                 = AREA* (G2 (GR(1))*G2 (GR(1))*DJ(1)*GW(1)
+ G2 (GR(2))*G2 (GR(2))*DJ(2)*GW(2))
                                                                                                                                                                                                                                                                                                                                                                                                                                            + G1(GR(2))*G1(GR(2))*DJ(2)*GW(2))
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                             G2 (GR(2)) *G1 (GR(2)) *DJ(2) *GW(2))
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                          E1=SVARS(1)
E2=SVARS(2)
DE = (E2-E1)
                                                                                                                                                                                                                                                                                                  CAP (1,1)
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                 CAP(1,2)
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                CAP(2,1)
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                 CAP (2,2)
```

```
COEFFICIENTS OF CONCENTRATION D.O.F. IN MASS BALANCE EQUATION
                                                                                = AREA*DEFF*(XJI(1)*(DGI(GR(1))*DGI(GR(1))
-XKON1*DGI(GR(1))*DE*GI(GR(1)))*GW(1)
+XJI(2)*(DGI(GR(2))*DGI(GR(2))
                                                                                                                                                                                                                           = AREA*DEFF*(XJI(1)*(DG1(GR(1))*DG2(GR(1))
-XKON1*DG1(GR(1))*DE*G2(GR(1)))*GW(1)
+ XJI(2)*(DG1(GR(2))*DG2(GR(2))
                                                                                                                                                                                                                                                                                                                                                                           = AREA*DEFF*(XJI(1)*(DG2(GR(1))*DG1(GR(1))
-XKON1*DG2(GR(1))*DB*G1(GR(1)))*GW(1)
+ XJI(2)*(DG2(GR(2))*DG1(GR(2))
-XKON1*DG2(GR(2))*DE*G1(GR(2)))*GW(2))
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                              = AREA*DEFF*(XJI(1)*(DG2(GR(1))*DG2(GR(1))
                                                                                                                                                                       -XKON1 *DG1 (GR(2)) *DE*G1 (GR(2))) *GW(2))
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                     -XKON1*DG2 (GR(1))*DE*G2 (GR(1))*GW(1)
+ XJ1(2)*(DG2 (GR(2))*DG2 (GR(2))
-XKON1*DG2 (GR(2))*DE*G2 (GR(2))
                                                                                                                                                                                                                                                                                                                 -XKON1*DG1(GR(2))*DE*G2(GR(2)))*GW(2))
                                                                                                                                                                                                                                                                                                                                                                              XKC(2,1)
                                                                                                                                                                                                                                 XKC(1,2)
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                              XKC(2,2)
0 0 0
```

CHECK LFLAGS(1) for ABAQUS analysis type. The numerical steps are minimized by skipping the calculation of the capacitance matrix if the indicated analysis is steady-state.

if(LFLAGS(1).ne.71)then

000

CAPACITANCE MATRIX contains the coefficients of the $\partial c/\partial t$ term in the mass balance equation. Each term of the 2×2 matrix is calculated by numerical integration.

$$[C_c] \equiv A_e \int_{-1}^{+1} \{g_i(r)\} [g_i(r)] |J| dr \equiv \sum_{k=1}^{2} A_k W_k \bigg(\{g_i(r_k)\} [g_i(r_k)] J(r_k) \bigg)$$

END LFLAGS (1) CAPACITANCE MATRIX SKIP

STIFFNESS MATRIX [K] must be calculated for both transient and steady-state analyses.

The dilatational strain gradient $\partial e/\partial r$ is assumed constant across the length of an element and is approximated using the first order difference, $\Delta e/\Delta r$, using values of e from the previous time step. The local gradient, $\Delta e/\Delta r$, is calculated and then transformed to $\Delta e/\Delta x$ in the following calculations via multiplication by the inverse Jacobian. Stiffness Matrix $[K_{cu}]$ Calculations: Displacement coupling in the mass balance equation occurs through the $\partial e/\partial x$ term. The approximation of $\partial e/\partial x$, as described above, eliminates any $[K_{cu}]$ contribution to the total stiffness matrix [K]. That is, $[K_{cu}] = [0]$. Stiffness Matrix [Kc] Calculations: contributions from the concentration dof in the mass balance equation. Numerical integration is performed using a two-point Gaussian scheme:

$$[K_c] \equiv A_c \int_{-1}^{+1} \left[J^{-1} \right]^2 D_{eff} \left(\left\{ \frac{dg_i(\tau)}{d\tau} \right\} \left[\frac{dg_i(\tau)}{d\tau} \right] - K_1 \left\{ \frac{dg_i(\tau)}{d\tau} \right\} \frac{\partial e}{\partial \tau} \left[g_i(\tau) \right] \right) |J| d\tau$$

$$[K_c] = \sum_{k=1}^2 A_c W_k \left(D_{eff} \left(\left\{ \frac{dg_i(\tau_k)}{d\tau} \right\} \left| \frac{dg_i(\tau_k)}{d\tau} \right| - K_1 \left\{ \frac{dg_i(\tau_k)}{d\tau} \right\} \frac{\partial e}{\partial \tau} \left[g_i(\tau_k) \right] \right) \frac{1}{J(\tau_k)} \right)$$

= (E2-E1)/(GR(2)-GR(1))

COEFFICIENTS OF CONCENTRATION IN LINEAR BLASTIC EQUATION

XKUC(1,1) = AREA*(B1*A2-B2)*(DH1(GR(1))*G1(GR(1))*GW(1)
1
XKUC(1,2) = AREA*(B1*A2-B2)*(DH1(GR(1))*G2(GR(1))*GW(2))
1
XKUC(2,1) = AREA*(B1*A2-B2)*(DH2(GR(1))*G1(GR(1))*GW(1))
XKUC(2,2) = AREA*(B1*A2-B2)*(DH2(GR(1))*G1(GR(1))*GW(2))
XKUC(3,2) = AREA*(B1*A2-B2)*(DH2(GR(1))*G2(GR(1))*GW(2))
1
XKUC(3,1) = AREA*(B1*A2-B2)*(DH2(GR(1))*G2(GR(1))*GW(2))
1
XKUC(3,2) = AREA*(B1*A2-B2)*(DH3(GR(1))*G1(GR(1))*GW(2))
1
XKUC(3,2) = AREA*(B1*A2-B2)*(DH3(GR(1))*G1(GR(1))*GW(2))
1
XKUC(3,2) = AREA*(B1*A2-B2)*(DH3(GR(2))*GW(2))
1
XKUC(3,2) = AREA*(B1*A2-B2)*(DH3(GR(2))*G2(GR(1))*GW(2))
1
XKUC(3,2) = AREA*(B1*A2-B2)*(DH3(GR(2))*G2(GR(2))*GW(2))
1

Stiffness Matrix $[K_{uc}]$ Calculations: contributions from the concentration dof in the linear momentum balance equation. Numerical integration is performed using a two-point Gaussian scheme:

$$[K_{uc}] \equiv A_e \int_{-1}^{+1} (B_1 A_2 - B_2) [J^{-1}] \left\{ \frac{dh_j(r)}{dr} \right\} [g_i(r)] |J| dr$$
$$[K_{uc}] = \sum_{k=1}^{2} A_e W_k \left((B_1 A_2 - B_2) \left\{ \frac{dh_j(r_k)}{dr} \right\} [g_i(r_k)] \right)$$

Stiffness Matrix [K_u]Calculations: contributions from the displacement dof in the linear momentum balance equation. Numerical integration is performed using a two-point Gaussian scheme:

$$[K_{u}] \equiv A_{e} \int_{-1}^{+1} (B_{1}A_{1}) [J^{-1}]^{2} \left\{ \frac{dh_{j}(r)}{dr} \right\} \left[\frac{dh_{j}(r)}{dr} \right] |J| J |dr$$

$$[K_{u}] = \sum_{k=1}^{2} A_{e} W_{k} \left((B_{1}A_{1}) \left\{ \frac{dh_{j}(r_{k})}{dr} \right\} \left[\frac{dh_{j}(r_{k})}{dr} \right] \frac{1}{J(r_{k})} \right) \right.$$

 $\circ \circ \circ$

WHERE [K] IS [XKC(1,1) 0 0 0 XKC(1,2) 0] [XKUC(1,1) XKU(1,1) XKU(1,2) XKUC(1,2) XKU(1,3)] [XKUC(2,1) XKU(2,1) XKU(2,2) XKUC(2,2) XKU(1,3)] [XKC(2,1,1) 0 0 XKC(2,2) 0] [XKUC(3,1,1) XKU(3,1) XKU(3,2) XKUC(3,2) XKU(3,3)] <<<<< CALCULATE AMATRX = [K] >>>>>>> if (LFLAGS(3).eq.1.or.LFLAGS(3).eq.2) then if(LFLAGS(1).eq.71) then

00000000

0 0 0 0

U

AMATRX(1,1) = XKC(1,1)
AMATRX(2,1) = XKC(1,2)
AMATRX(2,2) = XKU(1,1)
AMATRX(2,3) = XKU(1,2)
AMATRX(2,4) = XKU(1,2)
AMATRX(3,1) = XKU(1,2)
AMATRX(3,1) = XKU(1,3)
AMATRX(3,2) = XKU(2,1)
AMATRX(3,2) = XKU(2,1)
AMATRX(3,4) = XKU(2,2)
AMATRX(4,4) = XKU(2,3)
AMATRX(4,4) = XKU(2,3)
AMATRX(4,4) = XKU(2,3)
AMATRX(5,2) = XKU(3,3)
AMATRX(5,2) = XKU(3,1)
AMATRX(5,2) = XKU(3,2)
AMATRX(5,3) = XKU(3,2)
AMATRX(5,4) = XKU(3,2)
AMATRX(5,5) = XKU(3,3)
AMATRX(5,5) = XKU(3,3)

endif

if((LFLAGS(3).eq.1).or.(LFLAGS(3).eq.5)) then

O O

CHECK LFLAGS(1) for ABAQUS analysis type. If the analysis type is steady-state coupled deformation/ heat transfer, then assign the steady state matrices.

STEADY STATE MATRIX ASSIGNMENTS

CHECK LFLAGS(3) for ABAQUS information request. If needed, assign AMATRX for return to main program. AMATRX ASSIGNMENT for steady state analysis is a direct assignment of the previously calculated stiffness matrix components:

$$AMATRX = \begin{bmatrix} K_{11}^{11} & 0 & 0 & K_{12}^{12} & 0 \\ K_{11}^{11} & K_{11}^{11} & K_{12}^{12} & K_{12}^{12} & K_{13}^{13} \\ K_{21}^{21} & K_{21}^{21} & K_{22}^{22} & K_{22}^{23} & K_{23}^{23} \\ K_{21}^{21} & 0 & 0 & K_{22}^{22} & 0 \\ K_{31}^{31} & K_{31}^{31} & K_{32}^{32} & K_{33}^{33} & K_{33}^{33} \end{bmatrix}$$

where, for example, K_c^{21} represents the term in the second row, first column of the K_c matrix.

END LFLAGS (3) AMATRX ASSIGNMENT

CHECK LFLAGS (3) for ABAQUS information request. If needed, assign RHS array for return to main program.

0000000000000000

<-<<< CALCULATE (RHS) = {R}-[K]{U} >>>>>>>

NEUMANN BOUNDARY DATA ARE HOMOGENEOUS (1.e., ZERO). INHOMOGENEOUS (NON-ZERO) NEUMANN BOUNDARY DATA SUCH AS NON-ZERO BOUNDARY TRACTIONS AND/OR SOLUTE FLUXES ARE AUTOMATICALLY INCORPORATED INTO THE COMPUTATIONS BY ABAQUE. INHOMOGENEOUS DATA SHOULD BE ENTERED IN THE INPUT DECK AS CONCENTRATED NODAL FORCES. THIS USER ELEMENT SUBROUTINE ASSUMES THAT ALL OF NOTE:

THE FOLLOWING SECTION CALCULATES THE TOTAL USER SUPPLIED DISTRIBUTED LOADS (e.g. BODY FORCES) AT THE GAUSS POINTS ON THE ROD FOR USE IN FINDING THE RHS VECTOR.

FNU(I) = 0.0D0do 31 I=1,2 F(I) = 0.0D0FUN=0.0D0 continue 31

FUN=FUN+ADLMAG(K1,1)
elseif(JDLTYP(K1,1).lt.0)then
call KFCALC(JDLTYP(K1,1),F,XGAUS,TIME(2)) if(JDLTYP(K1,1).gt.0)then do 51 I=1,2 FNU(I)=FNU(I)+F(I) do 41 K1=1, MDLOAD

continue

endif 51 41

continue

FNU(I)=FNU(I)+FUN continue 61

CALCULATION OF THE RIGHT-HAND SIDE (RHS) VECTOR

DISTRIBUTED LOADS must be converted to consistent nodal loads for use in calculation of the {R_u} matrix. ABAQUS recognizes two types of distributed loads:

- 1. those with constant magnitude across the element. The magnitude is specified in the array ADLMAG and the variable JDLTYP > 0;
 - 2. those with magnitude specified in the UEL subroutine as a function of the global coordinate, x, and time, t. The variable JDLTYP < 0.

ZERO THE ARRAYS which will contain the values of the total loads at the Gauss points.

COMBINED TOTAL OF JDLTYP > 0 AND JDLTYP < 0 DISTRIBUTED LOADS: is given by the integer variable, MDLOAD. TYPE JDLTYP > 0 DISTRIBUTED LOADS: The total magnitude of type JDLTYP > 0 loads is found by summation.

is found by summing magnitudes at each Gauss point location. The subroutine KFCALC is TYPE JDLTYP < 0 DISTRIBUTED LOADS: The total magnitude of type JDLTYP < 0 loads called to determine the value of the load at each Gauss point. See the notes for subroutine KFCALC for additional explanation.

JDLTYP > 0 loads to the total JDLTYP < 0 types loads. This is the load value that needs to be TOTAL DISTRIBUTED LOAD at each Gauss point is determined by adding the total used in the calculations of $\{R_u\}$.

interpolation function and the distributed load over the element. The total distributed load at CONSISTENT NODAL LOADS are calculated by integrating the product of the nodal each Gauss point is required for the Gaussian integration.

```
- (B3-B1*A3) *XJT ((1)*DH2 (GR(1)))*DJ(1)*GW(1)
+ (H2 (GR(2)) *FNU(2)
- (B3-B1*A3) *XJT (2)*DH2 (GR(2)))*DJ(2)*GW(2))
                                                                                                                                                                                                                                                                                                                                                     -(B3-B1*A3)*XJI(2)*DH3(GR(2)))*DJ(2)*GW(2))
                                                                                                                -(B3-B1*A3) *XJI(2) *DH1(GR(2))) *DJ(2) *GW(2))
                                                                                                                                                                                                                                                                                            R(5) = AREA*((H3(GR(1))*FNU(1)
-(B3-B1*A3)*XJI(1)*DH3(GR(1)))*DJ(1)*GW(1)
+(H3(GR(2))*FNU(2)
                                                                          - (B3-B1*A3) *XJI(1) *DH1(GR(1))) *DJ(1) *GW(1)
+ (H1(GR(2)) *FNU(2)
                                                     R(2) = AREA*((H1(GR(1))*FNU(1)
                                                                                                                                                        AREA*((H2(GR(1))*FNU(1)
                 R(1) = 0.000
                                                                                                                                                        R(3) =
                                                                                                                                                                                                                                                        R(4)
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```
RHS(1,1) = R(1) -XKC(1,1)*U(1)-XKC(1,2)*U(4)

RHS(2,1) = R(2) -XKUC(1,1)*U(1)-XKU(1,1)*U(2)-XKU(1,2)*U(3)

LXKUC(1,2)*U(4)-XKU(1,3)*U(5)

RHS(3,1) = R(3) -XKUC(2,1)*U(1)-XKU(2,1)*U(5)-XKU(2,2)*U(3)

RHS(4,1) = R(4) -XKC(2,1)*U(1)-XKC(2,2)*U(4)

RHS(4,1) = R(4) -XKC(2,1)*U(1)-XKC(2,2)*U(4)

RHS(5,1) = R(5) -XKUC(3,1)*U(1)-XKU(3,1)*U(2)-XKU(3,2)*U(3)

1 -XKUC(3,2)*U(4)-XKU(3,3)*U(5)
```

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endif

CALCULATION of $\{\mathcal{R}\}$: The array R (NDOF) is determined using the column matrices $\{R_c\}$ and $\{R_{u_1}\}$ through the equation:

$$\mathbf{R}(i) {=} {\left[\begin{array}{cccc} R_{c}^{1} & R_{c}^{1} & R_{c}^{2} & R_{c}^{2} & R_{c}^{3} \end{array} \right]}^{\dagger}$$

where: $\{R_c\} \equiv -A_e \mathcal{J}_L^* \left\{g_i(r)\right\}_{-1}^{+1} \\ \left\{R_a\right\} \equiv A_e \mathcal{J}_{-1}^{+1} \left(\{h_j(r)\}F_x - (B_3 - B_1 A_3)[J^{-1}]\left\{\frac{dh_j(r)}{dr}\right\}\right) |J| dr + A_e \sigma_x \left\{h_j(r)\right\}_{-1}^{+1}$

The code does not incorporate the inhomogeneous Neumann boundary data included in the $\{R_c\}$ and $\{R_u\}$ expressions into the code. These are added directly by ABAQUS using the concentrated nodal load data specified in the input deck under the CLOAD option. As a result:

$$\{R_e\} = \sum_{k=1}^{2} W_k \left(\{h_j(r_k)\} F_x - \frac{B_3 - B_1 A_3}{J(r_k)} \left\{ \frac{dh_j(r_k)}{dr} \right\} \right) A_e |J(r_k)|$$

The quantity $A_e \mathcal{J}_{L-1}^*|_{-1}^{+1}$ is proportional to the net solute flow out of the element. Actual mass flow rates are given by $\rho(1+c_R^*K_R)\mathcal{J}_L^*$ in $[mass\ Solute/(area\times time)]$ units. The quantity $A_e \ \sigma_x$ represents a boundary force.

CALCULATION OF RHS (NDOF) ARRAY requires matrix multiplication, since

$$\{RHS\}=\{\mathcal{R}\}-[\mathcal{K}]\{U\}$$

where U(NDOF) is the vector of nodal concentrations and displacements from the previous iteration, and $[\mathcal{K}]$ is the element stiffness matrix (same form as AMATRX above).

END LFLAGS (3) RHS ASSIGNMENT

END LFLAGS(1) STEADY STATE MATRIX ASSIGNMENTS

elseif((LFLAGS(1).eq.72).or.(LFLAGS(1).eq.73)) then

0 0 0 0 0

if (LFLAGS(3).eq.1.or.LFLAGS(3).eq.2) then

AND [C] IS [CAP(1,1) 0 0 CAP(1,2) 0] [0 0 0 0] [0 0 0 0] [CAP(2,1) 0 0 CAP(2,2) 0] [0 0 0 0]

AMATRX (1,1) = XKC (1,1) + CAP (1,1) /DTIME
AMATRX (2,2) = XKC (1,1)
AMATRX (2,2) = XKU (1,1)
AMATRX (2,3) = XKU (1,1)
AMATRX (2,3) = XKU (1,2)
AMATRX (2,4) = XKU (1,2)
AMATRX (2,4) = XKU (1,2)
AMATRX (3,1) = XKU (1,2)
AMATRX (3,1) = XKU (2,1)
AMATRX (3,1) = XKU (2,1)
AMATRX (3,2) = XKU (2,2)
AMATRX (3,4) = XKU (2,3)
AMATRX (3,4) = XKU (2,3)
AMATRX (4,4) = XKU (2,3)
AMATRX (5,1) = XKU (3,1)
AMATRX (5,3) = XKU (3,1)
AMATRX (5,4) = XKU (3,2)
AMATRX (5,4) = XKU (3,3)
AMATRX (5,5) = XKU (3,3)

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endif

CHECK LFLAGS(1) for ABAQUS analysis type. If the analysis type is transient coupled deformation/heat transfer, then assign the TRANSIENT matrices.

TRANSIENT MATRIX ASSIGNMENTS

CHECK LFLAGS (3) for ABAQUS information request. If needed, assign AMATRX for return to main program.

AMATRX ASSIGNMENT for transient analysis requires combining the previously calculated stiffness and capacitance matrix components such that:

$$\text{AMATRX} = \begin{bmatrix} K_c^{11} + \frac{1}{4}, C_c^{11} & 0 & 0 & K_c^{12} + \frac{1}{4}, C_c^{12} & 0 \\ K_u^{11} & K_u^{11} & K_u^{12} & K_u^{12} & K_u^{13} \\ K_u^{21} & K_u^{21} & K_u^{21} & K_u^{22} & K_u^{22} \\ K_u^{21} + L_c^{21} & 0 & 0 & K_c^{12} + \frac{1}{4}, C_c^{22} & 0 \\ K_u^{31} & K_u^{31} & K_u^{31} & K_u^{32} & K_u^{32} \end{bmatrix}$$

where, for example, K_c^{21} represents the term in the second row, first column of the K_c matrix. The presence of capacitance terms distinguish this array from the steady state version of AMATRX.

END LFLAGS (3) AMATRX ASSIGNMENT

<><<< CALCULATE (RHS) = (R)-[K](U]-([C]/DTIME)(DU)>>>>> THE FOLLOWING SECTION CALCULATES THE TOTAL USER SUPPLIED DISTRIBUTED LOADS (e.g. BODY FORCES) AT THE GAUSS POINTS ON THE ROD FOR USE IN FINDING THE RHS VECTOR. call KFCALC(JDLYYP(K1,1),F,XGAUS,TIME(2))
do 151 I=1,2
FNU(I)=FNU(I)+F(I) if((LFLAGS(3).eq.1).or.(LFLAGS(3).eq.5)) then do 141 Kl=1,MDLOAD
 if(JDLTYP(Kl,1).gt.0)then
 FUN=FUN+ADLMAG(Kl,1)
 elseif(JDLTYP(Kl,1).lt.0)then FNU(I)=FNU(I)+FUN FUN=0.0D0 do 131 I=1,2 F(I) = 0.0D0 FNU(I) = 0.0D0 continue do 161 I=1,2 endif continue continue continue 131 141 161 151 C $\circ \circ \circ \circ \circ \circ \circ$

CHECK LFLAGS (3) for ABAQUS information request. If needed, assign RHS values.

CALCULATION OF THE RIGHT-HAND SIDE (RHS) VECTOR

<u>DISTRIBUTED LOADS</u> must be converted to consistent nodal loads for use in calculation of the $\{R_u\}$ matrix. ABAQUS recognizes two types of distributed loads:

- 1. those with constant magnitude across the element. The magnitude is specified in the array ADLMAG and the variable JDLTYP > 0;
 - **2.** those with magnitude specified in the UEL subroutine as a function of the global coordinate, x, and time, t. The variable JDLTYP < 0.

ZERO THE ARRAYS which will contain the values of the total loads at the Gauss points.

COMBINED TOTAL OF JULTYP > 0 AND JULTYP < 0 DISTRIBUTED LOADS: is given by the integer variable, MDLOAD.

<u>TYPE JDLTYP > 0 DISTRIBUTED LOADS</u>: The total magnitude of type JDLTYP > 0 loads is found by summation.

TYPE JDLTYP < 0 DISTRIBUTED LOADS: The total magnitude of type JDLTYP < 0 loads is found by summing magnitudes at each Gauss point location. The subroutine KFCALC is called to determine the value of the load at each Gauss point. See the notes for subroutine KFCALC for additional explanation.

TOTAL DISTRIBUTED LOAD at each Gauss point is determined by adding the total JDLTYP > 0 loads to the total JDLTYP < 0 types loads. This is the load value that needs to be used in the calculations of $\{R_a\}$.

CONSISTENT NODAL LOADS are calculated by integrating the product of the nodal interpolation function and the distributed load over the element. The total distributed load at each Gauss point is required for the Gaussian integration.

C

```
RHS(1,1) = R(1) -XKC(1,1)*U(1)-XKC(1,2)*U(4)
-1.0D0/DTIME*(CAP(1,1)*DU(1,1)+CAP(1,2)*DU(4,1))

RHS(2,1) = R(2) -XKUC(1,1)*U(1)-XKU(1,1)*U(2)-XKU(1,2)*U(3)
-XKUC(1,2)*U(4)-XKU(2,1)*U(5)-XKU(2,2)*U(3)

RHS(3,1) = R(3) -XKUC(2,2)*U(4)-XKU(2,1)*U(5)
-XKUC(2,2)*U(4)-XKU(2,3)*U(5)

RHS(4,1) = R(4) -XKC(2,1)*U(1)-XKC(2,2)*U(4)

1 -XKUC(2,1)*U(1)-XKC(2,2)*U(4)
-1.0D0/DTIME*(CAP(2,1)*DU(1,1)+CAP(2,2)*DU(4,1))

RHS(5,1) = R(5) -XKUC(3,1)*U(1)-XKU(3,1)*U(2)-XKU(3,2)*U(3)
-XKUC(3,2)*U(4)-XKU(3,3)*U(5)
```

endif

<<<<< CHECK THAT PROCEDURE FLAG SENT MATCHES PROCEDURE PERFORMED >>>
elseif((LFLAGS(1).ne.71).and.
1 (LFLAGS(1).ne.72).and.(LFLAGS(1).ne.73)) then
write(*,*) 'ERROR IN THE VALUE OF LFLAGS(1) = ', LFLAGS(1)
end; f.

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CALCULATION of $\{\mathcal{R}\}$: The array R (NDOF) is determined using the column matrices $\{R_c\}$ and $\{R_u\}$ through the equation:

where:
$$\{R_e\} \equiv -A_e \, \mathcal{J}_L^* \, \{g_l(\tau)\}\Big|_{-1}^{+1} \\ \{R_u\} \equiv A_e \, \int_{-1}^{+1} \left(\{h_j(\tau)\}F_x \, - (B_3 - B_1 A_3)[\, J^{-1}] \left\{\frac{dh_j(\tau)}{d\tau}\right\}\right) |\, J\, |d\tau \, + A_e \, \sigma_x \, \{h_j(\tau)\}\Big|_{-1}^{+1}$$

The code does not incorporate the inhomogeneous Neumann boundary data included in the $\{R_c\}$ and $\{R_u\}$ expressions into the code. These are added directly by ABAQUS using the concentrated nodal load data specified in the input deck under the CLOAD option. As a result:

$$\{R_{\epsilon}\} = \{0\}$$

$$\{R_{\epsilon}\} = \sum_{k=1}^{2} W_{k} \left(\{h_{j}(r_{k})\} F_{x} - \frac{B_{3} - B_{1} A_{3}}{J(r_{k})} \left\{ \frac{dh_{j}(r_{k})}{dr} \right\} \right) A_{\epsilon} |J(r_{k})|$$

The quantity $A_{\epsilon} \mathcal{J}_{\ell}^{*} \Big|_{-1}^{+1}$ is proportional to the net solute flow out of the element. Actual mass flow rates are given by $\rho (1 + c_{R}^{*}K_{R}) \mathcal{J}_{L}^{*}$ in $[mass\ Solute/(area imes time)]$ units.

The quantity $A_e \sigma_x$ represents a boundary force.

CALCULATION OF RHS (NDOF) ARRAY requires matrix multiplication, since

$$\{\text{RHS}\} = \{\mathcal{R}\} - [\mathcal{K}]\{\mathbf{U}\} - \frac{1}{\Delta t}[\mathcal{C}]\{\Delta\mathbf{U}\}$$

where U(NDOF) is the vector of nodal concentrations and displacements from the previous iteration, $[\mathcal{K}]$ is the element stiffness matrix, and $[\mathcal{C}]$ is the element capacitance matrix. $\{\Delta U\}$ represents the ABAQUS array DU which is defined as the *change* in the nodal values from the previous increment.

END LFLAGS(3) RHS ASSIGNMENT END LFLAGS(1) TRANSIENT MATRIX ASSIGNMENTS

CHECK LFLAGS (1) for ABAQUS analysis type. If the analysis type is neither steady-state nor transient coupled deformation/heat transfer, then set-off an error message indicating that this user element subroutine is incapable of performing the requested analysis.

CHECK LFILAGS (3) for ABAQUS information request. Update the state variables.

DILATATIONAL STRAIN is the only state variable actually used in the finite element calculations. All other state variables are calculated for output purposes only. Dilatational strain is calculated at the Gauss points for accuracy, using the equation:

$$e = A_1 \left| \frac{dH_j(x)}{dx} \right| \left\{ u_j(t) \right\} + A_2 \left[G_i(x) \right] \left\{ c_i(t) \right\} - A_3$$

AXIAL STRESS is calculated at the Gauss points for accuracy, using the equation:

$$\sigma_x = B_1 e - B_2 [G_i(x)] \{c_i(t)\} + B_3$$

AXIAL STRESS at the LHS element boundary node is calculated by replacing e in the above equation with the value:

$$e(r = -1) = e(r_1) - \left(\frac{\Delta e}{\Delta r}\right)(r_1 + 1)$$

and then evaluating $[G_i(x)]$ at the element boundary. A similar calculation scheme is used for the RHS nodal stress.

SOLUTE MASS FLUX is calculated at the center of the element from the equation:

$$\mathcal{J}_L = -D \left[\frac{dG_i(x)}{dx} \right] \left\{ c_i(t) \right\} + D \left. K_1 \left[G_i(x) \right] \left\{ c_i(t) \right\} \frac{\partial e}{\partial x}$$

GAUSS POINT LOCATIONS are calculated and printed with the Gauss point state variable data. This calculation is performed at the beginning of subroutine UEL using a quadratic shape function approximation in the axial coordinate, x.

END OF THE MAIN PART OF SUBROUTINE UEL

endif return

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return

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double precision function DETJ(R,X1,X2,X3) double precision R,X1,X2,X3 CALCULATE DETERMINANT OF JACOBIAN OF ISOPARAMETRIC TRANSFORMATION (DR=DETJ*DX) DETJ=(R-1.0D0/2.0D0)*X1-2.0D0*R*X2+(R+1.0D0/2.0D0)*X3 return

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LINEAR INTERPOLATION FUNCTIONS FOR CONCENTRATION

0 0 0

double precision function G1(R) double precision R G1=(1.0D0/2.0D0)*(1.0D0-R)

return end double precision function G2(R) double precision R G2=(1.0D0/2.0D0) *(1.0D0+R) return

υv

ISOPARAMETRIC TRANSFORM JACOBIAN between the global coordinate, x, and the local coordinate, r is given by:

$$[\ J\] \equiv \left[rac{dx}{dr}
ight] = \left[rac{dh_j(r)}{dr}
ight]\{x_j(t)\}$$

In the present situation, the Jacobian is a scalar function of the local coordinate, r, equal to:

$$J = \left(r - \frac{1}{2}\right)x_1 - (2\tau)x_2 + \left(r + \frac{1}{2}\right)x_3$$
$$[J^{-1}] = \frac{1}{J}$$

DETERMINANT of the JACOBIAN is a scalar function of the local coordinate, r, and is equal to J:

$$|J| = J$$

LINEAR SUPERPARAMETRIC INTERPOLATION FUNCTIONS are defined for the concentration:

$$g_1(r) \equiv \frac{1}{2}(1-r)$$
$$g_2(r) \equiv \frac{1}{2}(1+r)$$

QUADRATIC INTERPOLATION FUNCTIONS FOR DISPLACEMENT

O O

double precision function H1(R) double precision R H1=1.0D0/2.0D0*(R**2-R)

double precision function H2(R) double precision R H2=1.0D0-R**2

O

return

double precision function H3(R) double precision R H3=1.0D0/2.0D0*(R**2+R) return

O

DERIVATIVES OF LINEAR INTERPOLATIONS FOR CONCENTRATION

000

double precision function DG1(R) double precision R DG1=(-1.0D0/2.0D0)

return

double precision function DG2(R) double precision R DG2=(1.0D0/2.0D0)

DERIVATIVES OF QUADRATIC INTERPOLATIONS FOR DISPLACEMENT

000

double precision function DH1(R) double precision R DH1=(R-1.0D0/2.0D0) return double precision function DH2(R) double precision R DH2=-2.0D0*R return

U

double precision function DH3(R) DH3=(R+1.0D0/2.0D0) double precision R

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QUADRATIC ISOPARAMETRIC INTERPOLATION FUNCTIONS are defined for the displacement:

$$h_1(r) \equiv \frac{1}{2} (r^2 - r)$$
 $h_2(r) \equiv (1 - r^2)$
 $h_3(r) \equiv \frac{1}{2} (r^2 + r)$

INTERPOLATION FUNCTION DERIVATIVES

FOR THE LINEAR INTERPOLATION FUNCTIONS: these are used to interpolate the concentration derivatives; multiplication by the inverse Jacobian converts the local derivatives into global derivatives:

$$\frac{dg_1(r)}{dr} \equiv -\frac{1}{2}$$
$$\frac{dg_2(r)}{dr} \equiv +\frac{1}{2}$$

These interpolation function derivatives do not depend explicitly on the local coordinate, r, but function definitions are maintained as a reminder of the implied dependence on local coordinate. FOR THE QUADRATIC INTERPOLATION FUNCTIONS: these are used to interpolate displacement derivatives; multiplication by the inverse Jacobian converts the local derivatives into global derivatives:

$$\frac{dh_1(r)}{dr} \equiv \left(r - \frac{1}{2}\right)$$
$$\frac{dh_2(r)}{dr} \equiv -2r$$
$$\frac{dh_3(r)}{dr} \equiv \left(r + \frac{1}{2}\right)$$

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Gravitational body force for 4340 STEEL IS ${\sim}80.0D3~[{\rm N/m^3}]$ 'a. SHOULD BE SPECIFIED IN UNITS OF [FORCE/LENGTH^3/2] LOAD TYPE 1 INDICATES A DISTRIBUTED BODY FORCE OF THE FORM THIS LOAD ROUTINE CALCULATES THE BODY FORCE AT THE GAUSS POINTS FOR USE IN FINDING THE RHS VECTOR F(I)=A/sgrt((abs(XGAUS(I))+EPS)**3.0D0) subroutine KFCALC(JDLTYP, F, XGAUS, TTIME) DISTRIBUTED LOAD SPECIFICATION ROUTINE double precision F(*), XGAUS(*), TTIME $F(X) = A/(X + EPS)^3/2$ double precision A, EPS IF (JDLTYP.EQ.-1) THEN EPS=1.0D-18 DO 100 I=1,2 A=15.0D6 100 υo 00000000

return end

ENDIF

DISTRIBUTED LOAD SPECIFICATION ROUTINE returns a load value at the global Gauss point locations from an user-defined non-uniform load distribution. The coding in the present case is for a singular body force applied as a function of the global coordinate x. The value of the load at the *i*th Gauss point is returned to UEL as F(i). The subroutine receives the current distributed load type, JDLTYP, the global Gauss point coordinates, and the total time completed in the analysis.

Several different non-uniformly distributed load functions may be evaluated by adding IF statements to flag on the values of JDLTYP. These loads may be functions of the global location coordinate, x, and current analysis time, t. If additional load functions are to be added, they should follow the format of the load function given here, so that one load value is returned for each Gauss point on the element.

In steady-state analyses, a "time" is defined and used to indicate the fraction of analysis

completed. This allows for the gradual application of a distributed load in order to avoid potential numerical difficulties. See TIME in AUM (II; 5.2.16-7) for additional information.

NOTE ON CONSISTENT NODAL LOADINGS. UEL converts the distributed loads to consistent nodal loads by integration of the product of the quadratic displacement interpolation function and the distributed load over the length of the element. For example, on the first node, the consistent nodal load, P, would be

$$P = A_e \int_{-1}^{+1} \{h_1(r)\} F_x(r) |J| dr$$

which when evaluated via Gaussian integration gives:

$$P = W_1 A_{\epsilon} \{h_1(r_1)\} F_x(r_1) |J(r_1)| + W_2 A_{\epsilon} \{h_1(r_2)\} F_x(r_2) |J(r_2)|$$

where r_i and W_i are the Gauss locations and weights respectively. Since subroutine KFCALC returns the values of the load function at the Gauss points, $F_x(r_i)$, this evaluation becomes trivial.

APPENDIX C: Sample Input Deck

This Appendix contains a copies of the ABAQUS input decks that were used to solve the Case 1a, 2a, and 3a problems. A section-by-section annotated listing is also included.

ABAQUS Input Deck: Case 1a

```
*HEADING, UNSYMM
**
       C(0) = 0.0
                                     C(L) = 1E - 6
* *
      //----- F(x)=0
* *
++
       U(0) = 0.0
*PREPRINT
*NODE, NSET=ROD
1,0.0D0
21,0.10D0
*NGEN, NSET=ROD
1.21.1
*USER ELEMENT, NODES=3,UNSYMM,COORDINATES=1,PROPERTIES=11,VARIABLES=9,TYPE=U1
** THE FIRST AND THIRD NODES HAVE DEGREES OF FREEDOM IN CONCENTRATION AND
** DISPLACMENT, WHILE THE SECOND NODE ONLY HAS DISPLACEMENT DEGREES OF
** FREEDOM. NOTE THAT CONCENTRATIONS ARE TREATED AS TEMPERATURE D.O.F.
11,1
2,1
3,11,1
*ELEMENT, TYPE=U1, ELSET=SC1D
1,1,2,3
*ELGEN, ELSET≈SC1D
1,10,2,1,1,1,1,1
*UEL PROPERTY, ELSET=SC1D
** PROPERTIES ARE IN THE ORDER:
** CFLAG, AREA, E, NU, D, ALPHA, CREF, RHO,
** MW, TEMP, TRAPS
1.0D0, 1.0D-4, 200.0D9, 0.3D0, 1.0D-9, 5.2D0, 0.0D0, 7.8D3,
1.00797D-3,293.0D0,19.0D0
*MATERIAL, NAME=A4340
*DEPVAR
*BOUNDARY
** ZERO DISPLACEMENT AT LHS
1,1
** ZERO CONCENTRATION AT LHS
1,11
*INITIAL CONDITIONS, TYPE=TEMPERATURE
** 0.0 INITIAL CONCENTRATION OVER ROD LENGTH
ROD, 0.0D0
*WAVEFRONT MINIMIZATION, SUPPRESS
*STEP, INC=1000
*COUPLED TEMPERATURE-DISPLACEMENT, STEADY STATE
0.005,1.0, ,0.05
*CONTROLS, PARAMETERS=FIELD, FIELD=TEMPERATURE
, ,5.0D-21
*NODE PRINT, SUMMARY=NO, FREQUENCY=100
COOR1,U1,NT
*EL PRINT, SUMMARY=NO, FREQUENCY=100
SDV
*BOUNDARY
** SPECIFY RIGHT HAND BOUNDARY CONCENTRATION AS 1.0E-6
21,11,11,1.0D-6
*END STEP
```

ABAQUS Input Deck: Case 2a

```
*HEADING, UNSYMM
STATIC DIFFUSION -- STRONGLY COUPLED Standard Case 2
                                       C(L) = 1E - 6
       C(0) = 0.0
**
                                             11
      //----// F(x)=0
**
**
      11
                                               11
                                       U(L) = 0.0
       U(0) = 0.0
**
*PREPRINT
*NODE, NSET=ROD
1,0.0D0
21,0.10D0
*NGEN, NSET=ROD
1,21,1
*USER ELEMENT, NODES=3, UNSYMM, COORDINATES=1, PROPERTIES=11, VARIABLES=9, TYPE=U1
** THE FIRST AND THIRD NODES HAVE DEGREES OF FREEDOM IN CONCENTRATION AND
** DISPLACMENT, WHILE THE SECOND NODE ONLY HAS DISPLACEMENT DEGREES OF
** FREEDOM. NOTE THAT CONCENTRATIONS ARE TREATED AS TEMPERATURE D.O.F.
11,1
2,1
3,11,1
*ELEMENT, TYPE=U1, ELSET=SC1D
1,1,2,3
*ELGEN, ELSET=SC1D
1,10,2,1,1,1,1,1
*UEL PROPERTY, ELSET=SC1D
** PROPERTIES ARE IN THE ORDER:
** CFLAG, AREA, E, NU, D, ALPHA, CREF, RHO
** MW, TEMP, TRAPS
1.0D0, 1.0D-4, 200.0D9, 0.3D0, 1.0D-9, 5.2D0, 0.0D0, 7.8D3,
1.00797D-3,293.0D0,19.0D0
*MATERIAL, NAME=A4340
*DEPVAR
*BOUNDARY
** ZERO DISPLACEMENT AT LHS
1,1
** ZERO DISPLACEMENT AT RHS
21,1
** ZERO CONCENTRATION AT LHS
1,11
*INITIAL CONDITIONS, TYPE=TEMPERATURE
** 0.0 INITIAL CONCENTRATION OVER ROD LENGTH
ROD, 0.0D0
*WAVEFRONT MINIMIZATION, SUPPRESS
*STEP, INC=1000
*COUPLED TEMPERATURE-DISPLACEMENT, STEADY STATE
0.005,1.0, ,0.05
*CONTROLS, PARAMETERS=FIELD, FIELD=TEMPERATURE
, ,5.0D-21
*NODE PRINT, SUMMARY=NO, FREQUENCY=100
COOR1,U1,NT
*EL PRINT, SUMMARY=NO, FREQUENCY=100
*BOUNDARY
** SPECIFY RIGHT HAND BOUNDARY CONCENTRATION AS 1.0E-6
21,11,11,1.0D-6
*END STEP
```

ABAQUS Input Deck: Case 3a (Low Trapping)

```
*HEADING, UNSYMM
STATIC DIFFUSION -- STRONGLY COUPLED Standard Case 3 (200 elements) LOW TRAPPING
* *
       J(4.0E-5)=0.0
                                    C(L) = 1E - 7
**
     //-----
                                             F(x) = 15E6/(x + eps)^{(3/2)}
**
     11
       U(4.0E-5)=0.0
*PREPRINT
*NODE, NSET=A
1.4.0D-5
*NODE, NSET=B
401,0.10D0
*NFILL,NSET=ROD,BIAS=9.470049D-1,TWO STEP
A,B,400,1
*USER ELEMENT, NODES=3, UNSYMM, COORDINATES=1, PROPERTIES=11, VARIABLES=9, TYPE=U1
** THE FIRST AND THIRD NODES HAVE DEGREES OF FREEDOM IN CONCENTRATION AND
** DISPLACMENT, WHILE THE SECOND NODE ONLY HAS DISPLACEMENT DEGREES OF
** FREEDOM. NOTE THAT CONCENTRATIONS ARE TREATED AS TEMPERATURE D.O.F.
11,1
2,1
3,11,1
*ELEMENT, TYPE=U1, ELSET=SC1D
1,1,2,3
*ELGEN, ELSET=SC1D
1,200,2,1,1,1,1,1
*UEL PROPERTY, ELSET=SC1D
** PROPERTIES ARE IN THE ORDER:
** CFLAG, AREA, E, NU, D, ALPHA, CREF, RHO
** MW, TEMP, TRAPS
1.0D0,1.0D-4,200.0D9,0.3D0,1.0D-9,5.2D0,0.0D0,7.8D3
1.00797D-3,293.0D0,19.0D0
*MATERIAL, NAME=A4340
*DEPVAR
*BOUNDARY
** ZERO DISPLACEMENT AT LHS
*INITIAL CONDITIONS, TYPE=TEMPERATURE
** 1.0E-7 INITIAL CONCENTRATION OVER ROD LENGTH
ROD, 1.0D-7
*WAVEFRONT MINIMIZATION, SUPPRESS
*STEP, INC=5000, AMPLITUDE=STEP
*COUPLED TEMPERATURE-DISPLACEMENT, STEADY STATE
1.0D-4,1.0, ,5.0D-3
*CONTROLS, PARAMETERS=FIELD, FIELD=TEMPERATURE
, ,5.0D-21
*DLOAD, OP=NEW
SC1D, U1NU
*NODE PRINT, SUMMARY=NO, FREQUENCY=100
COOR1, U1, NT
*EL PRINT, SUMMARY=NO, FREQUENCY=100
SDV
*BOUNDARY
** SPECIFY RIGHT HAND BOUNDARY CONCENTRATION AS 1.0E-7
401,11,11,1.0D-7
*END STEP
```

ABAQUS Input Deck: Case 3a (High Trapping)

```
*HEADING, UNSYMM
STATIC DIFFUSION -- STRONGLY COUPLED Standard Case 3 (200 elements) HIGH TRAPPING
      C(L) = 1E - 7
       J(4.0E-5)=0.0
**
**
     //-
                                                 F(x) = 15E6/(x+eps)^{(3/2)}
* *
     //
**
       U(4.0E-5)=0.0
*PREPRINT
*NODE, NSET=A
1,4.0D-5
*NODE, NSET=B
401,0.10D0
*NFILL, NSET=ROD, BIAS=9.470049D-1, TWO STEP
A,B,400,1
*USER ELEMENT, NODES=3,UNSYMM,COORDINATES=1,PROPERTIES=11,VARIABLES=9,TYPE=U1
** THE FIRST AND THIRD NODES HAVE DEGREES OF FREEDOM IN CONCENTRATION AND
** DISPLACMENT, WHILE THE SECOND NODE ONLY HAS DISPLACEMENT DEGREES OF
** FREEDOM. NOTE THAT CONCENTRATIONS ARE TREATED AS TEMPERATURE D.O.F.
11,1
2,1
3,11,1
*ELEMENT, TYPE=U1, ELSET=SC1D
1,1,2,3
*ELGEN, ELSET=SC1D
1,200,2,1,1,1,1,1
*UEL PROPERTY, ELSET=SC1D
** PROPERTIES ARE IN THE ORDER:
** CFLAG, AREA, E, NU, D, ALPHA, CREF, RHO
** MW, TEMP, TRAPS
1.0D0,1.0D-4,200.0D9,0.3D0,1.0D-9,5.2D0,0.0D0,7.8D3
1.00797D-3,293.0D0,499.0D0
*MATERIAL, NAME=A4340
*DEPVAR
9
*BOUNDARY
** ZERO DISPLACEMENT AT LHS
*INITIAL CONDITIONS, TYPE=TEMPERATURE
** 1.0E-7 INITIAL CONCENTRATION OVER ROD LENGTH
ROD, 1.0D-7
*WAVEFRONT MINIMIZATION, SUPPRESS
*STEP, INC=5000, AMPLITUDE=STEP
*COUPLED TEMPERATURE-DISPLACEMENT, STEADY STATE
1.0D-4,1.0, ,5.0D-3
*CONTROLS, PARAMETERS=FIELD, FIELD=TEMPERATURE
  ,5.0D-21
*DLOAD, OP=NEW
SC1D, U1NU
*NODE PRINT, SUMMARY=NO, FREQUENCY=100
COOR1, U1, NT
*EL PRINT, SUMMARY=NO, FREQUENCY=100
*BOUNDARY
** SPECIFY RIGHT HAND BOUNDARY CONCENTRATION AS 1.0E-7
401,11,11,1.0D-7
*END STEP
```

*HEADING, UNSYMM STATIC DIFFUSION -- STRONGLY COUPLED Standard Case 3 (200 elements)

ample Input Deck

As with the notes on the User Element, these notes are complied for a user who is familiar with the typical components of an ABAQUS input deck. Explanations are primarily given for specific elements of the input deck which apply to User Elements in general or the Fully-Coupled Solute-Solid mixture theory in particular. Details concerning input deck command formats may be found in ABAQUS User's Manual II.

The sample input deck illustrates values typical of a steady-state coupled solute transport problem. The deck presented here is intended to demonstrate a variety of analysis options and their applications to the UEL routine. The problem geometry for this case is a $10\,cm$ long rod modeled using 200 finite elements with the specified boundary conditions:

Concentration Boundary Conditions $\begin{cases} \mathcal{J}(4.0\times 10^{-5} \, [m]) = 0.0 \, [m(gH/gFe)/s] \\ c(0.1 \, [m]) = 10^{-7} [gH/gFe] \end{cases}$ Momentum Boundary Conditions $\begin{cases} u(4.0\times 10^{-5} \, [m]) = 0.0 \, [m] \\ P(0.1 \, [m]) = -1000 \, [N] \end{cases}$

Body Force: $F_x = \frac{15 \times 10^6}{\sqrt{(x+\epsilon)^3}} \left[N/m^3 \right]$

Initial Concentration Distribution: $c(x) = 10^{-7}[gH/gFe]$

All of the property values are specified in SI basic units (kg,m,s), though any dimensionally consistent set of units can be used. All non-integer numerical values are entered using the double-precision format.

*NODE, NSET=A *NODE, NSET=B 1,4.0D-5

401,0.10D0 **

*NFILL, NSET=ROD, BIAS=9.470049D-1, TWO STEP

A, B, 400, 1

NODES=3,UNSYMM,COORDINATES=1,PROPERTIES=11,VARIABLES=9,TYPE=U1
** THE FIRST AND THIRD NODES HAVE DEGREES OF FREEDOM IN CONCENTRATION AND
** DISPLACMENT, WHILE THE SECOND NODE ONLY HAS DISPLACEMENT DEGREES OF
** FREEDOM. NOTE THAT CONCENTRATIONS ARE TREATED AS TEMPERATURE D.O.F. *USER ELEMENT,

3,11,1

NODE ASSIGNMENT follows standard ABAQUS conventions. Certain peculiarities arise in this problem which require care in the node selection process. The singular body force problem exhibits multi-valued solutions for the concentration as the singularity is approached. If one tries to use ABAQUS to solve the problem in this region, warnings for negative eigenvalues will be domain, as is done here by establishing the left-hand side of the model at $x = 4.0 \times 10^{-5}$ [m]; or returned. Two remedies exist for this problem, but both require an analytical solution for the problem. The user may restrict the region of the model to the well behaved portion of the solution the body force function may be altered so that the rod starts at x = 0 [m] but the singularity of the body force occurs at a point $x_s < 0$:

$$F_x = \frac{15 \times 10^6}{\sqrt{(x + ||x_s||)^3}}$$

NODAL GENERATION with biasing requires the use of the "two-step" parameter since the elements are three-noded and this places the internal node at the center of each undeformed models with different numbers of elements, the bias has been selected so that the ratio of the element (see AUM (II; 9.4.5-1) for further explanation of biasing). To allow comparison between lengths of the largest to smallest element in a model remain constant. Using this constraint, the bias of an n element model may be calculated as:

$$b_n = \left(b_N\right)^{\frac{N-1}{n-1}}$$

where N represents the number of elements in the "benchmark" model. The benchmark selected for this problem was a 10 element model with bias, $b_{10} = 0.3$.

corresponds to the TYPE parameter in the element definition. If the user wishes to study a model User Element Declaration should be entered into the input deck exactly as shown here for use subroutine does not check for element type, the TYPE declaration is arbitrary as long as it with the 1-D Coupled Deformation-Diffusion user element. Since the current coding of the UEL with more than one User Element type, coding must be added to the User Element to check this parameter, as specified in AUM (II; 5.2.16-7).

The degrees of freedom on the element are specified according to ABAQUS conventions as

$$\lfloor U(1), U(2), U(3), U(4), U(5) \rfloor = \lfloor c_1, u_1, u_2, c_2, u_3 \rfloor$$

The ABAQUS analysis type is "Coupled Temperature-Displacement, Steady-State", so the nodal concentrations, c., are specified as nodal temperatures.

```
1.0D0,1.0D-4,200.0D9,0.3D0,1.0D-9,5.2D0,0.0D0,7.8D3
1.00797D-3,293.0D0,499.0D0
                                                                                                                                                 ** PROPERTIES ARE IN THE ORDER:
** KFLAG, AREA, E, NU, D, ALPHA, CREF, RHO
** MW, TEMP, TRAPS
*ELEMENT, TYPE=U1, ELSET=SC1D
                                                                                                                              *UEL PROPERTY, ELSET=SC1D
                          1,1,2,3
*ELGEN, ELSET=SC1D
                                                                        1,200,2,1,1,1,1,1
```

** ZERO DISPLACEMENT AT LHS *MATERIAL, NAME=A4340 *BOUNDARY *DEPVAR

*INITIAL CONDITIONS, TYPE=TEMPERATURE ** 1.0E-7 INITIAL CONCENTRATION OVER ROD LENGTH ROD,1.0D-7

ELEMENT DEFINITION follows standard ABAQUS conventions. The TYPE specification must be identical to the type specification in the *USER ELEMENT command.

values must be entered in double-precision format, including the analysis case flag, KFLAG. The USER ELEMENT PROPERTIES are used to specify a particular analysis and the physical properties of the mixture. The physical properties are currently assumed to be constants. All required property specifications are defined below, and they must be entered in the given order, with 8 properties in the first line and 3 in the second, as specified by ABAQUS.

1. KFLAG1 = variable used to indicate the degree of coupling and the type of linear elastic constitutive behavior.

KFLAG values: 1 ⇒ Plane Stress, Fully Coupled

2 ⇒ Plane Strain, Fully Coupled

4 ⇒ Plane Strain, Weakly Coupled 3 ⇒ Plane Stress, Weakly Coupled

 $AREA \equiv cross-sectional area [m^2].$ $\mathbf{E} \equiv \text{modulus of elasticity } [Pa].$ 4 to 4 to

NU ≡ Poisson's ratio [1].

 $\mathbf{D} \equiv \text{diffusion coefficient } [m^2/s].$

ALPHA \equiv linear expansion coefficient for the solute $=\frac{1}{3}\frac{4}{Ms}\overline{V}_S[m/m/\Delta c_S]$. 9

CREF \equiv reference solute concentration (i.e., c_o term in $\Delta c = c - c_o$) [gSollgMix]

RHO \equiv density ($\simeq \rho_{\text{Solid}}$) [kg/m^3]. ∞;

MW \equiv molecular weight of solute species [kg/mol]. 6

10. TEMP \equiv temperature [K].

11. TRAPS² \equiv reversible trap site constant (i.e., $c_R^s K_R$) [1].

MATERIAL DEFINITION must be included to allow for the *DEPVAR sub option, which allows printout of the state variables calculated in the User Element. The name assigned to the material is arbitrary, since no material properties are included through this command.

ZERO BOUNDARY CONDITIONS are specified according to ABAQUS convention.

INITIAL CONDITIONS are specified according to ABAQUS convention.

¹ To run a case with no coupling (i.e. standard linear elasticity combined with classical diffusion) the parameter ALPHA must be set equal to zero. KFLAG is then used to select plane-stress or plane-strain analysis; changing between "Fully-Coupled" and "Weakly-Coupled" analysis will have no effect.

² For the hydrogen in steels, the term $(1 + c_h^2 K_R)$ is assumed to be in the range of 1 to 500, yielding a range in the TRAPS variable of 0 to 499.

*WAVEFRONT MINIMIZATION, SUPPRESS

*STEP, INC=5000, AMPLITUDE=STEP

*COUPLED TEMPERATURE-DISPLACEMENT, STEADY STATE 2.0D-4,1.0D5,2.0D-10,1.0D3

*CONTROLS, PARAMETERS=FIELD,FIELD=TEMPERATURE 5.0D-3,,5.0D-21

*DLOAD, OP=NEW SC1D, U1NU

*CLOAD, OP=NEW B, 1, -1.0D3

WAVEFRONT MINIMIZATION can be suppressed, since it is seldom needed in 1-D problems in which the element and node numbering begins at one end of the rod and continues along consecutively.

BEGIN ANALYSIS STEP

COUPLED TEMPERATURE-DISPLACEMENT analysis is used in conjunction with the User Element, with mass-fraction concentrations replacing the temperature degrees of freedom in the analysis.

flux, and a single criterion for both does not work very well. The steady state concentration flux for problems with concentrations of 10^{-7} specified at one end of the rod and 0 at the other end is CONTROLS for solution convergence must be set for the concentration (temperature) field because the magnitude of the concentration flux is much smaller than the magnitude of the stress on the order of 10⁻¹⁵, so specifying the initial control at 10⁻²¹ represents an essentially zero flux

using *AMPLITUDE definition. Type UnNU may be defined (in the UEL subroutine) as a DISTRIBUTED LOAD definition allows for the addition of body forces to the User Element Load type Un is independent of local or global position on the element, but may vary with time model. ABAQUS defines two types of distributed loads in a User Element: Un and UnNU. function of the global coordinates and time.

All UnNU type loads are defined in the subroutine FCALC in our user element coding. At present, the only UnNU load in subroutine FCALC is U1NU which corresponds to a singular body force of the form:

$$x_x = \frac{A}{\sqrt{(x+\epsilon)^3}} \left[\frac{N}{m^3} \right]$$

where A and ϵ are set within subroutine FCALC. ϵ is used to prevent a numerically unstable situation at x = 0. Additional load functions may be added by following the current format in subroutine FCALC.

calculating the corresponding consistent nodal loads and entering these in the concentrated load option, *CLOAD. These loads should be entered in using the proper ABAQUS format. CONCENTRATED LOADS Distributed surface tractions are included in the model by

*NODE PRINT, SUMMARY=NO, FREQUENCY=100 COOR1,U1,NT

*EL PRINT, SUMMARY=NO, FREQUENCY=100 SDV *BOUNDARY
** SPECIFY RIGHT HAND BOUNDARY CONCENTRATION AS 1.0E-7
401,11,11,1.0D-7

*END STEP

<u>OUTPUT SPECIFICATION</u> follows ABAQUS conventions. The *NODE PRINT option requests ABAQUS to print the nodal displacements in the axial direction (U1) and the nodal temperatures (NT), which, in this case, are actually the concentrations). The *EL PRINT request prints the state variables of the model under the output heading SDVi (i=1...9). These labels correspond to:

SDV1 \equiv Dilatational strain, e, at the left-hand integration point on the element [m/m]. SDV2 \equiv Dilatational strain, e, at the right-hand integration point on the element [m/m]. SDV3 \equiv Axial stress, σ_x , at the left-hand integration point on the element [Pa]. SDV4 \equiv Axial stress, σ_x , at the right-hand integration point on the element [Pa]. SDV5 \equiv Approximate axial stress, σ_x , at the left-hand end of the element [Pa]. SDV6 \equiv Approximate axial stress, σ_x , at the right-hand end of the element [Pa]. SDV7 \equiv Mass flux at the center of each element [m(gH/gFe)/s]. SDV7 \equiv Global coordinate of left-hand integration point (for plotting SDV1R3) [m]. SDV9 \equiv Global coordinate of right-hand integration point (for plotting SDV2R4) [m].

NON-ZERO BOUNDARY CONDITIONS are entered and applied according to standard ABAQUS conventions.

END ANALYSIS STEP

APPENDIX D: Sample Output

This Appendix contains a copy of an ABAQUS output listing generated in the solution of a 1-D fully coupled deformation-diffusion problem.

ABAQUS VEI	ABAQUS VERSION 5.3-1					DATE	DATE 17-MAR-95	TIME 14:29:46	PAGE	
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AAAAAAAAA	BEBEBEBE	AAAAAAAAA	Ø	Ø	Þ	D	SSSSSSSS			
A	В	A	A.	0	Þ	Þ	Ø			
A	В	A	A O	0	D	D	Ø			
A	В	Ą	A O	0	Þ	Þ	Ø			
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DATE 17-MAR-95

TIME 14:29:46

PAGE

THIS IS A PROPRIETARY PROGRAM AND IS MADE AVAILABLE FOR INTERNAL USE AT UNIVERSITY OF NOTRE DAME,

UNDER THE TERMS OF THE ACADEMIC LICENSE AGREEMENT WITH H.K.S. ALL USAGE MUST BE UNDER THE DIRECT SUPERVISION AND CONTROL OF THE DESIGNATED USER.

THE DESIGNATED USER IS PAUL GO. ANY NON-ACADEMIC USAGE OF THE PROGRAM REQUIRES PAYMENT OF A MONTHLY CHARGE. ASSISTANCE AND OTHER INFORMATION MAY BE OBTAINED FROM THE

DESIGNATED USER.

YOU ARE AUTHORIZED TO RUN ON MACHINE 8035E88D,

STANDARD, AQUA, AND POST UNTIL 11/30/95
YOUR SITE ID IS: U. NOTRE DAME

********** * NOTICE * THIS IS ABAQUS VERSION 5.3.

PLEASE MAKE SURE YOU ARE USING VERSION 5.3 MANUALS PLUS THE NOTES ACCOMPANYING THIS RELEASE. THESE NOTES CAN BE OBTAINED BY USING THE INFORMATION OPTION ON THE *

ABAQUS COMMAND LINE.

THIS PROGRAM MAY NOT BE USED FOR COMMERCIAL PURPOSES WITHOUT PAXMENT OF A MONTHLY CHARGE.

D-3

OR USE	AT UNIV	FOR USE AT UNIVERSITY OF NOTRE DAME UNDER ACADEMIC LICENSE FROM HKS, INC.
		ABAQUS INPUT ECHO 5 10 15 20 25 30 35 40 45 50 55 60 65 70 75 80
		*HEADING, UNSYMM STATIC DIFFUSION STRONGLY COUPLED Standard Case 1
CARD	S	C(0)=0.0 C(L)=1E-6
		** //
CARD	10	****************
		*PREPRINT *NODE, NSET=ROD 1,0.000 21,0.000
CARD	15	*NGEN,NSET=ROD
		*USER ELEMENT, NODES=3,UNSYMM,COORDINATES=1,PROFERTIES=11,VARIABLES=9,TYPE=U1 ** THE FIRST AND THIRD NODES HAVE DEGREES OF FREEDOM IN CONCENTRATION AND ** DISPLACMENT, WHILE THE SECOND NODE ONLY HAS DISPLACEMENT DEGREES OF
CARD	20	** FREEDOM. NOTE THAT CONCENTRATIONS ARE TREATED AS TEMPERATURE D.O.F. 2,1 3,11,1 *FREEDOM TVPR=111 PL.SEPT-COLD
CARD	25	1.1.2.3 *ELGEN, ELSET=SCID 1,10,2,1,1,1,1 **ED_PROPERTY_ELSET=SCID
CARD	30	** COPING, ARRA, E, NU, D, ALPHA, CREF, RHO, ** MW, TEMP, TRARS 1.0D0,1.0D-4,200,0D9,0.3D0,1.0D-9,5.2D0,0.0D0,7.8D3, **MM, TEMP, TRARS **MM, T
CARD	35	*DEPVAR 9 *BOUNDARY ** ZERO DISPLACEMENT AT LHS
CARD	40	** ZERO CONCENTRATION AT LHS
		5 10 15 20 25 30 35 40 45 50 55 60 65 70 75 80

	AT UNIT	FOR USE AT UNIVERSITY OF NOTRE DAME UNDER ACADEMIC LICENSE FROM HKS, INC.		
		5 10 15 20 25 30 35 40 45 50 55 60	65 70 75 8	80
		1,11 *INITIAL CONDITIONS, TYPE=TEMPERATURE ** 0.0 INITIAL CONCENTRATION OVER ROD LENGTH	1 1 1 1 1 1 1 1 1 1 1 1 1	!
CARD	45	ROD, 0.0D0 *WAVEFRONT MINIMIZATION, SUPPRESS *STEP, INC=1000 *COUPLED TEMPERATURE-DISPLACEMENT, STEADY STATE		
		0.005,1.0, , 0.05 **		
CARD	20	*CONTROLS, PARAMETERS=FIELD,FIELD=TEMPERATURE , ,5.0D-21 **		
CARD	55	*NODE PRINT, SUMMARY=NO,FREQUENCY=100 COOR1,U1,NT *EL PRINT, SUMMARY=NO,FREQUENCY=100		
CARD	09	*BOUNDARY ** SPECIFY RIGHT HAND BOUNDARY CONCENTRATION AS 1.0E-6 21,11,11,10D-6 *END STEP		
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MODELING OF HYDROGEN TRANSPORT

IN CRACKING METAL SYSTEMS

J. P. Thomas and C. E. Chopin University of Notre Dame Department of Aerospace and Mechanical Engineering 374 Fitzpatrick Hall Notre Dame, IN 46556-5637

Abstract

An extended stress-assisted diffusion theory with concentration induced deformations and equilibrium trapping effects was developed for modeling hydrogen transport in cracking metal systems. Special Fortran "user element" subroutines were written for the ABAQUS finite element code to solve the transport equations in 1-D geometries. A simple rod problem with a square-root singular stress was analyzed to assess the influence of the hydrogen induced deformation on the concentration and deformation field variables. Significant differences between the classical and extended stress-assisted diffusion theory were demonstrated and the implications for modeling crack tip hydrogen distributions discussed.

Introduction

Two fundamental questions naturally arise in modeling the influence of hydrogen on the crack growth rate of metals. Namely, how does hydrogen "enhance" the crack growth rate (CGR); and what is the relationship between the hydrogen distribution within the material and the corresponding "enhancement" in the CGR? Quantitative knowledge of the crack tip hydrogen distribution under service or laboratory test conditions is requisite to addressing these questions and will require: a) a hydrogen transport model that incorporates trapping and deformation-concentration coupling effects (governing equations); b) knowledge of the time dependent, non-uniform hydrogen distribution along the crack walls (boundary conditions); and c) a mathematical solution technique for the resulting system of non-linear equations.

Stress-assisted diffusion (SAD) theory [1] is commonly used to model hydrogen transport in cracking metals systems. This theory represents an extension of classical diffusion with a hydrostatic stress gradient term added as a driving force for diffusive transport. Equilibrium trapping effects are included through the use of an effective diffusion coefficient. The influence of hydrogen on the material deformation state is assumed to be negligible, and this uncouples the deformation equations from the concentration variable. Hydrostatic stresses determined from solutions to standard elasticity or plasticity problems are used with the SAD equation to solve for the resulting concentration. The steady-state hydrogen distribution for Mode I cracks in linear elastic materials with uniform hydrogen concentration along the boundaries is given by [2]:

$$c = c_0 \exp\left(constant \times \frac{K}{\sqrt{r}} \cos\frac{\theta}{2}\right)$$

where c_0 is the boundary concentration, K is the stress intensity factor, and r and θ are polar coordinates centered at the crack tip. An infinite hydrogen concentration is predicted at the crack tip. Transient and steady-state hydrogen distributions for a plastically deforming crack in iron have been obtained by Sofronis & McMeeking [3] using finite element methods. Their results show large, but finite, concentrations at the crack tip region, primarily in traps near the crack surface. They conclude that the crack tip hydrogen distribution is primarily determined by the creation of dislocation traps via plastic straining at the crack tip.

Damage models that attempt to link the crack tip hydrogen distribution to the fracture process have been reviewed in [4-6]. Applications to service cracking problems have met with some success, but the lack of information on the crack wall hydrogen distributions under service or laboratory conditions has limited the usefulness of these models.

The task of specifying the crack wall hydrogen distribution is difficult because of the complex nature of the interacting chemical, mechanical, and metallurgical processes operative during hydrogen assisted cracking of metals (Figure 1). In aqueous systems, the hydrogen production process is driven by the rapid and irreversible evolution of the chemically unstable "bare" surface at the crack tip to a more stable equilibrium "filmed" state. An electron flow is induced between the bare and filmed crack flank surfaces; net anodic (dissolution/filming) reactions take place on the bare surface and net cathodic (hydrogen reduction) reactions take place on the filmed surfaces.

Adsorbed hydrogen, H_{ads} , can be produced on both the bare and filmed crack surfaces by: (1) the reduction of hydrogen ions in acidic environments; or (2) by the reduction of water in alkaline environments. The MH_{ads} species are then free to be absorbed by the transition reaction (a); or combine to form H_2 gas via: recombination (b1); or electrochemical desorption (b2). Reactions (a), (b1), and (b2) occur in parallel, but one of the two (b1) or (b2) reactions is usually dominant (Figure 2).

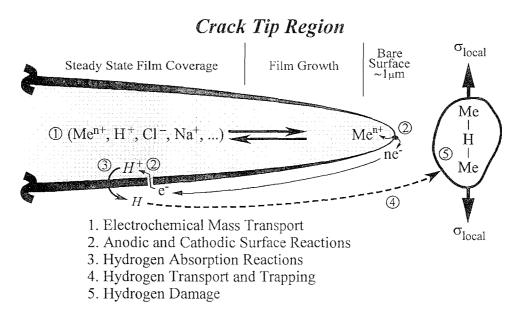


Figure 1: Schematic of the processes responsible for hydrogen assisted crack growth.

(1) Acidic:	$M + H^+ + e^- \Leftrightarrow MH_{ads}$
(2) Alkaline:	$M + H_2O + e^- \Leftrightarrow MH_{ads} + OH^-$

(a) Adsorption-Absorption:	$MH_{ads} \Leftrightarrow MH_{abs}$
(b1) Recombination:	$MH_{ads} + MH_{ads} \Leftrightarrow H_2 + 2M$
(b2) EC Desorption:	$MH_{ads} + H^+ + e^- \Leftrightarrow H_2 + M$

Figure 2: Summary of the hydrogen producing reactions.

The distribution of MH_{abs} along the crack surface is governed by the surface coverage of MH_{ads} and the kinetics of reaction (a) acting in parallel with reaction (b1) or (b2). These factors are influenced, in turn, by: the electrochemical environment at the crack tip region (e.g., the potential, pH, species concentrations, dissolved O_2 , etc.); the kinetics of the bare and filmed surface reactions; and the rate of transport of H_{abs} from the crack surface into the material.

Iyer and Pickering [7] review and model the mechanisms and kinetics of hydrogen evolution and entry in stress free metallic systems with homogeneous electrochemical conditions at the metal surface. Their model is used to quantify the rate constants associated with reactions (1) or (2) and (a) and (b1) or (b2) via analysis of experimental permeation data. Turnbull [8] has reviewed electrochemical conditions in cracks with particular emphasis on corrosion fatigue cracks of structural steels in sea water. Similarly, Beck [9] and Newman [10] have examined experimental techniques for characterizing bare (and filmed) surface reaction kinetics. It seems that the above models, data, and techniques, plus information concerning the rate controlling process during crack growth, will have to be used in an analysis of the mass transport process within the crack to develop realistic predictions of the MH_{ads} distribution.

The objective of this paper is to address the above items a) and c). A hydrogen transport model for isothermal linear elastic materials with mutual deformation-concentration coupling and equilibrium trapping is developed using continuum mixture theory. Computational solutions for 1-D geometries and arbitrary boundary conditions are obtained using the finite element code ABAQUS¹ supplemented with Fortran "user element" subroutines. The code is used to analyze hydrogen transport in a 4340 steel rod subjected to a singular body force($\sim A/x^{3/2}$) which produces a square root singular stress. The concentrations, displacements, and dilatational strains predicted by the fully coupled theory are somewhat larger than those predicted by classical SAD theory, depending on the extent of trapping. Further examination of the fully coupled theory in the context of hydrogen transport in 2-D crack systems is currently underway.

The paper begins with a description of the strongly coupled hydrogen transport model. The use of ABAQUS with its "user element" subroutines for solving 1-D problems is described next, followed by an analysis and discussion of hydrogen transport in a rod with a singular body force. Extension of the finite element model to 2-D geometries and its application to cracking metal systems is briefly discussed, followed by suggestions for future work.

¹ ABAQUS is a commercial finite element code supported by Hibbitt, Karlsson & Sorensen, Inc., Pawtucket, RI.

Modeling the Hydrogen Transport and Trapping Process

Three species of hydrogen are modeled in the analysis:

 $H_L \equiv$ interstitial or lattice hydrogen

 $H_R \equiv$ weak or moderately (reversibly) trapped hydrogen (e.g., $\Delta E_b \leq 30$)

 $H_I \equiv$ strongly (irreversibly) trapped hydrogen (e.g., $\Delta E_b > 30$ kJ/mol)

Balance Equations

Balance equations for the mixture mass, the three hydrogen species masses, the mixture linear momentum and moment of momentum, mixture energy, and mixture entropy can be written. Since our modeling considerations are restricted to isothermal linear elastic materials, only the balance equations for the hydrogen species mass and the mixture linear momentum (assuming negligible inertial effects) need be explicitly considered. They are given by:

Mass:
$$\frac{\partial c_k}{\partial t} + \vec{\nabla} \cdot \vec{\mathcal{J}}_k = a_k \quad (k = L, R, or I)$$
 (1)

Linear Momentum:
$$\sigma_{ij,j} + F_i = 0 \quad (i, j = x, y, or z)$$
 (2)

where: $c_k \equiv \text{mass fraction concentrations for } H_k \left[kg/kg \right]$.

 $\vec{\mathcal{J}}_k \equiv \text{concentration flux vectors for } H_k \left[kg/kg \cdot m/s \right].$

 $a_k \equiv \text{mass supply rates for } H_k \left[kg/kg/s \right].$

 $\sigma_{ij} \equiv \text{stress tensor} \left[N/m^2 \right].$

 $F_i \equiv \text{the } i^{th} \text{ component of body force vector } [N/m^3].$

Trapping Analysis

Expressions for the mass supply rates a_k in Eq. (1) are written in accordance with the trapping model of McNabb & Foster [11]:

Stoichiometry:
$$H_L \Leftrightarrow H_R \quad \text{and} \quad H_L \Rightarrow H_I$$
 (3)

The stoichiometric relations require the supply terms sum to zero (i.e., $a_R + a_I = -a_L$).

Kinetics of Trapping:
$$a_R = k_R^f (1 - \theta_R) c_L - k_R^b \theta_R$$

$$a_I = k_I^f (1 - \theta_I) c_L - k_I^b \theta_I \approx k_I^f (1 - \theta_I) c_L$$
 (4)

where: k_R^f , k_I^f , k_R^b , $k_I^b \equiv$ forward and backward trap rate constants for H_R and H_I [1/s]. θ_R and $\theta_I \equiv c_R/c_R^s$ and c_I/c_I^s , respectively, are the fraction of filled trap sites [1]. c_R^s and $c_I^s \equiv$ saturation mass fraction concentration of H_R and H_I [kg/kg].

The quantities k_R^f , k_I^f , k_R^b , k_I^b , c_R^s and c_I^s are related to trap site densities, probability of capture, etc. and can be determined from experimental measurements (e.g., [12-14]).

Significant simplifications are effected when the rate constants for trapping are much greater than those for diffusive transport. Trapping can then be modeled as a steady-state process (i.e., $a_L = a_R = a_I = 0$). This case is considered below.

$$c_R = \frac{c_R^s K_R c_L}{1 + K_R c_L} \approx c_R^s K_R c_L \quad \& \quad c_I = c_I^s$$

$$K_R \equiv \frac{k_R^f}{k_R^b} = \exp\left(\frac{-\Delta E_b}{\mathbb{R} T}\right)$$
(5)

so that the total internal hydrogen concentration is simply a linear function of $c_L(x_i, t)$:

$$c_{TOTAL}(x_i, t) = c_L(x_i, t) + c_R(x_i, t) + c_I(x_i, t) = (1 + c_R^s K_R)c_L(x_i, t) + c_I^s$$
(6)

The three versions of Eq. (1) can be summed to give a single equation by the following considerations. First, $\vec{\mathcal{J}}_R = \vec{\mathcal{J}}_I = 0$ because of the linear elastic material assumption which precludes trap site motion (e.g., dislocation motion during plastic deformation). And second, from Eq. (5a):

$$\frac{\partial c_R}{\partial t} \approx c_R^s K_R \frac{\partial c_L}{\partial t}$$
 and $\frac{\partial c_I}{\partial t} = 0$ (7)

The resulting mass balance equation is given by:

$$(1 + c_R^s K_R) \frac{\partial c_L}{\partial t} + \vec{\nabla} \cdot \vec{\mathcal{J}}_L = 0$$
 (8)

Constitutive Equations

Mass Flux:

$$\vec{\mathcal{J}}_L = -\frac{c_L D_L}{R_L T} \vec{\nabla} \mu_L \quad \text{Linear Onsager Relation}$$
 (9)

where μ_L is the *mass* based chemical potential [15] defined by:

$$\mu_L \equiv \frac{\partial \psi}{\partial c_L} = \frac{1}{\mathcal{M}_L} \left(\mu_L^o(T) + \mathbb{R}T \ln(c_L) - \overline{V}_H ke \right) \qquad \left[\frac{J}{g} \right]$$
 (10)

and:

 $D_L \equiv {
m diffusion\ coefficient\ for\ } H_L\ [m^2/s].$

 $R_L \equiv \text{gas constant for } H_L = \mathbb{R}/\mathcal{M}_L [J/kg \cdot {}^{\circ}K].$

 $\mathcal{M}_L \equiv \text{molecular mass of hydrogen } [kg/mol].$

 $T \equiv \text{temperature } [^{\circ}K].$

 $\psi \equiv \psi(c_L,\,c_R,\,c_I;\epsilon_{ij}\,;\,T)$ free energy per unit mixture mass [J/kg].

 $\mu_L^o(T) \equiv$ reference potential for H_L at temperature T[J/kg].

 $\overline{V}_H \equiv \text{partial molar volume of hydrogen in the metal } [m^3/mol H].$

 $k \equiv \text{bulk modulus of elasticity } [N/m^2].$

 $e \equiv$ trace of the strain tensor (i.e., $e = \epsilon_{ii} = \epsilon_{11} + \epsilon_{22} + \epsilon_{33}$) [m/m].

The use of mass based chemical potentials simplifies the analysis of fully coupled deformation-diffusion processes because of the primary role played by mass in the deformation equations.

The resulting expression for concentration flux is given by:

$$\vec{\mathcal{J}}_L = -D_L \, \vec{\nabla} c_L + \frac{\overline{V}_H \, D_L}{\mathbb{R} \, T} \, k c_L \vec{\nabla} e \tag{11}$$

The constitutive equation for the stress consists of Hooke's law combined with a dilatational stress contribution due to changes in the total hydrogen concentration:

Stress:
$$\sigma_{ij} \equiv \rho \, \frac{\partial \psi}{\partial \epsilon_{ij}} = \lambda \, e \, \delta_{ij} + 2G \epsilon_{ij} - 3 \, k \, \alpha_H \, (1 + c_R^s K_R) \Delta \, c_L \, \delta_{ij}$$
 (12)

where $\rho \equiv \text{mass density of the solid } [kg/m^3].$

 $\lambda \equiv \text{Lame' constant } [N/m^2].$

 $\delta_{ij} \equiv \text{Kronecker delta } (\delta_{ij} = 1 \text{ for } i = j \text{ and } 0 \text{ otherwise}).$

 $G \equiv \text{shear modulus } [N/m^2].$

 $\epsilon_{ij} \equiv \frac{1}{2} (u_{i,j} + u_{j,i})$ infinitesimal strain tensor [m/m].

 $u_i \equiv \text{the } i^{th} \text{ component of the displacement vector } [m].$

 $\alpha_H \equiv \text{linear expansion coefficient for internal hydrogen} = \frac{1}{3} \frac{\rho}{\mathcal{M}_L} \overline{V}_H \left[m/m/\Delta c_H \right].$

 $(1 + c_R^s K_R) \Delta c_L \equiv$ change in c_{total} from some reference level.

The relationship between the chemical potential and stress (Eqs. (10) and (12)) is dictated by the thermodynamic reciprocity relationship:

$$\frac{\partial \mu_L}{\partial \epsilon_{ij}} = \frac{\partial (\sigma_{ij}/\rho)}{\partial c_L} \tag{13}$$

Governing Equations

Combining the mass and momentum balance equations with the constitutive relations results in the following system of governing equations for transport:

Diffusion Equation:

$$\frac{\partial c_L}{\partial t} = D_{eff} \, \nabla^2 c_L - \frac{\overline{V}_H \, D_{eff}}{\mathbb{R} \, T} k \Big(\, \vec{\nabla} c_L \cdot \vec{\nabla} e + c_L \nabla^2 e \Big)$$
 (14)

Deformation Equations (i = 1, 2, 3):

$$(\lambda + G)\frac{\partial e}{\partial x_i} + G\nabla^2 u_i + F_i = 3k \alpha_H (1 + c_R^s K_R) \frac{\partial c_L}{\partial x_i}$$
(15)

where D_{eff} is an "effective" diffusion coefficient defined by: $D_{eff} \equiv D_L/(1+c_R^s K_R)$.

Equation (14) is identical to the stress-assisted diffusion equations published in the literature with the exception of the $\nabla^2 e$ term which is identically zero when linear elastic material behavior is assumed (and $F_i = 0$). In the present formulation it is given by:

$$\nabla^2 e = \frac{3k \,\alpha_H}{\lambda + 2G} (1 + c_R^s K_R) \nabla^2 c_L \tag{16}$$

Computational Modeling

Equations (14) and (15) form a system of non-linearly coupled partial differential equations that must be solved for c_L and u_i as functions of the space and time coordinates (x_i, t) . This is accomplished for 1-D geometries by writing Fortran "user element" subroutines for the ABAQUS finite element code to effect a computational solution. The 1-D "plane stress" form of the governing equations² $(\sigma_{yy} = \sigma_{zz} = 0)$ is given by

² "Plane strain" forms of the 1-D equations can be obtained by a simple change in constants.

$$(1 + c_R^s K_R) \frac{\partial c_L}{\partial t} + \frac{\partial \mathcal{J}_L}{\partial x} = 0$$

$$\mathcal{J}_L = -D_L \frac{\partial c_L}{\partial x} + \frac{\overline{V}_H D_L}{\mathbb{R} T} k c_L \frac{\partial e}{\partial x}$$
(17)

Deformation:

$$\frac{\partial \sigma_x}{\partial x} + F_x = 0 \tag{18}$$

$$\sigma_x = 3k(e - 3lpha_H \left(1 + c_R^s K_R
ight)\!\Delta\,c_L) = E\!\left(rac{\partial u}{\partial x} - lpha_H \left(1 + c_R^s K_R
ight)\!\Delta\,c_L
ight)$$

A single differential equation for c_L can be obtained by combining Eqs. (17) and (18):

$$\frac{\partial c_L}{\partial t} = \frac{\partial}{\partial x} \left[D_{eff} \left(1 - \frac{\rho \, \overline{V}_H^2 \, k}{\mathcal{M}_L \, \mathbb{R} \, T} (1 + c_R^s \, K_R) c_L \right) \frac{\partial c_L}{\partial x} + \frac{\overline{V}_H \, D_{eff}}{3 \, \mathbb{R} \, T} F_x c_L \right] \tag{19}$$

which, in the absence of body forces, is a standard diffusion equation with a concentration dependent diffusion coefficient. Solutions to the steady-state problem, with and without body forces, can be obtained by integration (symbolic computation is highly recommended).

A coupled set of equations for a single finite element of length h and cross-sectional area A can be written from Eqs. (17-18) using the method of weighted residuals. The result, after integration by parts, is given by:

$$\int_{0}^{h} \left((1 + c_{R}^{s} K_{R}) \frac{\partial c_{L}}{\partial t} \{ G_{i}(x) \} - \mathcal{J}_{L} \left\{ \frac{\partial G_{i}(x)}{\partial x} \right\} \right) dx = - \mathcal{J}_{L} \{ G_{i}(x) \} \Big|_{0}^{h}$$

$$\int_{0}^{h} \left(-\sigma_{x} \left\{ \frac{\partial H_{j}(x)}{\partial x} \right\} + F_{x} \{ H_{j}(x) \} \right) dx = -\sigma_{x} \{ H_{j}(x) \} \Big|_{0}^{h}$$

$$(20)$$

where:

$$c_{L} \equiv \sum_{i=1}^{2} G_{i}(x)c_{i}(t) = \lfloor G_{i}(x) \rfloor \{c_{i}(t)\} \qquad i = 1, 2$$

$$u \equiv \sum_{j=1}^{3} H_{j}(x)u_{j} = \lfloor H_{j}(x) \rfloor \{u_{j}\} \qquad j = 1, 2, 3$$
(21)

are 2 and 3 node finite element representations for the lattice concentration and displacement, $G_i(x)$ and $H_j(x)$ are the respective linear and quadratic interpolation functions, and $\lfloor \cdots \rfloor$ indicates a row matrix while $\{\cdots\}$ indicates a column matrix.

Substituting Eqs. (21) into (17b) and (18b) and converting to isoparametric coordinates, results in finite element equations of the form:

$$\begin{bmatrix} \begin{bmatrix} C_c \end{bmatrix} & \begin{bmatrix} 0 \\ 0 \end{bmatrix} & \begin{Bmatrix} \{\dot{c}_i \} \\ \{\dot{u}_j \} \end{bmatrix} + \begin{bmatrix} \begin{bmatrix} K_c \end{bmatrix} & \begin{bmatrix} K_{cu} \\ K_{uc} \end{bmatrix} & \begin{bmatrix} K_{cu} \\ K_{u} \end{bmatrix} \end{bmatrix} \begin{pmatrix} \{c_i \} \\ \{u_j \} \end{pmatrix} = \begin{pmatrix} \{R_c \} \\ \{R_u \} \end{pmatrix}$$
 (22)

The capacitance, stiffness, and RHS matrices, [C], [K], and $\{R\}$, are functions of c_L and u, which necessitates the use of nonlinear solution techniques. The integrations required for the computation of the [C] and [K] matrices are accomplished using two-point Gauss quadrature.

The term $\partial e/\partial x$ in \mathcal{J}_L includes the second derivative in u (from Eq. (18b):

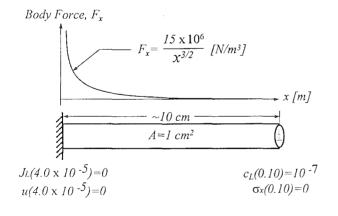
$$\frac{\partial e}{\partial x} = (1 - 2\nu) \frac{\partial^2 u}{\partial x^2} + 2(1 + \nu)\alpha_H \left(1 + c_R^s K_R\right) \frac{\partial c_L}{\partial x}$$
(23)

This would normally require the use of C^1 continuous elements in order for u to satisfy the element interface compatibility requirement [16]. To avoid this complication, we use the values of u and c_L from the previous time step to approximate $\partial e/\partial x$ in the current time calculations.

The deformation equations are analogous to those of linear thermoelasticity. This allows the use of ABAQUS' coupled temperature-displacement routine to solve the problem. Time integration is performed using an implicit backwards difference scheme. The computations and setup required of the user element subroutine include [17]: calculation of the [C], [K] and $\{R\}$ matrices; calculation of the "Jacobian" and {right-hand-side} matrices for the Newton-Raphson non-linear solver routine; and updating the solution dependent state variables: \mathcal{J}_L , σ_x , e, and $\partial e/\partial x$. The subroutine is written in double precision Fortran, and the code is run on a Sun SPARC10 workstation.

Applications

Steady-state hydrogen transport in a 4340 steel rod subjected to a body force selected to give a $1/\sqrt{x}$ singular stress (Figure 3) was studied using the ABAQUS code. The rod was discretized using 200 elements of length $l_i=0.104/0.94^{i-1}$ [μm], where the first and smallest element ($l_1=0.104$ [μm]) was placed at the left-hand end of the bar ($x=4\times 10^{-3}$ [cm]). The parameter values used in the analysis were: $\rho=7.8$ [g/cm^3]; T=293 [K]; $D_L=1\times 10^{-5}$ [cm^2/s]; $\overline{V}_H=2.0$ [cm^3/mol]; $\mathcal{M}_L=1.008$ [g/mol]; $(1+c_R^sK_R)=20$ (low trapping) & 500 (high trapping); E=200 [GPa]; $\nu=0.3$; $F_x=15\times 10^6$ [N/m^3], and $(c_L)_{ref}=0$.



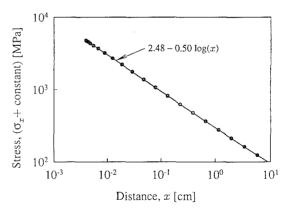


Figure 3: The rod geometry with the applied body force and boundary conditions.

Figure 4: Gauss integration point stresses with a least squares line fit through the data.

Results

An analytical solution to this problem³ was obtained by direct integration of Eq. (19). The singular nature of the body force results in a mathematical solution for the concentration that is a multi-valued function of x. In order to avoid the multiple concentration values, we must restrict

³ To be described in more detail in a future publication.

our considerations to the region $x > 4 \times 10^{-3} \ [cm]$. The difficulty arises when the sign of the $\partial c_L/\partial x$ coefficient term in Eq. (19) changes from positive to negative. That is, when c_L exceeds some "critical" value given by: $\mathcal{M}_L \mathbb{R} T/\rho \overline{V}_H^2 k(1+c_R^s K_R)$.

Figures 4 through 8 show the steady-state analytical and finite element (FE) results (every 20th point) for the high and low trapping conditions. Figure 4 shows the FE stresses at the Gauss integration points with a least squares fit line through the FE data. The stress in this problem is not influenced by the hydrogen; it is simply related to the integral of the body force per Eq. (18a). The required square root singularity in the stress is clearly predicted by the FE code as indicated by the slope of the least squares line fit.

Figures 5 & 6 show the concentration distribution for the fully coupled and stress-assisted diffusion (SAD) theories with $\sigma_{HYD}=10\times 10^6/\sqrt{x}$. The fully coupled model predicts larger concentrations than the SAD model as the singularity is approached, and the difference increases as the degree of trapping increases (i.e., $1+c_R^sK_R=500$ -vs.- 20). Figure 6 shows that the fully coupled concentration singularity is more severe than the exponential square root singularity of the SAD model for the higher trapping case.

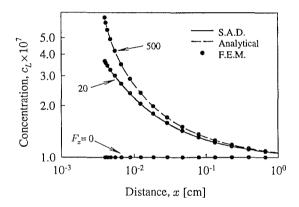


Figure 5: Concentration predictions for the fully coupled and SAD theory under high and low trapping conditions.

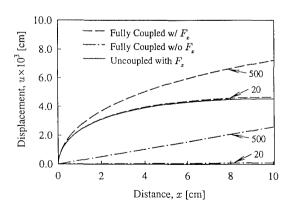


Figure 7: Nodal displacement curves for a variety of conditions.

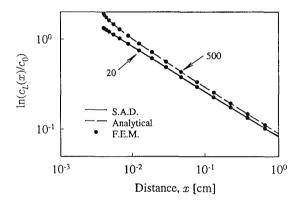


Figure 6: Plot to illustrate the difference in singular behavior between the fully coupled and SAD model concentration predictions.

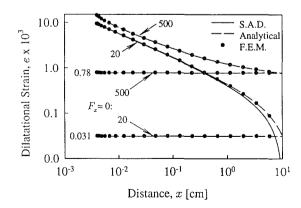


Figure 8: Analytical and FE dilatational strains for a variety of conditions.

Figures 7 & 8 show nodal displacement curves and Gauss point dilatational strains, respectively. The fully coupled theory predicts larger displacements and dilatational strains throughout the rod. Again, the differences increase as the degree of trapping increases. The displacements and strains for the zero body force case are also shown.

Discussion

This simple rod problem with its square root singular stress clearly illustrates the differences between the fully coupled and classical SAD models. The concentration differences are relatively small but dependent on the trapping constants (i.e., the amount of trapped hydrogen). The non-physical nature of the multi-valued concentration in the fully coupled theory may be an indication that singularities, like the inverse square root singularity found at 2-D crack tips in classical elasticity, do not naturally arise in the fully coupled theory. This could have important implications in modeling the hydrogen damage at 2-D crack tips.

The influence of the concentration coupling to the deformation state, and its role in the hydrogen damage process, is also an important consideration. The fully coupled theory exhibits larger dilatational strains than those predicted by SAD theory (classical elasticity), but identical stresses. On the other hand, a bar fixed between two walls and subject to a change in concentration results in differences between both strain *and* stress. This is also likely to be true in two and three-dimensional problems where the deformations in different directions are coupled through the constitutive equations.

The results suggest the need for an examination of the coupled theory in the context of crack tip hydrogen predictions. We are currently developing 2-D isoparametric rectilinear (8-node displacement; 4-node concentration) and axisymmetric user element routines for use in this task. Extension of the model to include the effects of non-equilibrium trapping, crack tip plasticity, and hydride formation will be considered in the future.

Acknowledgments

The discussions with Dr. D. Kirkner concerning the finite element aspects of this work, and the support of this work by the Office of Naval Research under Contract No. N00014-93-1-0845, are both gratefully acknowledged.

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APPENDIX F: Summary Of The Publications/Reports/Presentations

1. Papers Published in Refereed Journals:

- 1. J. P. Thomas and R. P. Wei, "Standard Error Estimates for Rates of Change From Indirect Measurements", *TECHNOMETRICS*, (in review).
- 2. J. P. Thomas and P. Matic, "Solute Transport Modeling in Elastic Solids", *International Journal of Engineering Science*, in preparation.

2. Non-Refereed Publications and Technical Reports:

- 1. J. P. Thomas and C. E. Chopin, "Modeling of Hydrogen Transport in Cracking Metal Systems", *Proceedings of the 5th International Conference on Hydrogen Effects on Material Behavior*, A. W. Thompson and N. R. Moody, Eds., The Materials Society (TMS), Jackson Lake Lodge, Wyoming, Sept. 1994 (to appear).
- 2. J. P. Thomas and C. E. Chopin, "An ABAQUS User Element Routine for One-Dimensional Coupled Deformation-Diffusion Problems", *Department of Aerospace and Mechanical Engineering Report*, University of Notre Dame, Department of Aerospace and Mechanical Engineering, Notre Dame, IN, (in preparation).

3. Presentations

a. Invited:

- 1. J. P. Thomas, April 1994, "Modeling the Influence of Hydrogen on the Crack Growth Rate of Metals", Lehigh University, Department of Mechanical Engineering and Mechanics, Bethlehem, PA.
- 2. J. P. Thomas, July 1994, "Environmental Effects in Fatigue Crack Growth", Ladish Corporation, Inc., Cudahy, Wisconsin.
- 3. J. P. Thomas, October 1994, Invited Lecture, "Solute Transport in Elastic Solids", D. G. B. Edelen Symposium, 31st Annual Technical Meeting of the Society of Engineering Science (SES), College Station, Texas.
- 4. J. P. Thomas, October 1994, "Modeling the Influence of Internal Hydrogen on the Crack Growth Rate of Metals", University of Kentucky, Department of Engineering Mechanics, Lexington, Kentucky.
- 5. J. P. Thomas, January 1995, "Modeling the Influence of Internal Hydrogen on the Crack Growth Rate of Metals", Westinghouse Electric Corporation, Bettis Atomic Power Laboratory, West Mifflin, Pennsylvania.

b. Contributed:

- 1. J. P. Thomas, September 1994, "Modeling Crack Tip Hydrogen Distributions", Poster Session, 5th International Conference on *Hydrogen Effects on Material Behavior*, The Materials Society (TMS), Jackson Lake Lodge, Wyoming.
- 2. J. P. Thomas and C. E. Chopin, October 1994, "Finite Element Modeling of Hydrogen Transport in Metals", 31st Annual Technical Meeting of the Society of Engineering Science (SES), College Station, Texas (planned).

4. Books: none

5. List Of Honors/Awards:

Name of Person Receiving Award

Mr. Charles E. Chopin, Graduate Student

Recipient's Institution

Department of Aerospace and Mechanical Engineering University of Notre Dame Notre Dame, IN

Name, Sponsor and Purpose of the Award

SES 31st Annual Technical
Meeting Student Stipend
National Science
Foundation & Office of
Naval Research
To support student
participation in the SES
Annual Meeting.

6. Participants And Their Status

Principal Investigator:

1. Dr. James P. Thomas, Assistant Professor, University of Notre Dame, Department of Aerospace and Mechanical Engineering, Notre Dame, IN.

Graduate Student:

1. Mr. Charles E. Chopin, 2nd year Doctoral Student, University of Notre Dame, Department of Aerospace and Mechanical Engineering, Notre Dame, IN.